

**GEOTECHNICAL AND INFILTRATION EVALUATION
PROPOSED RESIDENTIAL DEVELOPMENT
ASSESSOR'S PARCEL NUMBERS (APNs) 167-091-09 THROUGH -12
NORTHEAST OF THE 210 FREEWAY AND WEST SAN BERNARDINO AVENUE
REDLANDS, SAN BERNARDINO COUNTY, CALIFORNIA**

PREPARED FOR

**REDLANDS PIONEER PARTNERS, LLC
c/o CITIVEST
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PREPARED BY

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April 8, 2025
Project No. 4212-CR

Redlands Pioneer Partners, LLC
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Attention: Mr. Quantas Corman and Mr. Michael Mossman

Subject: **Geotechnical and Infiltration Evaluation**
Proposed Residential Development
Assessor's Parcel Numbers (APNs): 167-091-09 through -12
Northeast of the 210 Freeway and West San Bernardino Avenue
Redlands, San Bernardino County, California

Dear Mr. Quantas Corman and Mr. Michael Mossman:

GeoTek, Inc. (GeoTek) is pleased to provide the results of this Geotechnical and Infiltration Evaluation for a proposed residential development that will be constructed at the northeast intersection of the 210 Freeway and West San Bernardino Avenue, in the City of Redlands, San Bernardino County, California. This report presents a discussion of GeoTek's evaluation and provides preliminary geotechnical recommendations for site preparation, foundation design, preliminary infiltration rates and construction of the proposed site improvements.

Based upon review and evaluation, site development appears feasible from a geotechnical viewpoint provided that the recommendations included in this report are incorporated into the design and construction phases of site development.

The opportunity to be of service is sincerely appreciated. If you have any questions, please do not hesitate to contact GeoTek.

Respectfully submitted,
GeoTek, Inc.



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I. PURPOSE AND SCOPE OF SERVICES

The purpose of this study was to evaluate the site geotechnical conditions for a proposed residential development to be constructed at the northeast corner of the 210 Freeway and West San Bernardino Avenue, in the City of Redlands, San Bernardino County, California, as outlined in GeoTek's proposal P-0102225-CR dated February 7, 2025. Services provided for this study included the following:

- Research and review of available geologic and geotechnical data and general information pertinent to the site,
- A site reconnaissance,
- Excavation of five (5) exploratory borings to depths ranging between about 15 to 51.5 feet below existing grades and advancement of four (4) Cone Penetration Test (CPT) soundings to a maximum depth of approximately 60 feet below existing grades for the geotechnical portion of the evaluation,
- Excavation of four (4) additional test borings to depths of about 5 feet below existing grades in anticipated water quality management basin areas and performing a percolation test in each boring to estimate preliminary infiltration rates of the subsoils for the preliminary design of water quality management basins,
- Collection of soil samples in the test borings,
- Laboratory testing of selected soil samples,
- Review and evaluation of site seismicity, and
- Compilation of this geotechnical report which presents preliminary recommendations for site development.

The intent of this report is to aid in the evaluation of the site for future proposed development from a geotechnical perspective. The professional opinions and geotechnical information contained in this report may need to be updated based upon review of the final site development plans. These plans should be provided to GeoTek Inc. for review when available.

2. SITE DESCRIPTION AND PROPOSED DEVELOPMENT

2.1 SITE DESCRIPTION

The approximate 31.73-acre site is located northeast of the intersection of the 210 Freeway and West San Bernardino Avenue, in the City of Redlands, San Bernardino County, California (See Figure 1). The site is comprised of four (4) parcels of land identified as San Bernardino County Assessor's Parcel Number's (APN's) 167-091-09 through -12. The subject site is bounded by Pioneer Avenue, followed by vacant land and Citrus Valley High School to the north; a residential development to the east; West San Bernardino Avenue, followed by vacant land to the south; and a concrete lined drainage channel and the 210 Freeway to the west. Tennessee Street, a paved improved street, traverses through the center of the site in a north to south direction. The site is gently sloping from east to west, with an approximately grade change of 20 feet. Surface drainage is directed by site topography to the west.

Based on review of historical aerial photographs (<https://www.historicaerials.com/viewer>), the site appears to have been previously utilized as an agricultural orchard since 1938, the oldest available imagery available. It appears the orchard was removed from the site in 2003. Based on Google Earth aerial imagery and GeoTek's recent site reconnaissance, the site can currently be considered as vacant land. There appears to be a stormwater drainage or an irrigation utility that traverses through the center of the site from east to west and outlets to a concrete lined drainage channel parallel to the western boundary of the site.

2.2 PROPOSED DEVELOPMENT

Based on the referenced "Conceptual Site Plan, Pioneer and Tennessee Site," prepared by WHA., dated January 24, 2024 and information provided by Heather Pearce of TK Consulting, Inc., GeoTek understands that the property will be developed with about 289 two-story residential units consisting of 94 duplexes and 195 single family residential structures along with associated improvements (See Figure 3). Associated improvements are anticipated to include concrete flatwork, hardscapes, interior streets, perimeter walls, utilities, and typical landscaping. Tennessee Street, a paved roadway, currently traverses through the center of the project site in a north to south direction and is proposed to be renamed New York Street and relocated parallel to the eastern boundary of the project site. Currently, two water quality basins are proposed in the northwest and southwest corners of the project site.

It is anticipated that the proposed residential structures will be up to two-stories in height, of wood frame construction and supported by post-tensioned (PT) slab-on-grade foundation systems. Although structural loading information was not provided, GeoTek has assumed the structures will exert relatively light to moderate foundation loads. Maximum column and wall loads on the order of 50 kips and 2.5 kips per lineal foot, respectively, are estimated for the proposed structures. Once actual loads are known, that information should be provided to GeoTek to determine if modifications to the recommendations presented in this report are warranted.

Cut and fills of less than approximately 5 feet in height, not including any recommended remedial grading are anticipated for site development. Retaining and perimeter walls are anticipated. Sewage disposal is anticipated to be provided by a public sewer system. If site development differs from the assumptions made herein, the recommendations included in this report should be subject to further review and evaluation.

3. FIELD EXPLORATION AND LABORATORY TESTING

3.1 FIELD EXPLORATION

GeoTek conducted a field reconnaissance prior to field explorations at the site. The field explorations for this evaluation were conducted on February 31, 2025 and March 24, 2025. For the geotechnical portion of the investigation, five (5) exploratory borings (Borings B-1 through B-5) were excavated with a hollow-stem auger drill rig to depths ranging from about 15 to 51.5 feet below the existing ground surface at the boring locations and four (4) CPT soundings were advanced to depths ranging from about 42 to 60 feet below the existing ground surface at the sounding locations. CPT-2 was terminated prior to achieving the proposed depth (60 feet) due to encountering very dense soils and/or gravel/cobbles.

For the infiltration part of the study, four (4) test borings were excavated to depths of approximately 5 feet, below the existing ground surface in the locations of the proposed water quality basins.

A hollow-stem auger with an outside diameter of approximately 8.0 inches was utilized to conduct the borings. The inside diameter of the auger was approximately 4.5 inches. The CPT soundings were performed by Kehoe Testing & Engineering utilizing a 30-ton CPT rig equipped with a 15 cm² cone.



In the geotechnical borings, relatively undisturbed soil samples were recovered at various intervals with a California sampler. The California sampler is a 2.9-inch outside diameter, 2.5-inch inside diameter, split barrel sampler lined with brass rings. The sampler is approximately 18 inches in length. The sampler conformed to the requirements of ASTM D 3550. A 140-pound automatic trip hammer was utilized, dropping approximately 30 inches for each blow. The relatively undisturbed samples, together with bulk samples of representative soil types, were returned to the laboratory for testing and evaluation.

In Boring B-4, standard penetration tests (SPT) were performed with a 2.0-inch outside diameter, 1.5-inch inside diameter, and split-barrel sampler. The sampler was 18 inches long. The inside diameter of the sampler shoe was 1.4 inches. The sampler was unlined. The sampler conformed to the requirements of ASTM D 1586. A 140-pound automatic trip hammer was utilized, dropping 30 inches for each blow. The sampler penetration test data are presented on the Logs of Borings for Boring B-4.

A geologist from GeoTek observed the CPT soundings, kept detailed logs of the borings, and collected disturbed as well as undisturbed soil samples at selected boring depths. The approximate locations of these excavations are shown on Figure 2 – Site Exploration Location Map. Detailed boring logs and the results of the CPT soundings are presented in Appendix A. The exploration logs in Appendix A show subsurface conditions at the dates and locations indicated and may not be representative of other locations and times. The stratification lines presented on the logs represent the approximate boundaries between soil types, and the transitions may be gradual.

Infiltration Testing

Four (4) borings (Borings I-1 through I-4) were excavated on the project site to evaluate preliminary infiltration rates for the proposed water quality basins. Infiltration testing was conducted in these borings in general accordance County of San Bernardino guidelines.

The infiltration tests consisted of drilling eight-inch diameter test holes to the desired depth and installing approximately two (2) inches of gravel in the bottom of the holes. A three-inch diameter perforated PVC pipe, wrapped in a filter sock, was placed in the excavations and the annular space was filled with gravel to prevent caving within the borings. Water was then placed in the borings to presoak the holes and percolation testing was performed following the pre-soak period. Following presoaking, the percolation tests were performed which consisted of adding water to each test hole and measuring the water drop over a 10-minute period. The water drop was recorded and six (6) 10-minute test intervals were completed. Water was added to the test holes after each test interval. The field percolation rates were then converted to an infiltration

rate using the Porchet Method. The infiltration rates calculated using the Porchet Method are presented in the following table:

SUMMARY OF PRELIMINARY INFILTRATION RATES		
Boring	Depth of Test (Feet)	Preliminary Infiltration Rate* (Inches per hour)
I-1	5.0	2.98
I-2	5.0	2.14
I-3	5.0	4.53
I-4	5.0	3.43

*Porchet Method converted infiltration rate from field measured rate.

Copies of the percolation data sheets and the Porchet infiltration rate conversion calculations are presented in Appendix C. No factors of safety were applied to the rates provided. Based on Appendix D of the County of San Bernardino Technical Guidance Document for Water Quality Management Plans (County of San Bernardino, 2013), the factor of safety used to compute the design infiltration rate shall not be less than 2.0, but may be higher at the discretion of the design engineer and acceptance of the plan reviewer. Over the lifetime of the infiltration areas, the infiltration rates may be affected by sediment build up and biological activities, as well as local variations in near surface soil conditions. A suitable factor of safety should be applied to the field rate in designing the infiltration system.

It should be noted that the infiltration rates provided above were performed in relatively undisturbed on-site soils. Infiltration rates will vary and are mostly dependent on the underlying consistency of the site soils and relative density. Infiltration rates may be impacted by weight of equipment travelling over the soils, placement of engineered fill and other various factors. GeoTek assumes no responsibility or liability for the ultimate design or performance of the storm water facility. Stormwater BMP systems should be setback from foundations and improvements as required by regulatory agencies or local jurisdictions.

Representatives of GeoTek should observe the soils exposed at the bottom of the stormwater management facilities during construction/earthwork operations to confirm suitability and that the conditions exposed are as anticipated for the proposed stormwater disposal basin.

3.2 LABORATORY TESTING

Laboratory testing was performed on selected soil samples obtained during the field exploration. The purpose of the laboratory testing was to confirm the field classification of the soils



encountered and to evaluate the physical properties of the soils for use in engineering design and analysis.

Included in the laboratory testing were moisture-density determinations on relatively undisturbed samples. The optimum moisture content-maximum dry density relationships were established for typical soil types so that the relative compaction of the subsoils could be determined. Expansion index testing was performed on selected samples to evaluate the expansion potential of the on-site soils. Direct shear testing was conducted to determine the shear strength parameters of the site soils. Percent passing the #200 sieve testing was performed on representative soil samples to determine the soil classification of site soils. Collapse testing was conducted to evaluate the collapse potential of the onsite soils. Chemical testing comprised of pH, soluble sulfate, chloride and resistivity testing was conducted on selected samples. The moisture-density test data and percent passing the #200 sieve are presented on the exploration boring logs in Appendix A. The maximum density, expansion index, direct shear, collapse, and chemical test data are presented in Appendix B.

4. GEOLOGIC AND SOILS CONDITIONS

4.1 REGIONAL SETTING

The subject property is situated in the Peninsular Ranges geomorphic province. The Peninsular Ranges province is one of the largest geomorphic units in western North America. It extends approximately 975 miles south of the Transverse Ranges geomorphic province to the tip of Baja California. This province varies in width from about 30 to 100 miles. It is bounded on the west by the Pacific Ocean, on the south by the Gulf of California and on the east by the Colorado Desert Province.

The Peninsular Ranges are essentially a series of northwest-southeast oriented fault blocks. Several major fault zones are found in this province. The Elsinore Fault zone and the San Jacinto Fault zone trend northwest-southeast and are found near the middle of the province. The San Andreas Fault zone borders the northeasterly margin of the province.

More specific to the subject property, the site is located in an area geologically mapped to be underlain by alluvial sand and clay of valley areas (Dibblee, T.W. and Minch, J.A., 2004).

4.2 GENERAL SOIL/GEOLOGIC CONDITIONS

A brief description of the lithologic units encountered on the site is presented in the following sections. Based on the field exploration and observations, the area investigated is locally underlain by disturbed soil due to the previous agricultural use of the site, overlying alluvial materials.

4.2.1 Disturbed Soils

Disturbed soils were encountered in all of the exploratory borings performed to depths ranging from approximately 2 to 4 feet below the existing ground surface. These materials were found to generally consist of silty fine to coarse sand to fine to coarse sand (SM and SP soil types based upon the Unified Soil Classification System). These soils were most likely disturbed during the previous agricultural use of the site and the removal of the agricultural orchard in 2003.

Based on the laboratory test results, the near surface soils exhibit a “Very Low” ($0 \leq EI \leq 20$) Expansion Index when tested in accordance with ASTM D 4829. Based on the laboratory test results, the near surface soils have a soluble sulfate content of less than 0.1 percent (ASTM D 4327). The laboratory test results are provided in Appendix B.

4.2.2 Alluvium

Alluvial materials were encountered in all of the borings performed beneath the disturbed soils. The alluvial materials were found to generally consist of interbedded loose to medium dense fine to coarse sand, fine to coarse silty sand, and clayey fine sand with varying amounts of gravel (SP, SM and SC soil types). Although cobbles and boulders were not observed during the drilling, it is anticipated that various amounts of cobbles or boulders are also present in the alluvial materials. Alluvial materials were encountered to the maximum depths explored of approximately 51.5 feet (Boring B-4).

Based on the laboratory test results, the near surface soils exhibit a “Very Low” ($0 \leq EI \leq 20$) Expansion Index when tested in accordance with ASTM D 4829. Based on the laboratory test results, the near surface soils have a soluble sulfate content of less than 0.1 percent (ASTM D 4327). Additionally, based on the laboratory testing, the site soils at a depth of about 7 feet below grades exhibit a “very low” potential for hydroconsolidation (settlement upon wetting with or without additional loading). Furthermore, results of direct shear testing confirmed GeoTek’s observations that the site soils are mostly granular with little cohesion. Such soils are relatively more susceptible to erosion. The laboratory test results are provided in Appendix B.

4.3 SURFACE AND GROUNDWATER

4.3.1 Surface Water

Surface water was not observed during the site exploration. If encountered during earthwork construction, surface water on this site is the result of precipitation or possibly some minor surface run-off from surrounding areas. Overall site drainage is generally in a westernly direction, as directed by site topography. Provisions for surface drainage will need to be accounted for by the project civil engineer.

4.3.2 Groundwater

Groundwater was not encountered to a maximum depth of about 51.5 feet within Boring B-4 at the time of drilling. Based on review of the California Department of Water Resources website (<https://wdl.water.ca.gov/WaterDataLibrary/Map.aspx>), the depth to groundwater recently and historically is greater than 50 feet below the existing ground surface based on somewhat continuous reading between the time period of December 10, 1931 and December 17, 2024 (State Well Number's: 01S03W21H007S and 01S03W17H001S).

Based on the above, groundwater is not anticipated to be a factor during the site grading. However, seasonal perched groundwater may be encountered during grading within portions of the site.

4.4 FAULTING AND SEISMICITY

The geologic structure of the entire southern California area is dominated mainly by northwest-trending faults associated with the San Andreas system. The site is in a seismically active region.

The site is not situated within an "Alquist-Priolo" Earthquake Fault Zone. The site has not been evaluated by the California Geological Survey (CSG) for liquefaction or landslide hazards (Bryant, W.A., and Hart, E.W., 2007). Based on review of San Bernardino County Geologic Hazard Overlay Map FH31 C (San Bernardino County, 2010), the site is not mapped in a zone of suspected liquefaction susceptibility or generalized landslide susceptibility.

4.4.1 Seismic Design Parameters

The site is located at approximately 34.079323 Latitude and -117.197565 Longitude. Based on the average standard penetration test resistance (N-Value) of 15 or greater encountered within Boring B-4, a Site Class "D" is considered appropriate for this project. Site spectral accelerations (S_a and S_1), for 0.2 and 1.0 second periods for a Class "D" site, were determined from the

SEAOC/OSHPD web interface that utilizes the USGS web services and retrieves the seismic design data and presents that information in a report format.

The following seismic design parameters, based on the 2015 National Earthquake Hazards Reduction Program (NEHRP)/ASCE 7-16/2022 CBC, are presented below:

SITE SEISMIC PARAMETERS	
Mapped 0.2 sec Period Spectral Acceleration, S_s	1.938g
Mapped 1.0 sec Period Spectral Acceleration, S_1	0.761g
Site Coefficient for Site Class "D", F_a	1.0
Site Coefficient for Site Class "D", F_v	1.7**
Maximum Considered Earthquake Spectral Response Acceleration for 0.2 Second, S_{MS}	1.938g
Maximum Considered Earthquake Spectral Response Acceleration for 1.0 Second, S_{M1}	1.941g*
5% Damped Design Spectral Response Acceleration Parameter at 0.2 Second, S_{DS}	1.292g
5% Damped Design Spectral Response Acceleration Parameter at 1 second, S_{D1}	1.295g*
Site Modified Peak Ground Acceleration, PGA_M	0.88g
Seismic Design Category	E

*ASCE 7-16 Supplement 3 Section 11.4.8 requires a ground motion hazard analysis for structures on Site Class "D" for values of S_1 greater than or equal to 0.2g. However, a ground motion hazard analysis is not required where the values of S_{M1} and S_{D1} are increased by 50%. The S_{M1} and S_{D1} values shown above already include the 50% increase, so that exception can be obtained.

**ASCE 7-16 Supplement 3 Section 11.4.8 indicates that the value of F_v should only be used for calculation of T_s , determination of Seismic Design Category, linear interpolation for intermediate values of S_1 , and when taking the exceptions under Items 1 and 2 of Section 11.4.8 for the calculation of S_{D1} .

Final selection of the appropriate seismic design coefficients should be made by the project structural engineer based upon the local practices and ordinances, expected building response and desired level of conservatism.

4.5 LIQUEFACTION AND SEISMIC-INDUCED SETTLEMENT POTENTIALS

Liquefaction describes a phenomenon in which cyclic stresses, produced by earthquake-induced ground motion, create excess pore pressures in relatively cohesionless soils. These soils may acquire a high degree of mobility which can lead to lateral movement, sliding, settlement of loose sediments, sand boils and other damaging deformations. This phenomenon occurs only below the water table, but, after liquefaction has developed, the effects can propagate upward into overlying non-saturated soil as excess pore water dissipates.

The factors known to influence liquefaction potential include soil type and grain size, relative density, groundwater level, confining pressures, and both intensity and duration of ground shaking. In general, materials that are susceptible to liquefaction are loose, saturated granular soils having low fines content under low confining pressures.

The project site has not been evaluated by the California Geological Survey (CSG) for liquefaction hazards. Based on review of San Bernardino County Geologic Hazard Overlay Map FH31 C (San Bernardino County, 2010), the site is not mapped in a zone of suspected liquefaction susceptibility.

Based on the relatively dense nature of the subsurface soils encountered, along with the relatively deep groundwater depth, the liquefaction potential at the subject site is considered to be very low.

Loose to medium dense granular soils tend to densify and settle under strong ground shaking. The potential for seismically induced settlement of “dry sands” was assessed using the data of the CPT soundings with the aid of the computer program Cliq Version 3.5.2.5 (Geologismiki, 2006). A mode magnitude weighted (M_w) seismic event of 7.91 obtained using the USGS deaggregation web application <https://earthquake.usgs.gov/hazards/interactive/> (based on a two percent exceedance in 50 years) and a PGA_M value of 0.88g obtained from the website <https://seismicmaps.org/> were used in the assessment.

The results of the analyses indicate the presence of some layers of loose sands and silty sands that would be prone to settlement during a strong earthquake. The following table summarizes the amount of total seismically induced settlement of “dry sands” estimated at each CPT location and includes 5-foot remedial overexcavations and 12-inch scarification of the removal bottoms.

ESTIMATED SEISMICALLY INDUCED TOTAL SETTLEMENT		
CPT Sounding	Exploration Depth (ft)	Total Settlement (in)
CPT-1	60	3.86*
CPT-2	42	1.09*
CPT-3	60	3.85*
CPT-4	60	3.42*

*Settlement based on 5-foot remedial overexcavations and 12 inch scarification of the removal bottom.

As noted above, a maximum seismically induced settlement of 3.86-inches is estimated for this site incorporating the recommended remedial overexcavations. The maximum estimated differential seismic settlement for the site is estimated to be approximately one-half the estimated

total seismic settlement, or approximately 1.93 inches over a horizontal distance of 40 feet. Based upon the results of the seismic settlement calculations, minimal seismic settlements are expected to occur below a depth of approximately 50 feet. The results of the seismic settlement analyses are presented within Appendix D.

4.6 OTHER SEISMIC HAZARDS

Evidence of ancient landslides or slope instability at this site was not observed during the field investigation. Based on review of San Bernardino County Geologic Hazard Overlay Map FH31 C (San Bernardino County, 2010), the site is not mapped in a zone of suspected landslide susceptibility. Due to the relatively flat site topography, the potential for seismic-induced lateral displacements (i.e., “lateral spread”) and slope instability hazards are considered negligible.

The potential for secondary seismic hazards such as a seiche and tsunami is considered negligible due to site elevation and distance from an open body of water.

5. CONCLUSIONS AND RECOMMENDATIONS

5.1 GENERAL

The anticipated site development appears feasible from a geotechnical viewpoint provided that the following recommendations, and those provided by this firm at a later date are incorporated into the design and construction phases of development. Site development, grading and foundation plans should be reviewed by GeoTek, Inc. when they become available so the recommendations contained in this report can be confirmed.

The upper on-site soils exhibit a “Very Low” (0-20) Expansion Index. Expansion Index testing should be conducted at the completion of earthwork operations to verify this design value.

Based on the results of the “dry sand” settlement analyses, a seismically induced settlement of 3.86-inches and a seismically induced differential settlement of 1.93 inches over a horizontal distance of 30 feet is estimated with the recommended remedial overexcavations. Due to the estimated total and differential settlements (Static + Seismic), GeoTek recommends that the site structures be supported by PT slab foundation systems.



The near-surface upper site soils were found to consist of disturbed soils, likely due to the previous agricultural activities onsite. In addition, the upper site soils are anticipated to be further disturbed from the demolition of existing Tennessee Street. To provide a more uniform support for the proposed PT slab foundation systems and to reduce estimated total and differential settlement potentials, it is recommended that remedial grading (over-excavations) be performed. It is recommended that removals be performed to a minimum depth of five (5) feet below existing grades.

5.2 EARTHWORK CONSIDERATIONS

Earthwork and grading should be performed in accordance with the applicable grading ordinances of the City of Redlands, the 2022 California Building Code (CBC) and recommendations contained in this report. The Grading Guidelines included in Appendix E outline general procedures and do not anticipate all site-specific situations. In the event of conflict, the recommendations presented in the text of this report should supersede those contained in Appendix E.

5.2.1 Site Clearing and Demolition

Initial site preparation should commence with removal of debris, pavements, underground utilities, deleterious materials and vegetation within the limits of the planned improvements. In areas of planned grading and improvements, the locations of existing utilities should be determined. Existing utilities should be relocated or abandoned. Debris should be properly disposed of off-site. Voids resulting from site clearing should be backfilled with engineered fill.

Asphaltic concrete (AC) generated from the demolition of existing site improvements may be incorporated into site fills provided the following guidelines are implemented. AC should not be used in fill for building pads or landscape areas. This material may be used in pavement areas of the project or should be removed from the site.

Alternatively, existing AC and aggregate base may be pulverized and reused as Crushed Miscellaneous Base (CMB) in new pavement sections. These pulverized materials should not contain any detrimental or deleterious materials (reinforcing steel, nails, metal, glass, plastic rubber, wood, organic matter, friable materials, elongated or laminated pieces, etc.). Any materials to be used as CMB should be tested to confirm conformance with appropriate Standard Specifications for Public Works Construction ("Greenbook") specifications (Section 200-2.4).

5.2.2 Site Preparation

Due to the previous agricultural use of the project site, the presence of disturbed soils, the anticipated disturbance of the upper site soils due to site demolition operations, and the estimated total settlements (Static + Seismic), it is recommended that the soils be removed beneath the planned building footprints and structural areas (walls) to a depth of at least five (5) feet below existing grades, or three (3) feet below the base of the proposed foundations, whichever is greater. All disturbed soils shall be removed as part of remedial grading. The lateral extent of this recommended over-excavation should extend at least five (5) feet beyond the building limits or at a 1:1 (H:V) projection to the the recommended removal depth, whichever is greater. Removal bottoms should be relatively uniform in soil type and not adversely porous and having an in-place density of at least 85 percent of the soil's maximum dry density as determined by ASTM D 1557 test procedures. Deeper removals may be required and should be determined during site grading by a GeoTek representative.

Cut lots should be overexcavated to a minimum depth of 5 feet below existing grades or 3 feet below the deepest planned foundation, whichever is greater. In addition, the cut portions of transition lots should be excavated to a minimum depth of 5 feet below existing grades, to a minimum depth of 3 feet below the deepest foundation, or to a minimum of one-third of the maximum fill thickness, whichever is greater.

All foundations should be underlain by at least 3 feet of compacted engineered fill.

Following site clearing operations, over-excavation and lowering of site grades, where necessary, it is recommended that the exposed subgrade soils beneath all surface improvements be proof rolled with a heavy rubber-tired piece of construction equipment approved by and in the presence of the geotechnical engineering representative. All soil that ruts or excessively deflects during proof rolling should be removed as recommended by the GeoTek representative.

Following proof rolling and removal of any unsuitable bearing soil, the exposed subgrade should be scarified to a depth of about 12 inches, be moisture conditioned to slightly above the soil's optimum moisture content and then be compacted to at least 90 percent of the soil's maximum dry density as determined by ASTM D-1557 test procedures.

5.2.3 Pavement Areas

Disturbed soils should be removed below proposed pavement areas to a depth of approximately 5 feet. If no disturbed soil is encountered or is relatively shallow, the natural soils should be overexcavated to a depth of 24 inches below existing grade or 24 inches below proposed finished grade, whichever is deeper. Finished grade is defined as the top of the subgrade. The exposed

soils in these areas and in cut areas should be scarified to a depth of approximately twelve inches, moistened to slightly above the optimum moisture content and compacted to a minimum relative compaction of at least 95 percent of maximum dry density as determined by ASTM D 1557 test procedures.

5.2.4 Hardscape Areas

Disturbed should be removed below hardscape areas to a depth of approximately five feet. The exposed soils in these areas and in cut areas should be scarified to a depth of approximately twelve inches, moistened to slightly above the optimum moisture content and compacted to a minimum relative compaction of at least 90 percent of maximum dry density as determined by ASTM D 1557 test procedures.

5.2.5 Preparation of Excavation Bottoms

A representative of this firm should observe the bottom of all excavations. Upon approval, the exposed soils should be scarified to a depth of approximately twelve inches, moistened to slightly above the optimum moisture content and compacted to a minimum relative compaction of at least 90 percent of maximum dry density as determined by ASTM D 1557 test procedures.

5.2.6 Engineered Fill

The on-site soils are generally considered suitable for reuse as engineered fill provided they are free from vegetation, oversize materials, debris and other deleterious material. Engineered fill should be placed in loose lifts with a thickness of eight inches or less and moisture conditioned to at least the optimum moisture content.

The alluvial materials are anticipated to contain oversize materials (i.e., particles greater than six inches maximum size). Special excavation and grading techniques may be required to perform site excavations (including utility lines) and grading. The project budget should include contingencies for possible crushing or removal of the oversized materials.

Below and within the proposed building area, engineered fill should be moisture conditioned to slightly above the optimum moisture content and be compacted to at least 90 percent of maximum dry density as determined by ASTM D 1557 test procedures if a shallow foundation system is used.

Below other structural elements, such as hardscape areas and walls independent of the building, engineered fill should be moisture conditioned to slightly above the optimum moisture content and be compacted to at least 90 percent of maximum dry density as determined by ASTM D 1557 test procedures.

5.2.7 Excavation Characteristics

Excavation of the on-site soils is expected to be feasible utilizing heavy-duty grading equipment in good operating condition. Some excavation difficulties should be anticipated due to the possible presence of cobbles and/or boulders. Oversized rocks (>6 inches) should be anticipated on this site. The alluvial materials are anticipated to contain oversize (i.e., greater than six inches maximum size) materials. Special excavation techniques and/or equipment (screens) may be required to create acceptable fill.

All temporary excavations for grading purposes and installation of underground utilities should be constructed in accordance with local and Cal-OSHA guidelines. Temporary excavations within the on-site materials should be stable at 1.5:1 (horizontal: vertical) inclinations for cuts less than five feet in height.

5.2.8 Trench Excavations and Backfill

Temporary trench excavations within the on-site materials should be stable at 1.5:1 inclination for short durations during construction and where cuts do not exceed 20 feet in height. It is anticipated that temporary cuts to a maximum height of four (4) feet can be excavated vertically, but local sloughing and/or failure could occur due to the granular nature of the soils at this site. Increased caution should be applied when working near or within any excavations at this site. "Popouts" and/or sloughing of oversize cobbles and boulders should be anticipated during utility trench operations.

The alluvial materials are anticipated to contain significant quantities of oversize (i.e., greater than six inches maximum size). Special excavation techniques and/or equipment (screens) may be required to create acceptable fill.

Trench excavations should conform to Cal-OSHA regulations. The contractor should have a competent person, per OSHA requirements, on site during construction to observe conditions and to make the appropriate recommendations.

Utility trench backfill should be compacted to at least 90 percent relative compaction (as determined by ASTM D 1557 test procedures). Under-slab trenches should also be compacted to project specifications. Where applicable, based on jurisdictional requirements, the top 12 inches of backfill below subgrade for road pavements should be compacted to at least 95 percent relative compaction. On-site materials may not be suitable for use as bedding material but should be suitable as backfill provided particles larger than six (6) inches are removed.

Compaction should be achieved with a mechanical compaction device. Ponding or jetting of trench backfill is not recommended. If backfill soils have dried out, they should be properly moisture conditioned prior to placement in trenches.

5.2.9 Shrinkage and Subsidence

Several factors will impact earthwork balancing on the site, including shrinkage, subsidence, removal of oversize material, trench spoil from utilities and footing excavations, as well as the accuracy of topography. Shrinkage and subsidence are primarily dependent upon the degree of compactive effort achieved during construction. For planning purposes, a shrinkage factor of 5 to 10 percent may be considered for the materials requiring removal and/or recompaction. Disturbed soils could shrink more. Site balance areas should be available in order to adjust project grades, depending on actual field conditions at the conclusion of earthwork. Subsidence on the order of up to 0.2 foot may be anticipated for the underlying soils. Additional loss should be anticipated for removal and offsite disposal of oversized materials.

5.2.10 Import Soils

Import soils should have a “Very Low” (0-20) Expansion Index. GeoTek, Inc. also recommends that the proposed import soils be tested for expansion and corrosivity potential. GeoTek, Inc. should be notified a minimum of 72 hours prior to importing so that appropriate sampling and laboratory testing can be performed.

5.3 DESIGN RECOMMENDATIONS

5.3.1 Foundation Design

The soils are classified as having a “Very Low” ($0 \leq EI \leq 20$) Expansion Index in accordance with ASTM D 4829. Since the California Building Code (CBC) indicates Post-Tensioning Institute (PTI) design methodology is intended for expansive soils conditions, which do not apply to this project, no e_m or y_m parameters as used in the PTI methodology are provided.

The foundation elements for the proposed structures should bear entirely in engineered fill soils and should be designed in accordance with the 2022 California Building Code (CBC). The following design recommendations for post-tensioned (PT) foundations for this project are provided:

MINIMUM DESIGN REQUIREMENTS FOR POST-TENSIONED FOUNDATIONS	
Foundation Design Parameter	"Very Low" Expansion Index (ASTM D 4829)
Foundation Depth or Minimum Perimeter Beam Depth/Turned Down Edge (inches below lowest adjacent grade)	One and Two-Stories – 12 inches
Minimum Beam/Wall Foundation Width	One and Two-Stories – 12 inches*
Minimum Slab Thickness (actual)	4.5 inches
Presaturation of Subgrade Soil (Percent of Optimum)	Minimum 100% of the optimum moisture content to a depth of at least 12 inches prior to placing concrete

*Code minimums per Table 1809.7 of the 2022 CBC should be complied with.

An allowable bearing capacity of 2,000 pounds per square foot (psf) may be used for design of stiffening beams and/or perimeter beams 12 inches deep and 12 inches wide and pad footings 24 inches square and 12 inches deep. This allowable soil bearing capacity may be increased by 500 psf for each additional 12 inches of embedment depth and by 200 psf for each additional 12 inches in width to a maximum of 3,000 psf. These allowable bearing capacities utilize a minimum factor of safety of 3. The allowable bearing capacity may also be increased by one-third when considering short-term wind and seismic loads.

The recommended allowable bearing capacity is based on an estimated maximum post-construction static settlement of 3/4-inch. Static differential settlement of about one-half of the total static settlement over a horizontal distance of 40 feet could result. Estimated total seismic settlements of up to approximately 3.86 inches is estimated for this site. The maximum seismic differential settlement for the site is estimated to be approximately one-half the estimated total seismic settlement, or approximately 1.93 inches over a horizontal distance of 40 feet. The estimated combined (static + seismic) differential settlement of 2.3 inches over a horizontal distance of 40 feet is anticipated for the project site. The project structural engineer, foundation engineer, and earth retention structure designer should incorporate these settlement estimates into the design, as appropriate.

The passive earth pressure may be computed as an equivalent fluid having a density of 245 psf per foot of depth, to a maximum earth pressure of 3,675 psf for footings founded on engineered fill or competent native soil. These passive earth pressures include a minimum factor of safety of 1.5. A coefficient of friction between soil and concrete of 0.35 may be used with dead load forces. Passive pressure and frictional resistance may be combined without reduction. The upper one foot of soil should be ignored in the passive pressure calculations unless the surface is covered with pavements.

5.3.2 Moisture Vapor and Retarding System

A moisture and vapor retarding system should be placed below slabs-on-grade where moisture migration through the slab is undesirable. Guidelines for these are provided in the 2022 California Green Building Standards Code (CALGreen) Section 4.505.2, the 2022 CBC Section 1907.1 and ACI 360R-10. The vapor retarder design and construction should also meet the requirements of ASTM E 1643. A portion of the vapor retarder design should be the implementation of a moisture vapor retardant membrane.

The effectiveness of the vapor retarding membrane can be adversely impacted as a result of construction related punctures. These occurrences should be limited as much as possible during construction. Thicker membranes are generally more resistant to accidental puncture than thinner ones. Products specifically designed for use as moisture/vapor retarders may also be more puncture resistant. Although the CBC specifies a 6-mil vapor retarder membrane, a minimum 10-mil thick membrane with joints properly overlapped and sealed should be considered, unless otherwise specified by the slab design professional. The membrane should consist of Stego wrap or the equivalent.

Moisture and vapor retarding systems are intended to provide a certain level of resistance to vapor and moisture transmission through the concrete, but do not eliminate it. The acceptable level of moisture transmission through the slab is to a large extent based on the type of flooring used and environmental conditions. Ultimately, the vapor retarding system should be comprised of suitable elements to limit migration of water and reduce transmission of water vapor through the slab to acceptable levels. The selected elements should have suitable properties such as thickness, composition, strength, and permeability to achieve the desired performance level. Moisture retarders can reduce, but not eliminate, moisture vapor rise from the underlying soils up through the slab. Moisture retarder systems should be designed and constructed in accordance with applicable American Concrete Institute, Portland Cement Association, Post-Tensioning Concrete Institute, ASTM and California Building Code requirements and guidelines.

GeoTek recommends that a qualified person, such as a flooring contractor, structural engineer, architect, and/or other experts specializing in moisture control within buildings be consulted to evaluate the general and specific moisture and vapor transmission paths and associated potential impact on the proposed construction. That person should provide recommendations relative to the slab moisture and vapor retarder systems and for migration of potential adverse impact of moisture vapor transmission on various components of the structures, as deemed appropriate.

In addition, the recommendations in this report and GeoTek's services in general are not intended to address mold prevention, since GeoTek, along with geotechnical consultants in

general, do not practice in the area of mold prevention. If specific recommendations addressing potential mold issues are desired, then a professional mold prevention consultant should be contacted.

5.3.3 Miscellaneous Foundation Recommendations

To minimize moisture penetration beneath the slab-on-grade areas, utility trenches should be backfilled with engineered fill, lean concrete or concrete slurry where they intercept the perimeter footing or thickened slab edge.

Soils from the footing excavations should not be placed in the slab-on-grade areas unless properly compacted and tested. The excavations should be free of loose/sloughed materials and be neatly trimmed at the time of concrete placement.

5.3.4 Foundation Setbacks

Minimum setbacks for all foundations should comply with the 2022 CBC or City of Redlands requirements, whichever is more stringent. Improvements not conforming to these setbacks are subject to the increased likelihood of excessive lateral movement and/or differential settlement. If large enough, these movements can compromise the integrity of the improvements.

- The outside top edge of all footings should be set back a minimum of $H/3$, where H is the slope height, from the face of any descending slope. The setback should be at least five feet and need not exceed 40 feet.
- The bottom of any proposed foundations should be deepened so as to extend below a 1:1 upward projection from the bottom edge of the nearest excavation and the bottom edge of the closest footing.

5.3.5 Soil Corrosivity

Based on the chemical test results presented in Appendix B, the corrosivity test results indicate that the on-site soils are “mildly corrosive” to “moderately corrosive” (8,040 to 14,740 ohm-cm) to buried ferrous metal. This corrosion classification is obtained from “Corrosion Basics: An Introduction,” by Pierre R. Roberge, 2nd Edition, 2005. Recommendations for protection of buried ferrous metal should be provided by a corrosion engineer. Laboratory testing should be performed at the completion of site grading to verify the soil corrosivity of the near-surface soils.

5.3.6 Soil Sulfate Content

Based on the chemical test results presented in Appendix B, the sulfate test results on samples obtained from the project site indicate a soluble sulfate content of less than 0.1% by weight. Soluble sulfate contents of this level would be in the range of “negligible” (i.e., “S0” exposure classification) in accordance with ACI 318-19. Based on the test results and ACI 318-19, no

special recommendations for concrete are required for this project due to soil sulfate exposure. Laboratory testing should be performed at the completion of site grading to verify the soil sulfate content of the near-surface soils. Additional sulfate testing should be performed at the time of grading to assess sulfate levels for the as-graded soils at finish grade elevations for the building pads.

5.4 RETAINING AND SCREEN WALL DESIGN AND CONSTRUCTION

5.4.1 General Design Criteria

Retaining wall foundations independent of the building should be embedded a minimum of 12 inches into engineered fill and underlain by a minimum of 2 feet of compacted engineered fill. Retaining wall foundations should be designed in accordance with Section 5.3.1 of this report. Structural requirements may govern and should be evaluated by the project structural engineer. All earth retention structure plans should be reviewed by this office prior to finalization.

Site clearing and remedial earthwork for all earth retention structures should meet the requirements of this report, unless specifically provided otherwise, or more stringent requirements or recommendations are made by the wall designer. The soil used as backfill behind retaining walls should have a “Very Low” (0-20) Expansion Index and should be compacted to at least 90 percent relative compaction (ASTM D 1557).

In general, cantilever retaining walls which are designed to yield at least $0.001H$, where H is equal to the height of the wall to the base of the footing may be designed using the active condition. Rigid earth retention structures (including but not limited to rigid walls, and walls braced at the top, such as typical basement walls) should be designed using the at-rest condition.

In addition to the design lateral forces due to retained earth, surcharges due to improvements, such as an adjacent structure, should be considered in the design of the retaining walls. Loads applied within a 1:1 (h:v) projection from the surcharging structure on the stem of the retaining wall should be considered in the design.

Final selection of the appropriate design parameters should be made by the retaining wall designer based upon the local practices and ordinances, expected structure response, and desired level of conservatism.

5.4.2 Cantilevered Walls

The recommendations presented below are for cantilevered retaining walls. Active earth pressure may be used for retaining wall design, provided the top of the wall is not restrained from minor deflections. Appropriate fluid unit weights are given below for specific slope gradients of the retained material. These do not include other superimposed loading conditions such as traffic, structures, seismic events, or adverse geologic conditions.

ACTIVE EARTH PRESSURES	
Surface Slope of Retained Materials (h:v)	Equivalent Fluid Pressure* (pcf)
Level	40
2:1	61

*The design pressures assume the backfill materials have an expansion index less than or equal to 20 and minimum internal friction angle of 31° . Backfill zone includes the area between the back of the wall to a plane (1:1, h:v) up from the bottom of the wall foundation to the adjacent ground surface.

For walls with a retained height greater than six (6) feet, an incremental seismic pressure should be included into the wall design. Where needed, it is recommended that an incremental seismic load for unrestrained walls with level backfill of $13H^2$ [Units: pounds per lineal foot of wall] should be included into the wall design to account for seismic loading conditions, where H is the retained height of the wall. For unrestrained walls with a retained height greater than six (6) feet with backfill of a 2:1 [horizontal: vertical] gradient, a dynamic load increment of $22H^2$ should be included in the wall design. These incremental seismic loads may be assumed to be applied at a point $1/3H$ above the base of the wall.

5.4.3 Retaining Wall Backfill and Drainage

Wall backfill should include a minimum one foot wide section of $3/4$ - to one-inch clean crushed rock or approved equivalent. The rock should be placed immediately adjacent to the back of the wall and extend up from a backdrain to within approximately 12 inches of finish grade. The portion of the rock opposite the back of the wall adjacent to the soil backfill should be covered with a layer of filter fabric comprised of Mirafi 140N or the equivalent. The upper 12 inches of backfill should consist of compacted on-site soil. Backfill placed within the active zone as defined by a 1:1 (H:V) projection from the back of the retaining wall footing up to the retained surface behind the wall should consist of very low expansive soil. The presence of other soils placed within the 1:1 projection will necessitate revision to the parameters provided and modification of wall designs.

The backfill soil should be placed in lifts no greater than eight inches in thickness, moisture conditioned to slightly above the optimum moisture content and compacted to at least 90 percent of maximum dry density as determined by ASTM D 1557 test procedures. Proper surface drainage needs to be provided and maintained. Water should not be allowed to pond behind retaining walls. Waterproofing of site walls should be performed where moisture migration through the walls is undesirable.

Retaining walls should be provided with an adequate pipe and gravel back drain system to reduce the potential for hydrostatic pressures to develop. A 4-inch diameter perforated collector pipe (Schedule 40 PVC or approved equivalent) in a minimum of one cubic foot per linear foot of $\frac{3}{4}$ -inch or one inch clean crushed rock or equivalent, wrapped in filter fabric should be placed near the bottom of the backfill and the water should be directed to an appropriate disposal area.

As an alternative to the drain, rock and fabric, a pre-manufactured wall drainage product (example: Mira Drain 6000 or approved equivalent) may be used behind the retaining wall. The wall drainage product should extend from the base of the wall to within two (2) feet of the ground surface. The subdrain should be placed in direct contact with the wall drainage product.

Walls less than two (2) feet in height do not require backdrains. Walls from two to four feet in height may be drained using localized gravel packs (e.g., approximately 1.5 cubic feet of gravel in a woven plastic bag) behind weep holes. Weep holes should be provided at 5 feet maximum spacing or the head joints omitted in the first course of block extended above the ground surface. However, nuisance water may still collect in front of the wall; therefore, the ground adjacent to the wall face must be graded to maintain positive drainage away from the wall.

Drain outlets should be maintained over the life of the project and should not be obstructed or plugged by adjacent improvements.

5.4.4 Restrained Retaining Walls

Retaining walls that will be restrained prior to placing and compacting backfill material or that have reentrant or male corners, should be designed for an at-rest equivalent fluid pressure of 60 pcf, plus any applicable surcharge loading. For areas of male or reentrant corners, the restrained wall design should extend a minimum distance of twice the height of the wall laterally from the corner, or a distance otherwise determined by the project structural engineer.

For walls with a retained height greater than six (6) feet, an incremental seismic pressure should be included into the wall design. For restrained walls with a retained height greater than six (6) feet with level backfill, a dynamic load increment of $21H^2$ should be included in the wall design.

These incremental seismic loads may be assumed to be applied at a point $1/3H$ above the base of the wall.

5.4.5 Additional Retaining Wall Design Considerations

- Retaining and screen wall foundation elements should be designed in accordance with building code setback requirements. A minimum horizontal setback distance of five feet as measured from the top outside edge of the footing to an adjacent slope face is recommended.
- Wall design should consider the additional surcharge loads from superjacent slopes and/or footings, where appropriate.
- No backfill should be placed against concrete until minimum design strengths are evident by compression tests of cylinders.
- The retaining wall footing excavations, backcuts, and backfill materials should be approved by the project geotechnical engineer or their authorized representative.
- Positive separations should be provided in screen walls at horizontal distances not exceeding 20 feet.

5.5 CONCRETE FLATWORK

5.5.1 Exterior Concrete Slabs, Sidewalks and Driveways

Exterior concrete slabs, sidewalks and driveways should be designed using a four-inch minimum thickness. Some shrinkage and cracking of the concrete should be anticipated as a result of typical mix designs and curing practices utilized in construction.

Sidewalks may be under the jurisdiction of the governing agency. If so, jurisdictional design and construction criteria will apply, if more restrictive than the recommendations presented in this report.

Subgrade soils should be pre-moistened prior to placing concrete. The subgrade soils below exterior slabs and sidewalks should be pre-saturated to a minimum of 100 percent of the optimum moisture content to a depth of 12 inches.

All concrete installation, including preparation and compaction of subgrade, should be done in accordance with the City of Redlands specifications, and under the observation and testing of GeoTek, Inc. and a City inspector, if necessary.

5.5.2 Concrete Performance

Concrete cracks should be expected. These cracks can vary from sizes that are hairline to more than 1/8 inch in width. Most cracks in concrete, while unsightly, do not significantly impact long-term performance. While it is possible to take measures (proper concrete mix, placement, curing, control joints, etc.) to reduce the extent and size of cracks that occur, some cracking will occur despite the best efforts to minimize it. Concrete undergoes chemical processes that are dependent on a wide range of variables which are difficult, at best, to control. Concrete, while seemingly a stable material, is subject to internal expansion and contraction due to external changes over time.

One of the simplest means to control cracking is to provide weakened control joints for cracking to occur along. These do not prevent cracks from developing; they simply provide a relief point for the stresses that develop. These joints are a widely accepted means to control cracks but are not always effective. Control joints are more effective the more closely spaced they are. GeoTek, Inc. suggests that control joints be placed in two directions and located a distance approximately equal to 24 to 36 times the slab thickness.

5.6 PRELIMINARY PAVEMENT DESIGN

5.6.1 Asphaltic Concrete Pavement

Although planned final grades beneath the proposed streets and drive aisles within the site are not yet known, the following preliminary pavement design recommendations are based on an assumed Traffic Index of 6.0 and 8.0.

Preliminary pavement thickness design is based on the Caltrans Highway Design Manual (2018). An R-value of 40 was assumed for the determination of preliminary pavement sections for this report. Once the traffic loading information becomes more defined, revision to the pavement design recommendations may be warranted. It is recommended that the final pavement design be based on R-value testing of the as-graded subgrade soils within the pavement areas.

Based on the assumptions noted above, the following preliminary pavement section recommendations are provided for the site:



PRELIMINARY MINIMUM PAVEMENT SECTION		
Traffic Index	Thickness of Asphalt Concrete (inches)	Thickness of Aggregate Base (inches)
6.0 (Interior Streets and Drive Aisles)	4.0	5.0
8.0 (Tennessee Street)	6.0/5.0	6.0/8.0

*Minimum thickness of AC and AB per City of Redlands

Traffic Indices (TIs) used in the pavement design are designated by the City of Redlands and should provide a pavement life of approximately 20 years with a normal amount of flexible pavement maintenance. Irrigation adjacent to pavements, without a deep curb or other cutoff to separate landscaping from the paving may result in premature pavement failure. Traffic parameters used for design were selected based upon engineering judgment and not upon information furnished to us such as an equivalent wheel load analysis or a traffic study.

All base material and the upper 12 inches of subgrade should be compacted to at least 95 percent of the material's maximum dry density as determined by ASTM D1557 test procedures. All materials and methods of construction should conform to the requirements of the City of Redlands.

5.6.2 Permeable Interlocking Concrete Pavers

Interlocking concrete pavers can be used for this project. The pavers are assumed to be approximately 3.2 inches (80 millimeters) in thickness. Concrete pavers should be underlain by 1.0 inch to 1.5 inches of bedding sand overlying four inches of aggregate base founded on compacted subgrade soils. The aggregate base should be compacted to at least 95 percent of maximum dry density as determined by ASTM D 1557 test procedures.

Where possible, the aggregate base should extend beyond the perimeter of the pavers a minimum distance of four inches.

The bedding sand should be placed and lightly moistened and compacted. Since this compaction cannot be tested it should be observed by a representative of this firm.

Historically, paver systems have experienced failures in areas where water has degraded the support characteristics of the underlying base and/or subgrade soils. Since paver systems are permeable and allow transmission of water into the underlying materials, it may be prudent to discuss with the paver designer/manufacturer what methods may be employed to address the

issue of potential water introduction into the underlying materials. Underdrain systems, local subgrade reinforcement, tilting the pavement subgrade to drain away from paver areas and not pond, or additional structural elements such as geotextiles can be considered, particularly in high traffic areas and/or low areas where water will tend to collect. The recommendations of the designer/manufacture should then be implemented in the design and construction of the paver system.

5.6.3 Portland Cement Concrete (PCC) Pavement

For vehicle service and access lanes, it is recommended that a minimum of six (6.0) inches of PCC pavement over 12 inches subgrade compacted to at least 95 percent of maximum dry density be utilized. This section should also be used in heavy truck traffic areas such as fire lanes and approaches.

Requirements of Section 90 of Caltrans Standard Specifications, and various ACI and ASTM standards regarding mixing and placing concrete should be followed. The PCC pavement should have a minimum modulus of rupture of 600 pounds per square inch, and a minimum 28-day compressive strength of 4,000 pounds per square inch. Concrete should incorporate 1-inch maximum size aggregate and should be proportioned to achieve a maximum slump of four inches. Instead of increasing the water content, a plasticizing admixture may be utilized to increase the workability of the concrete. The concrete should be properly cured after placement. Concrete should not be placed during hot and windy weather.

Crack control joints should be provided in the transverse direction spaced at horizontal intervals ranging from 24 to 36 times the thickness of the concrete.

5.6.4 Pavement Construction

All pavement installation, including preparation and compaction of subgrade and base material, placement and rolling of asphaltic concrete and placement of concrete pavement, should be done in accordance with the City of Redlands/County of San Bernadino guidelines, and under the observation and testing of GeoTek and a City inspector, where required.

Any aggregate base used should consist of crushed rock with an R-Value and gradation in accordance with Crushed Aggregate Base (Section 400-2.4 of the "Greenbook" Regional Supplement Amendments). Any Crushed Miscellaneous Base used shall conform to Section 200-2.4 of the Green Book.

Minimum compaction requirements for aggregate base should be 95 percent of maximum dry density as determined by ASTM D 1557 test procedures for both soil subgrade and aggregate base. Jurisdictional minimum compaction requirements in excess of the aforementioned minimums may govern. The upper 12 inches of subgrade should be moisture-conditioned to at least optimum moisture. The top of the subgrade and aggregate base should be graded to drain to the perimeter of the pavement.

5.7 POST CONSTRUCTION CONSIDERATIONS

5.7.1 Landscape Maintenance and Planting

Water has been shown to weaken the inherent strength of soil, and slope stability is significantly reduced by overly wet conditions. Positive surface drainage away from graded slopes should be maintained and only the amount of irrigation necessary to sustain plant life should be provided for planted slopes. Controlling surface drainage and runoff and maintaining a suitable vegetation cover can minimize erosion. Plants selected for landscaping should be lightweight, deep-rooted types that require little water and are capable of surviving the prevailing climate.

Overwatering should be avoided. An abatement program to control ground-burrowing rodents should be implemented and maintained. Burrowing rodents can decrease the long-term performance of slopes.

It is common for planting to be placed adjacent to structures in planter or lawn areas. This will result in the introduction of water into the ground adjacent to the foundations. This type of landscaping should be avoided.

5.7.2 Drainage

Positive site drainage should be maintained at all times. Drainage should not flow uncontrolled down any descending slope. Water should be directed away from foundations and not allowed to pond or seep into the ground adjacent to the footings and floor-slabs. Pad drainage should be directed toward approved areas and not be blocked by other improvements. Roof gutters should be installed that will direct the collected water at least 20 feet from the buildings.

5.8 PLAN REVIEW AND CONSTRUCTION OBSERVATIONS

It is recommended that specifications and foundation and grading plans be reviewed by GeoTek prior to construction to check for conformance with the recommendations of this report. It is also recommended that GeoTek, Inc. representatives be present during site grading and



foundation construction to observe and document proper implementation of the geotechnical recommendations. The owner/developer should verify that GeoTek, Inc. representatives perform at least the following duties:

- Observe site clearing and grubbing operations for proper removal of unsuitable materials.
- Observe and test bottom of removals prior to fill placement.
- Evaluate the suitability of on-site and import materials for fill placement and collect soil samples for laboratory testing where necessary.
- Observe the fill for uniformity during placement, including utility trench backfill. Perform field density testing of the fill materials.
- Observe and probe foundation excavations to confirm suitability of bearing materials with respect to density.

If requested, a construction observation and compaction report can be provided by GeoTek, Inc., which can comply with the requirements of the governmental agencies having jurisdiction over the project. It is recommended that these agencies be notified prior to commencement of construction so that necessary grading permits can be obtained.

6. INTENT

It is the intent of this report to aid in the design and construction of the proposed development. Implementation of the advice presented in this report is intended to reduce risk associated with construction projects. The professional opinions and geotechnical advice contained in this report are not intended to imply total performance of the project or guarantee that unusual or variable conditions will not be discovered during or after construction.

The scope of this evaluation is limited to the boundaries of the subject property. This review does not and should in no way be construed to encompass any areas beyond the specific area of the proposed construction as indicated to us by the client. Further, no evaluation of any existing site improvements is included. The scope of this evaluation is based on GeoTek's understanding of the project and geotechnical engineering standards normally used on similar projects in this locality.

7. LIMITATIONS

GeoTek's findings are based on site conditions observed and the stated sources. Thus, GeoTek's comments are professional opinions that are limited to the extent of the available data.

GeoTek has prepared this report in a manner consistent with that level of care and skill ordinarily exercised by members of the engineering and science professions currently practicing under similar conditions in the jurisdiction in which the services are provided, subject to the time limits and physical constraints applicable to this report.

Since GeoTek's recommendations are based on the site conditions observed and encountered, and laboratory testing, GeoTek's conclusions and recommendations are professional opinions that are limited to the extent of the available data. Observations during construction are important to allow for any change in recommendations found to be warranted. These opinions have been derived in accordance with current standards of practice and no warranty of any kind is expressed or implied. Standards of care/practice are subject to change with time.

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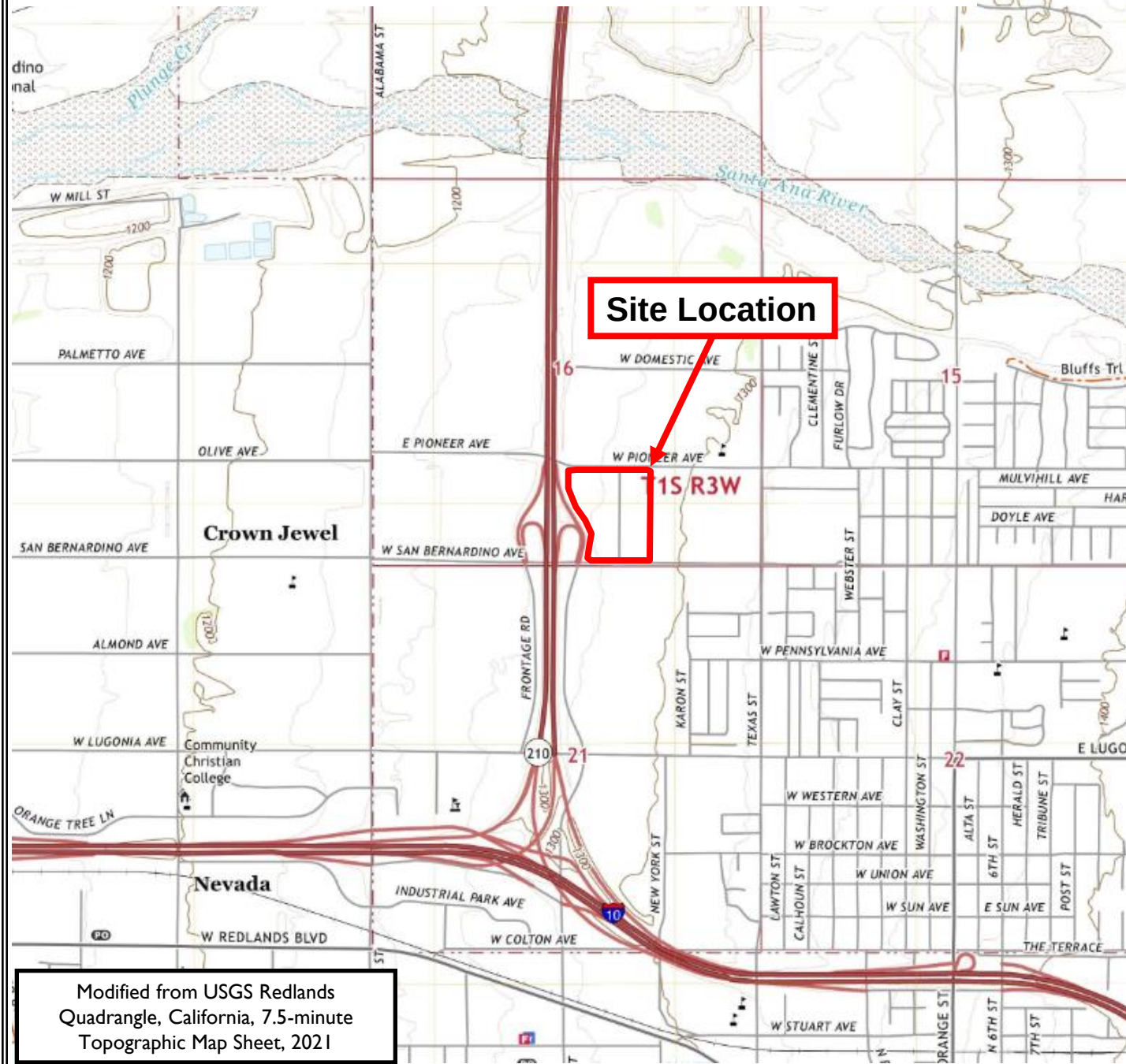
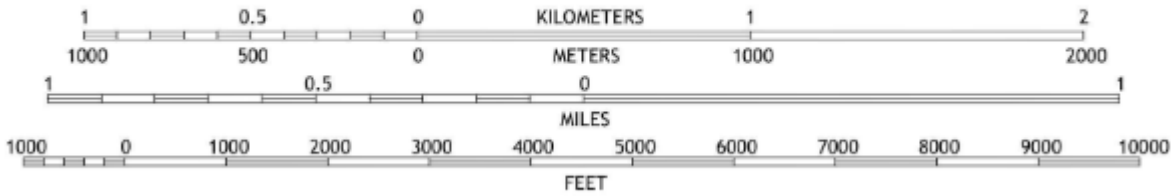
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SCALE 1:24 000



Modified from USGS Redlands
Quadrangle, California, 7.5-minute
Topographic Map Sheet, 2021

Redlands Pioneer Partners, LLC
Proposed Residential Development
APN's: 167-091-09 through -12
Northeast of the 210 FWY and West San Bernardino Avenue
Redlands, San Bernardino County, California
Project No. 4212-CR

Figure I

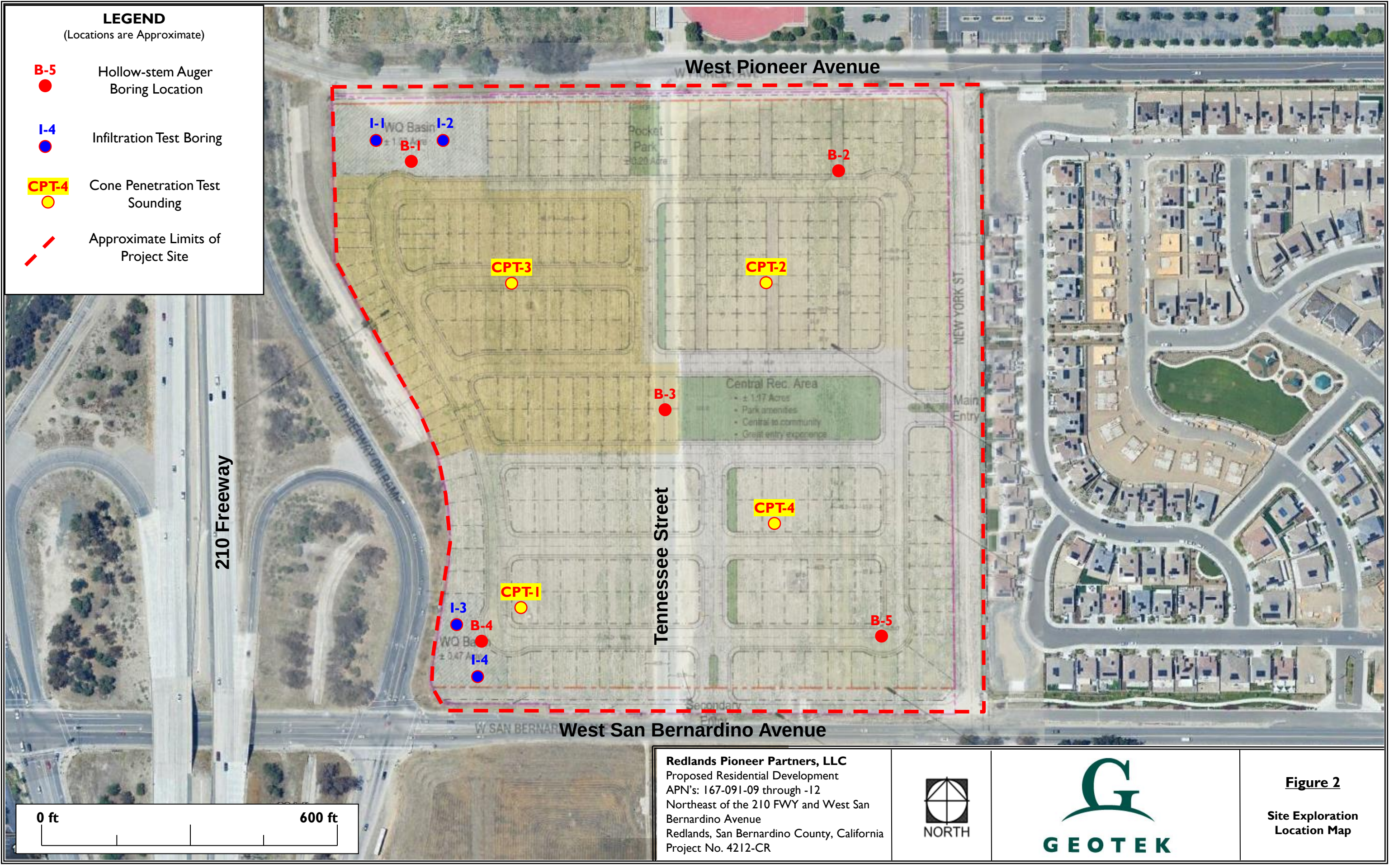
**Site Location and
Topography
Map**



LEGEND

(Locations are Approximate)

- B-5** Hollow-stem Auger Boring Location
- I-4** Infiltration Test Boring
- CPT-4** Cone Penetration Test Sounding
- Approximate Limits of Project Site



Redlands Pioneer Partners, LLC
Proposed Residential Development
APN's: 167-091-09 through -12
Northeast of the 210 FWY and West San Bernardino Avenue
Redlands, San Bernardino County, California
Project No. 4212-CR



Figure 2
Site Exploration Location Map

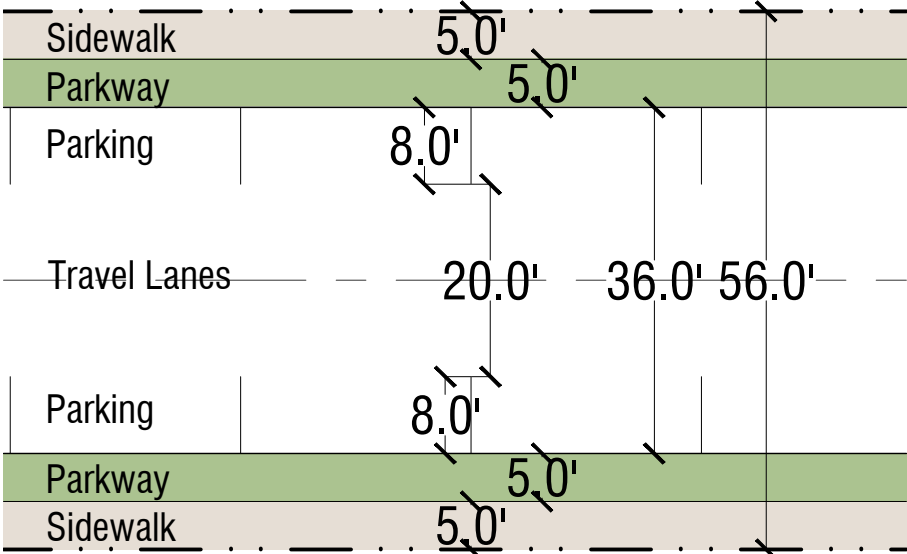
Assessor's Parcel Numbers

016709109
016709110
016709111
016709112

Design Objectives

- Create a community with strong sense of arrival by aligning main and secondary entries to come in at the central park
- Ensure pedestrian friendly, walkable streets with curb separated sidewalks, paseos and desirable block lengths
- Design variety of homes around the central amenity area
- Align homes backing towards the channel and on ramp with front doors on internal streets

typical
2-Story Duplex
±94 Units | ±7.16 Acres | 13.1 du/ac
• ± 1,670-1,770 SF
• 3-4 bed/ 2.5 bath
• 2 car side-by-side garage
• No Driveway



Typical Street - 56' R.O.W.

- Notes:
1. Site plan is for conceptual purposes only.
 2. The size and location of retention basin to be determined and may affect yield.
 3. Site plan must be reviewed by planning, building, and fire departments for code compliance.
 4. Civil engineer to verify all setbacks and grading information.
 5. Building Footprints might change due to the final design elevation style.
 6. Open space area is subject to change due to the balcony design of the elevation.
 7. Building setbacks are measured from property lines to building foundation lines.

REDLANDS PIONEER
PARTNERS LLC



Project Summary

Total Site Area: ± 31.73 Acres Gross
± 30.10 Acres Net (Excl. Public Right-of-ways)

- Total Units: 289 2-Story Homes
- (88) 46' X 62' Small Lot Conventional SFD (2-car Garage)
 - 1,850-2,150 SF; 3-5 bed/ 3 bath
 - (107) Small Lot Court SFD (2-car Garage)
 - (53) Plan 1: 34' X 59' Lot; 1,650 SF; 3 bed+loft/ 2.5 bath
 - (27) Plan 2: 38' X 59' Lot; 1,805 SF; 4 bed/ 2.5 bath
 - (27) Plan 3: 38' X 59' Lot; 1,905 SF; 4 bed+loft/ 3 bath
 - (94) Duplex (2-car Garage)
 - (31) Plan 1: 30' X 60' Lot; 1,670 SF; 3 bed+loft/ 2.5 bath
 - (32) Plan 2: 30' X 60' Lot; 1,710 SF; 4 bed/ 2.5 bath
 - (31) Plan 3: 30' X 60' Lot; 1,770 SF; 4 bed+tech/ 2.5 bath

Density: ±9.6 Homes per Acre Net

- Parking: 813 Spaces (2.8 Spaces per Unit)
- 578 Garage Spaces (2-car Garage per Unit)
 - 88 Driveway Spaces (1 Space per Driveway for Conv. SFD)
 - 147 Parallel Spaces

- Open Space: ± 1.58 Acres
- 1.17 Acres Central Rec. Area
 - 0.21 Acre Green Belt
 - 0.20 Acre Pocket Park

Retention: ±1.52 Acres

- typical
2-Story Small Lot Court SFD
±107 Units | ±8.47 Acres | 12.6 du/ac
- ± 1,650-1,905 SF
 - 3-4 bed/ 2.5-3 bath
 - 2 car side-by-side garage
 - No Driveway

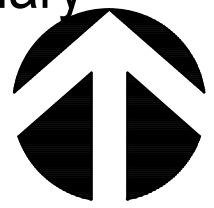
- typical
2-Story Small Lot Conventional SFD
±88 Units | ±9.72 Acres | 9.1 du/ac
- ±1,850-2,150 SF
 - 3-5 bed/ 3 bath
 - 2 car side-by-side garage
 - Full Length Driveway

CONCEPTUAL SITE PLAN
PIONEER AND TENNESSEE SITE

REDLANDS, CA

Gross Area Boundary
± 31.73 Acres

Net Area Boundary
± 30.10 Acres



0 40 80 160

PRELIMINARY REVIEW APPLICATION

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Figure 3: Project Plan

APPENDIX A

LOGS OF EXPLORATORY BORINGS AND CONE PENETRATION TEST SOUNDINGS

Geotechnical and Infiltration Evaluation

Proposed Residential Development

APNs 167-091-09 through -12

Northeast of the 210 Freeway and West San Bernardino Avenue

Redlands, San Bernardino County, California

Project No. 4212-CR



A - FIELD TESTING AND SAMPLING PROCEDURES

The Modified Split-Barrel Sampler (Ring)

The Ring sampler is driven into the ground at various depths in accordance with ASTM D 3550 test procedures. The sampler, with an external diameter of 3.0 inches, is lined with 1-inch long, thin brass rings with inside diameters of approximately 2.4 inches. The sampler is typically driven into the ground 12 or 18 inches with a 140-pound hammer free falling from a height of 30 inches. Blow counts are recorded for every 6 inches of penetration as indicated on the log of boring. The samples are removed from the sample barrel in the brass rings, sealed, and transported to the laboratory for testing.

The Standard Penetration Test (SPT) Sampler

Standard penetration tests (SPT) were performed with a 2.0-inch outside diameter, 1.5-inch inside diameter, split-barrel sampler. The sampler was 18 inches long. The inside diameter of the sampler shoe was 1.4 inches. The sampler was unlined. The sampler conformed to the requirements of ASTM D 1586. A 140-pound automatic trip hammer was utilized, dropping 30 inches for each blow. Blow counts are recorded for every 6 inches of penetration as indicated on the log of boring. Disturbed samples are removed from the sample barrel, sealed in a plastic bag, and transported to the laboratory for testing.

Bulk Samples (Large)

These samples are normally large bags of earth materials over 20 pounds in weight collected from the field by means of hand digging or exploratory cuttings.

Bulk Samples (Small)

These are plastic bag samples which are normally airtight and contain less than 5 pounds in weight of earth materials collected from the field by means of hand digging or exploratory cuttings. These samples are primarily used for determining natural moisture content and classification indices.

B – BORING/TRENCH LOG LEGEND

The following abbreviations and symbols often appear in the classification and description of soil and rock on the logs of borings/trenches:

SOILS

USCS	Unified Soil Classification System
f-c	Fine to coarse
f-m	Fine to medium

GEOLOGIC

B: Attitudes	Bedding: strike/dip
J: Attitudes	Joint: strike/dip
C: Contact line	
.....	Dashed line denotes USCS material change
————	Solid Line denotes unit / formational change
————	Thick solid line denotes end of boring/trench

(Additional denotations and symbols are provided on the log of borings/trenches)



CLIENT:	Redlands Pioneer Partners	DRILLER:	2R Drilling	LOGGED BY:	Lyn
PROJECT NAME:	210 FWY & San Bernardino Ave	DRILL METHOD:	Hollow Stem	OPERATOR:	Jerry
PROJECT NO.:	4212-CR	HAMMER:	140#/30"	RIG TYPE:	CME 55 track rig
COORDINATES:	See Boring Location Map	ELEVATION:	1269'	DATE:	2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-I	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0				SM	Disturbed Soil: Silty f-c SAND, light brown, slightly moist, trace f gravel, trace organics			
		4	R1	SP	F-c SAND, medium brown, slightly moist, loose, trace silt	4.3	110.1	
		4						
		4						
5		3	R2	SP/SM	Alluvium: F-m SAND to silty f-m SAND, light brown, slightly moist, loose, trace silt, trace f gravel	8.2	106.5	
		4						
		4						
		6						
		6	R3	SP	F SAND, light brown, slightly moist, medium dense	6.4	99.6	
		9						
		5	R4		F-m SAND, medium dense	3.1	105.2	
10		9						
		12						
		13	R5		F-c SAND, trace f gravel	2.0	108.2	
15		12			F-m SAND			
		10						
					BORING TERMINATED AT 15.0 FEET			
					No groundwater encountered Boring backfilled with soil cuttings			
20								
25								
30								

Sample type:	 ---Ring	 ---SPT	 ---Small Bulk	 ---Large Bulk	 ---No Recovery	 ---Water Table		
Lab testing:	AL = Atterberg Limits	SR = Sulfate/Resistivity Test	EI = Expansion Index	SH = Shear Test	SA = Sieve Analysis	HC= Consolidation	RV = R-Value Test	MD = Maximum Density

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1286'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-2 MATERIAL DESCRIPTION AND COMMENTS	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0				SM	Disturbed Soil: Silty f-c SAND, light brown, slightly moist, trace organics, trace f gravel			
4		4	R1	SP	F-m SAND, brown, slightly moist, loose, trace silt, trace f gravel	4.3	114.9	
6		6			Alluvium:			
10		10		SP	F-c SAND, light brown, slightly moist, medium dense, trace silt, trace f gravel			
5		5	R2		Light gray-brown in color			
8		8						
11		11						
12		7	R3			1.7	103.6	
13		12						
14		13						
10		6	R4		F-m SAND, light brown, slightly moist, medium dense, trace silt	2.8	105.2	
9		9			F-c SAND			
14		14						
15		6	R5		F-m SAND, light brown, slightly moist, medium dense, trace f gravel	2.9	110.4	
9		9						
12		12						
20		9	R6		F SAND, light brown, slightly moist, medium dense			
12		12						
15		15						
					BORING TERMINATED AT 21.5 FEET			
					No groundwater encountered Boring backfilled with soil cuttings			
25								
30								

LEGEND

Sample type:  ---Ring  ---SPT  ---Small Bulk  ---Large Bulk  ---No Recovery  ---Water Table

Lab testing: AL = Atterberg Limits EI = Expansion Index SA = Sieve Analysis RV = R-Value Test
SR = Sulfate/Resistivity Test SH = Shear Test HC = Consolidation MD = Maximum Density

CLIENT:	Redlands Pioneer Partners	DRILLER:	2R Drilling	LOGGED BY:	Lyn
PROJECT NAME:	210 FWY & San Bernardino Ave	DRILL METHOD:	Hollow Stem	OPERATOR:	Jerry
PROJECT NO.:	4212-CR	HAMMER:	140#/30"	RIG TYPE:	CME 55 track rig
COORDINATES:	See Boring Location Map	ELEVATION:	1280'	DATE:	2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-3	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0				SM	Disturbed Soil: Silty f-c SAND, light brown, slightly moist, trace organics, trace f gravel			EI, MD, SH, SR Expansion Index = 0
		3 3 4	R1	SP	F-m SAND, medium brown, slightly moist, loose, trace silt, trace f gravel	5.3	110.8	
5		5 6 7	R2	SP	Alluvium: F-m SAND, light brown, slightly moist, loose, trace silt, trace f gravel			
		9 12 14	R3		F-c SAND, medium dense			
		9 12 14	R4					
10								
		5 7 9	R5	SM	Silty f SAND, medium brown, slightly moist, medium dense			
15								
		13 19 23	R6	SP	F-c SAND, light brown, slightly moist, medium dense, trace f gravel	1.9	111.7	
20					BORING TERMINATED AT 21.5 FEET No groundwater encountered Boring backfilled with soil cuttings			
25								
30								

LEGEND

Sample type:

---Ring

---SPT

---Small Bulk

---Large Bulk

---No Recovery

---Water Table

Lab testing:

AL = Atterberg Limits

EI = Expansion Index

SA = Sieve Analysis

RV = R-Value Test

SR = Sulfate/Resistivity Test

SH = Shear Test

HC= Consolidation

MD = Maximum Density

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1273'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-4	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0				SM	Disturbed Soil: Silty f-c SAND, light brown, slightly moist, trace organics, trace f gravel			El, MD, SH, SR Expansion Index = 0
		4 6 6	R1	SM	Alluvium: Silty f-m SAND, light brown, slightly moist, loose, trace silt, trace f gravel	3.9	107.9	Percent Passing #200 Sieve = 21.1%
5		4 5 7	R2		Silty f-c SAND	3.9	110.2	
		6 7 10	R3		Medium dense	4.8	104.0	HC
10		5 6 9	R4		Medium dense	5.3	103.3	Percent Passing #200 Sieve = 25.8%
15		4 6	R5	SC	Clayey f SAND, light brown, slightly moist, medium dense			
		10		SP	F SAND, light brown, slightly moist, medium dense, trace silt			
20		3 8 18	R6		F-m SAND	3.8	106.8	
25		3 5 7	S1		F SAND, light gray-brown			
30		7 11 12	S2					

LEGEND	Sample type:	---Ring	---SPT	---Small Bulk	---Large Bulk	---No Recovery	---Water Table
	Lab testing:	AL = Atterberg Limits	El = Expansion Index	SA = Sieve Analysis	RV = R-Value Test		
		SR = Sulfate/Resistivity Test	SH = Shear Test	HC= Consolidation	MD = Maximum Density		

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1273'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-4 (continued)	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
35		7 10 12	S3		Light brown in color			
40		6 9 9	S4					
45		7 10 9	S5					
50		7 11 12	S6					
55					BORING TERMINATED AT 51.5 FEET No groundwater encountered Boring backfilled with soil cuttings			
60								

LEGEND

Sample type:

---Ring

---SPT

---Small Bulk

---Large Bulk

---No Recovery

---Water Table

Lab testing:

AL = Atterberg Limits

SR = Sulfate/Resistivity Test

EI = Expansion Index

SH = Shear Test

SA = Sieve Analysis

HC= Consolidation

RV = R-Value Test

MD = Maximum Density

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners
PROJECT NAME: 210 FWY & San Bernardino
PROJECT NO.: 4212-CR
COORDINATES: See Boring Location Map

DRILLER: 2R Drilling
DRILL METHOD: Hollow Stem
HAMMER: 140#/30"
ELEVATION: 1289'

LOGGED BY: Lyn
OPERATOR: Jerry
RIG TYPE: CME 55 track rig
DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: B-5	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
	MATERIAL DESCRIPTION AND COMMENTS							
0					Disturbed Soil: Silty f-c SAND, light brown, slightly moist, trace organics, trace f gravel			
		2	R1	SP	F-m SAND, medium brown, slightly moist, loose, trace silt, trace f gravel, trace organics	5.3	107.7	
		4						
		5						
5		4	R2	SP	Alluvium: F-m SAND, light brown, slightly moist, loose, trace silt, trace f gravel	2.6	105.9	
		6						
		8						
		5	R3		F-c SAND, light gray, medium dense	1.9	103.4	
		10						
		14						
		8	R4		F-m SAND, light brown	2.2	100.9	
		11						
10		10						
		7	R5		F SAND, medium dense, no gravel	6.2	102.4	
		7						
		8						
15					BORING TERMINATED AT 15.0 FEET No groundwater encountered Boring backfilled with soil cuttings			

LEGEND

Sample type:  ---Ring  ---SPT  ---Small Bulk  ---Large Bulk  ---No Recovery  ---Water Table

Lab testing: AL = Atterberg Limits EI = Expansion Index SA = Sieve Analysis RV = R-Value Test
SR = Sulfate/Resistivity Test SH = Shear Test HC= Consolidation MD = Maximum Density

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1268'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: I-I MATERIAL DESCRIPTION AND COMMENTS	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0					Disturbed Soil:			
				SM	Silty f-c SAND, light brown, slightly moist, trace f gravel, trace organics			
				SP	F-m SAND, medium brown, slightly moist, trace silt, trace organics			
					Alluvium:			
				SP	F-m SAND, light brown, slightly moist, trace silt, trace f gravels			
5					BORING TERMINATED AT 5 FEET			
					No groundwater encountered Boring set with pipe, sock, and gravel			
10								
15								
20								
25								
30								

LEGEND	Sample type:		---Ring		---SPT		---Small Bulk		---Large Bulk		---No Recovery		---Water Table			
	Lab testing:	AL = Atterberg Limits		EI = Expansion Index		SA = Sieve Analysis		RV = R-Value Test		SR = Sulfate/Resistivity Test		SH = Shear Test		HC= Consolidation		MD = Maximum Density

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1269'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: I-2	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
	MATERIAL DESCRIPTION AND COMMENTS							
0					Disturbed Soil:			
				SM	Silty f-c SAND, light brown, slightly moist, trace f gravel, trace organics			
				SP	F-m SAND, medium brown, slightly moist, trace silt, trace organics			
					Alluvium:			
				SP	F-c SAND, light brown, slightly moist, trace silt, trace f gravels			
5					BORING TERMINATED AT 5 FEET			
					No groundwater encountered Boring set with pipe, sock, and gravel			
10								
15								
20								
25								
30								

LEGEND	Sample type:	---Ring	---SPT	---Small Bulk	---Large Bulk	---No Recovery	---Water Table
	Lab testing:	AL = Atterberg Limits	EI = Expansion Index	SA = Sieve Analysis	RV = R-Value Test		
	SR = Sulfate/Resistivity Test	SH = Shear Test	HC= Consolidation	MD = Maximum Density			

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1272'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: I-3	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
	MATERIAL DESCRIPTION AND COMMENTS							
0					Disturbed Soil:			
				SM	Silty f-c SAND, light brown, slightly moist, trace f gravel, trace organics			
				SP	F-m SAND, medium brown, slightly moist, trace silt, trace organics			
					Alluvium:			
				SP	F-m SAND, light brown, slightly moist, trace silt, trace f gravels			
5					BORING TERMINATED AT 5 FEET			
					No groundwater encountered Boring set with pipe, sock, and gravel			
10								
15								
20								
25								
30								

LEGEND	Sample type:	---Ring	---SPT	---Small Bulk	---Large Bulk	---No Recovery	---Water Table
	Lab testing:	AL = Atterberg Limits	EI = Expansion Index	SA = Sieve Analysis	RV = R-Value Test		
	SR = Sulfate/Resistivity Test	SH = Shear Test	HC= Consolidation	MD = Maximum Density			

GeoTek, Inc.
LOG OF EXPLORATORY BORING

CLIENT: Redlands Pioneer Partners	DRILLER: 2R Drilling	LOGGED BY: Lyn
PROJECT NAME: 210 FWY & San Bernardino Ave	DRILL METHOD: Hollow Stem	OPERATOR: Jerry
PROJECT NO.: 4212-CR	HAMMER: 140#/30"	RIG TYPE: CME 55 track rig
COORDINATES: See Boring Location Map	ELEVATION: 1272'	DATE: 2/21/2025

Depth (ft)	SAMPLES			USCS Symbol	Boring No.: I-4 MATERIAL DESCRIPTION AND COMMENTS	Laboratory Testing		
	Sample Type	Blows/ 6 in	Sample Number			Water Content (%)	Dry Density (pcf)	Others
0					Disturbed Soil:			
				SM	Silty f-c SAND, light brown, slightly moist, trace f gravel, trace organics			
				SP	F-m SAND, medium brown, slightly moist, trace silt, trace organics			
					Alluvium:			
				SP	F-m SAND, light brown, slightly moist, trace silt, trace f gravels			
5	BORING TERMINATED AT 5 FEET							
	No groundwater encountered Boring set with pipe, sock, and gravel							
10								
15								
20								
25								
30								

LEGEND	Sample type:		---Ring		---SPT		---Small Bulk		---Large Bulk		---No Recovery		---Water Table
	Lab testing:	AL = Atterberg Limits	EI = Expansion Index	SA = Sieve Analysis	RV = R-Value Test	SR = Sulfate/Resistivity Test	SH = Shear Test	HC= Consolidation	MD = Maximum Density				

SUMMARY OF CONE PENETRATION TEST DATA

Project:

**APNs 167-091-09 through 12
210 Fwy & W. San Bernardino Avenue
Redlands, CA
March 24, 2025**

Prepared for:

**Mr. Michael Lucas
GeoTek, Inc.
1548 N. Maple Street
Corona, CA 92878
Office (951) 710-1160 / Fax (951) 710-1167**

Prepared by:



KEHOE TESTING & ENGINEERING

5415 Industrial Drive
Huntington Beach, CA 92649-1518
Office (714) 901-7270 / Fax (714) 901-7289
www.kehoetesting.com

TABLE OF CONTENTS

- 1. INTRODUCTION**
- 2. SUMMARY OF FIELD WORK**
- 3. FIELD EQUIPMENT & PROCEDURES**
- 4. CONE PENETRATION TEST DATA & INTERPRETATION**

APPENDIX

- CPT Plots
- CPT Classification/Soil Behavior Chart
- CPT Data Files (sent via email)

SUMMARY OF CONE PENETRATION TEST DATA

1. INTRODUCTION

This report presents the results of a Cone Penetration Test (CPT) program carried out for the APNs 167-091-09 through 12 project located at 210 Fwy & W. San Bernardino Avenue in Redlands, California. The work was performed by Kehoe Testing & Engineering (KTE) on March 24, 2025. The scope of work was performed as directed by GeoTek, Inc. personnel.

2. SUMMARY OF FIELD WORK

The fieldwork consisted of performing CPT soundings at four locations to determine the soil lithology. A summary is provided in **TABLE 2.1**.

LOCATION	DEPTH OF CPT (ft)	COMMENTS/NOTES:
CPT-1	60	
CPT-2	42	Refusal
CPT-3	60	
CPT-4	60	

TABLE 2.1 - Summary of CPT Soundings

3. FIELD EQUIPMENT & PROCEDURES

The CPT soundings were carried out by **KTE** using an integrated electronic cone system manufactured by Vertek. The CPT soundings were performed in accordance with ASTM standards (D5778). The cone penetrometers were pushed using a 30-ton CPT rig. The cone used during the program was a 15 cm² cone with a cone net area ratio of 0.83. The following parameters were recorded at approximately 2.5 cm depth intervals:

- Cone Resistance (qc)
- Sleeve Friction (fs)
- Dynamic Pore Pressure (u)
- Inclination
- Penetration Speed

The above parameters were recorded and viewed in real time using a laptop computer. Data is stored at the KTE office for up to 2 years for future analysis and reference. A complete set of baseline readings was taken prior to each sounding to determine temperature shifts and any zero load offsets. Monitoring base line readings ensures that the cone electronics are operating properly.

4. CONE PENETRATION TEST DATA & INTERPRETATION

The Cone Penetration Test data is presented in graphical form in the attached Appendix. These plots were generated using the CPeT-IT program. Penetration depths are referenced to ground surface. The soil behavior type on the CPT plots is derived from the attached CPT SBT plot (Robertson, "Interpretation of Cone Penetration Test...", 2009) and presents major soil lithologic changes. The stratigraphic interpretation is based on relationships between cone resistance (q_c), sleeve friction (f_s), and penetration pore pressure (u). The friction ratio (R_f), which is sleeve friction divided by cone resistance, is a calculated parameter that is used along with cone resistance to infer soil behavior type. Generally, cohesive soils (clays) have high friction ratios, low cone resistance and generate excess pore water pressures. Cohesionless soils (sands) have lower friction ratios, high cone bearing and generate little (or negative) excess pore water pressures.

The CPT data files have also been provided. These files can be imported in CPeT-IT (software by GeoLogismiki) and other programs to calculate various geotechnical parameters.

It should be noted that it is not always possible to clearly identify a soil type based on q_c , f_s and u . In these situations, experience, judgement and an assessment of the pore pressure data should be used to infer the soil behavior type.

If you have any questions regarding this information, please do not hesitate to call our office at (714) 901-7270.

Sincerely,

KEHOE TESTING & ENGINEERING



Steven P. Kehoe
President

APPENDIX



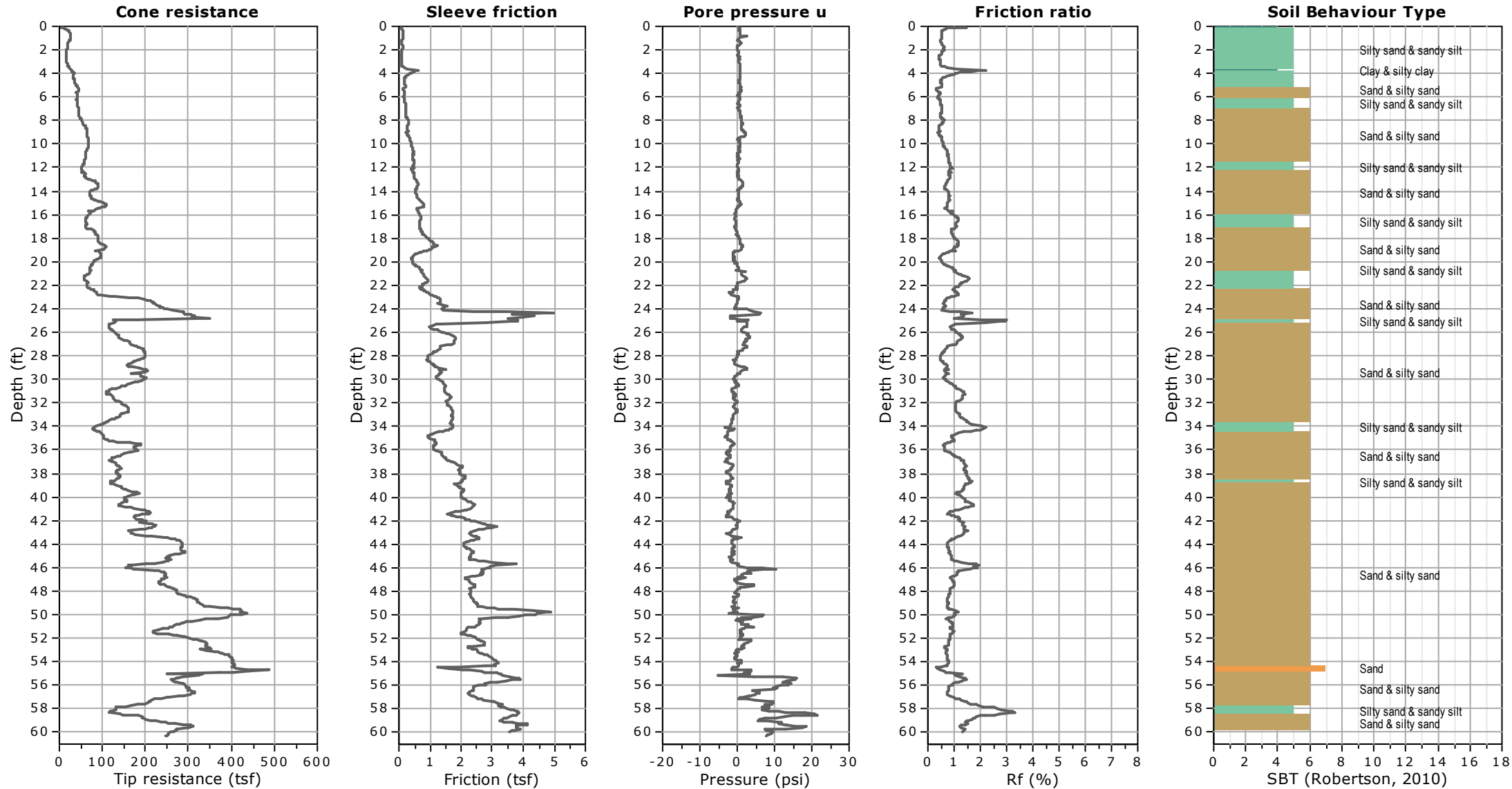
Kehoe Testing and Engineering
714-901-7270
steve@kehoetesting.com
www.kehoetesting.com

Project: GeoTek / APNs 167-091-09 through 12

Location: 210 Fwy & West San Bernadino Ave., Redlands, CA

CPT-1

Total depth: 60.31 ft, Date: 3/24/2025





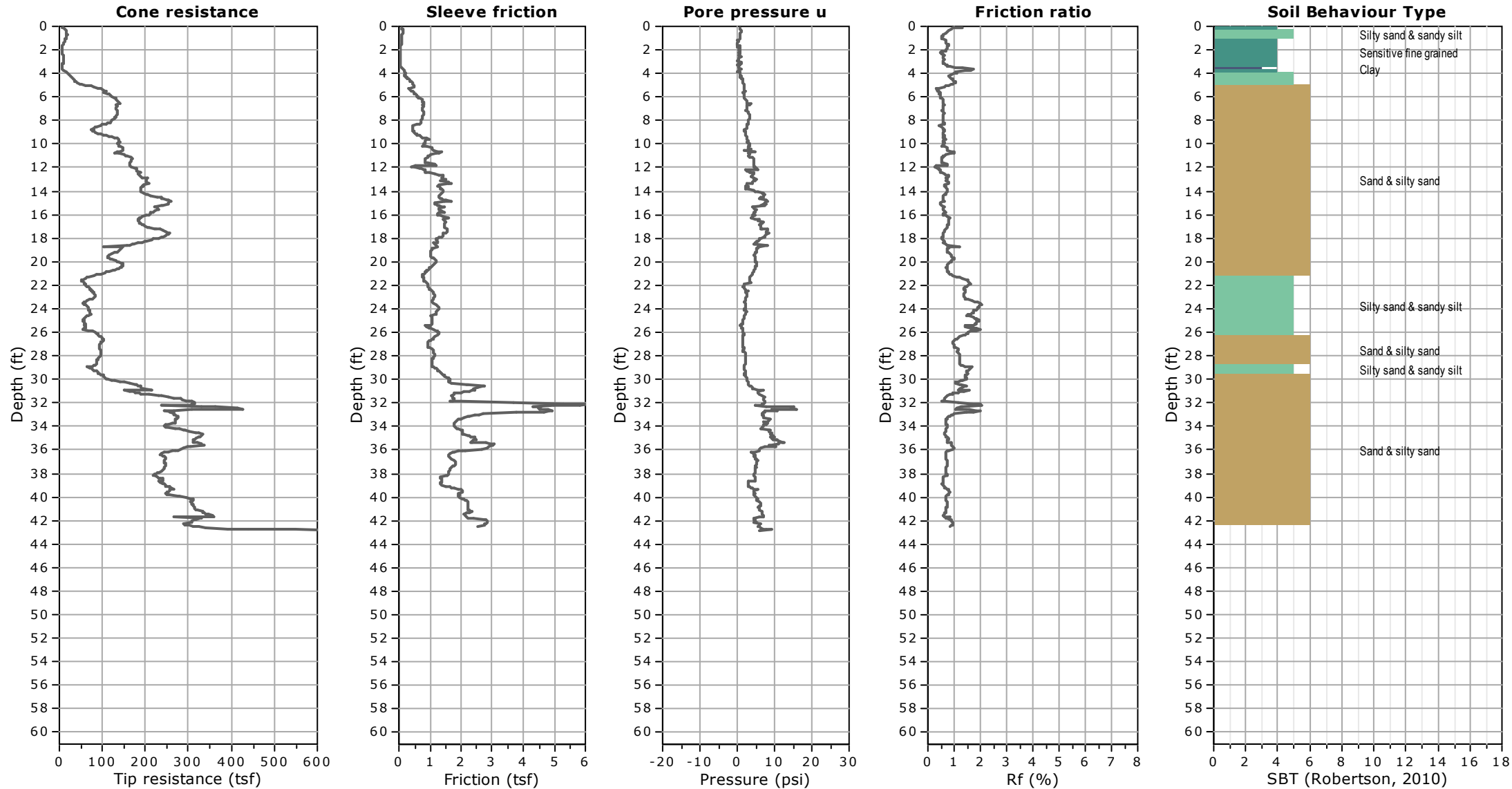
Kehoe Testing and Engineering
714-901-7270
steve@kehoetesting.com
www.kehoetesting.com

Project: GeoTek / APNs 167-091-09 through 12

Location: 210 Fwy & West San Bernadino Ave., Redlands, CA

CPT-2

Total depth: 42.85 ft, Date: 3/24/2025





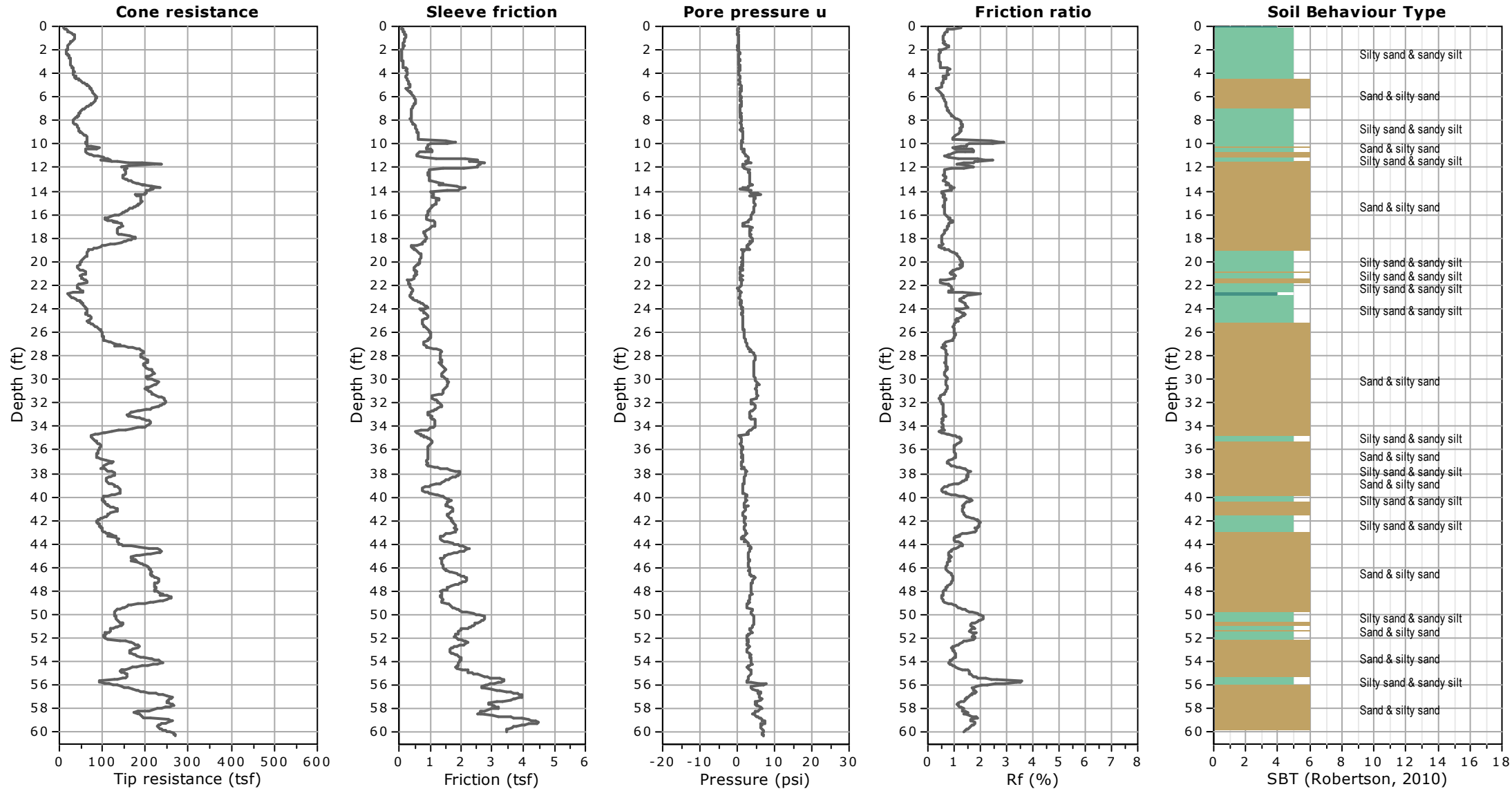
Kehoe Testing and Engineering
714-901-7270
steve@kehoetesting.com
www.kehoetesting.com

Project: GeoTek / APNs 167-091-09 through 12

Location: 210 Fwy & West San Bernadino Ave., Redlands, CA

CPT-3

Total depth: 60.31 ft, Date: 3/24/2025





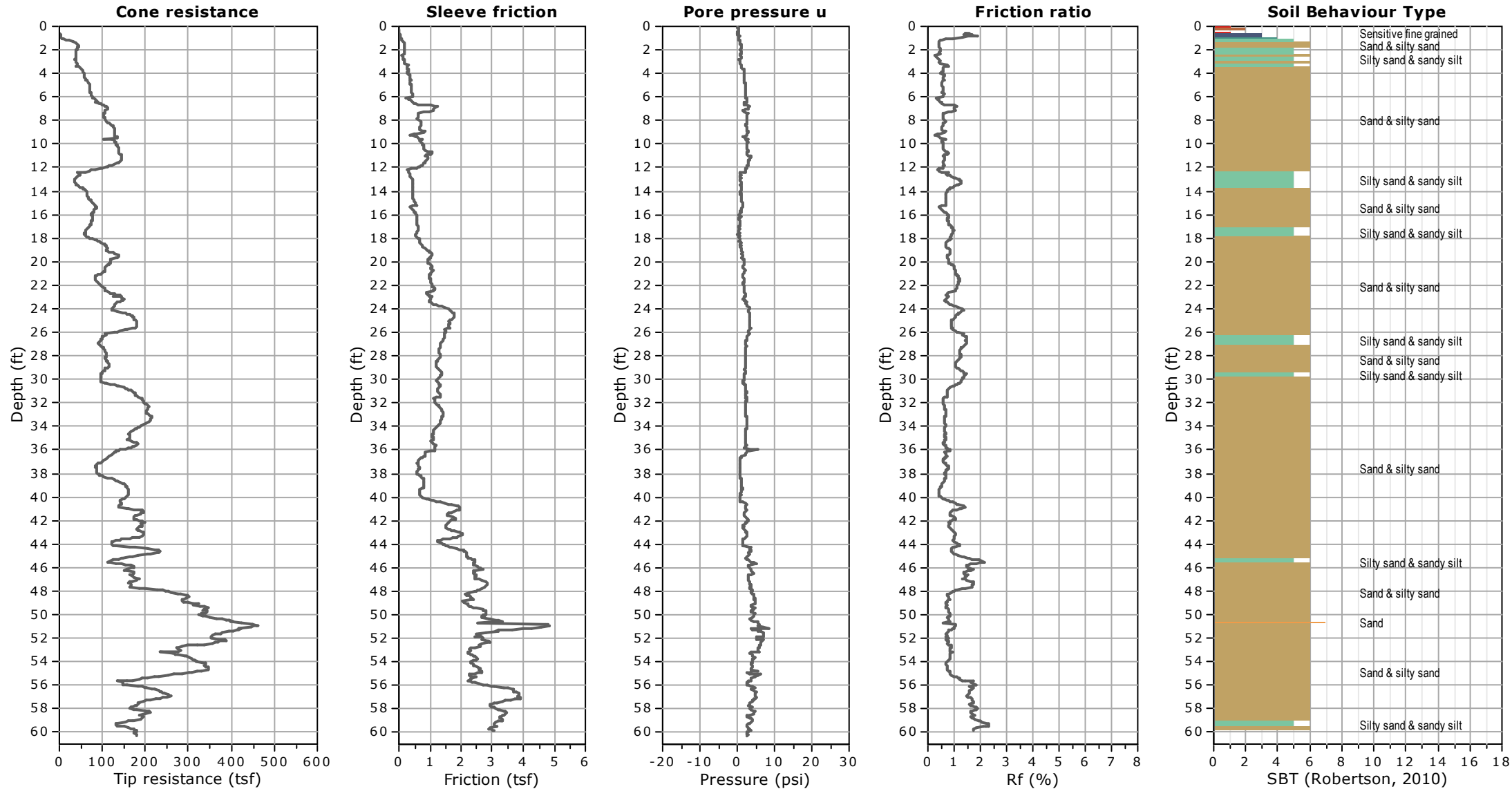
Kehoe Testing and Engineering
714-901-7270
steve@kehoetesting.com
www.kehoetesting.com

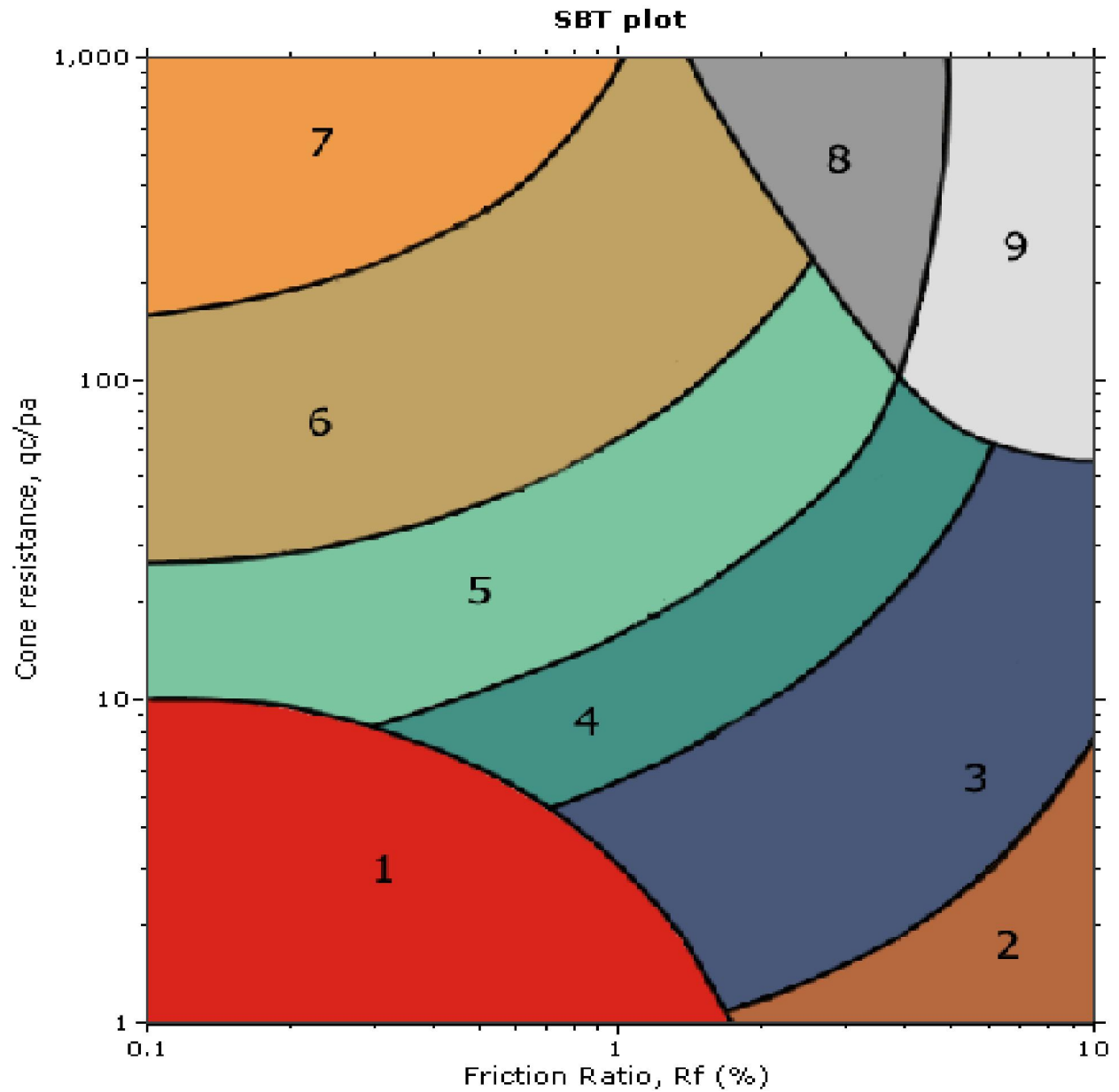
Project: GeoTek / APNs 167-091-09 through 12

Location: 210 Fwy & West San Bernadino Ave., Redlands, CA

CPT-4

Total depth: 60.26 ft, Date: 3/24/2025





SBT legend

- | | | |
|---|---|---|
| ■ 1. Sensitive fine grained | ■ 4. Clayey silt to silty clay | ■ 7. Gravely sand to sand |
| ■ 2. Organic material | ■ 5. Silty sand to sandy silt | ■ 8. Very stiff sand to clayey sand |
| ■ 3. Clay to silty clay | ■ 6. Clean sand to silty sand | ■ 9. Very stiff fine grained |

APPENDIX B

LABORATORY TEST RESULTS

Geotechnical and Infiltration Evaluation

Proposed Residential Development

APNs 167-091-09 through -12

Northeast of the 210 Freeway and West San Bernardino Avenue

Redlands, San Bernardino County, California

Project No. 4212-CR



SUMMARY OF LABORATORY TESTING

Classification

Soils were classified visually in general accordance with the Unified Soil Classification System (ASTM Test Method D 2487). The soil classifications are shown on the boring logs in Appendix A.

Collapse

Selected soil samples were tested to evaluate their potential for collapse. Testing was performed in general accordance with ASTM Test Method D 4546. The test results are provided graphically in Appendix B.

Direct Shear

Shear testing was performed on remolded samples in a direct shear machine of the strain-control type in general accordance with ASTM Test Method D 3080. The rate of deformation is approximately 0.01 inch per minute. The samples were sheared under varying confining loads in order to determine the coulomb shear strength parameters, angle of internal friction and cohesion. The results of the testing are presented graphically in Appendix B.

Expansion Index

The expansion potential of the site soil was estimated by performing Expansion Index testing on selected samples in general accordance with ASTM D 4829. The results of the testing are presented graphically in Appendix B.

In-Situ Moisture and Density

The natural water content was determined (ASTM D 2216) on samples of the materials recovered from the subsurface exploration. In addition, in-place dry density determinations (ASTM D 2937) were performed on relatively undisturbed samples to measure the unit weight of the subsurface soils. Results of these tests are shown on the logs at the appropriate sample depths in Appendix A.

Moisture-Density Relationship

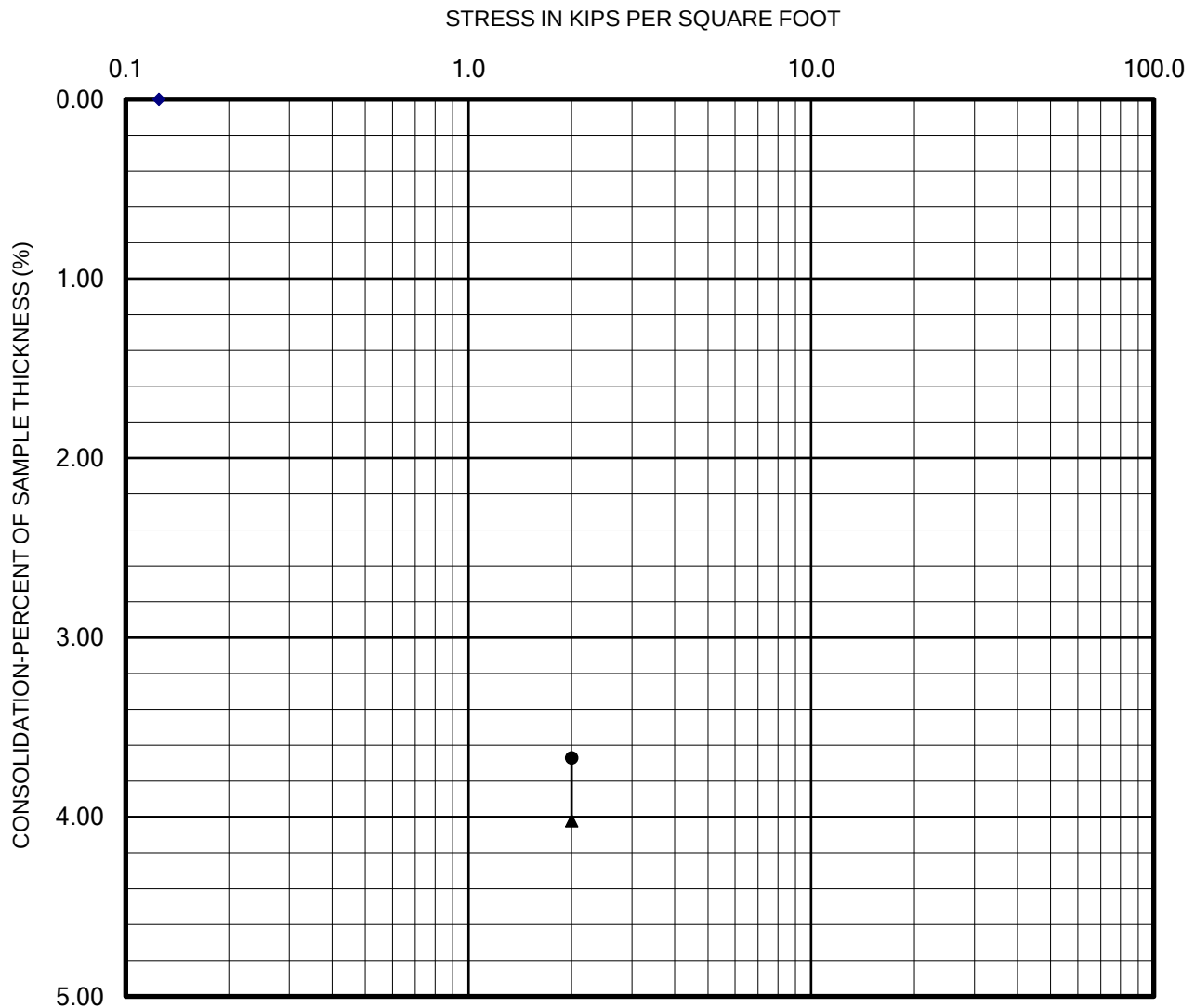
Laboratory testing consisting of a moisture-density relationship was performed on samples obtained during the subsurface exploration. The laboratory maximum dry density and optimum moisture content was determined in general accordance with ASTM D 1557 test procedures. The results of the testing are presented graphically in Appendix B.

Materials Finer Than the No. 200 Sieve

A #200 sieve wash was performed on a selected samples of the soils according to ASTM Test Method D 1140. The results of this testing are presented on the boring logs in Appendix A.

Sulfate Content, Resistivity and Chloride Content

Laboratory testing was performed by Project X Corrosion Engineering in general accordance with ASTM procedures. The testing included pH and water-soluble sulfate content determinations, and resistivity and chloride content testing. The results of the testing are presented in Appendix B.



- Seating Cycle
- Loading Prior to Inundation
- ▲— Loading After Inundation
- ▲--- Rebound Cycle

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 4546



COLLAPSE REPORT

Sample: B-4 @ 7 Feet

Plate B-1

CHECKED BY: EC

Lab: Corona

PROJECT NO.: 4212-CR

Date: 3/3/2025

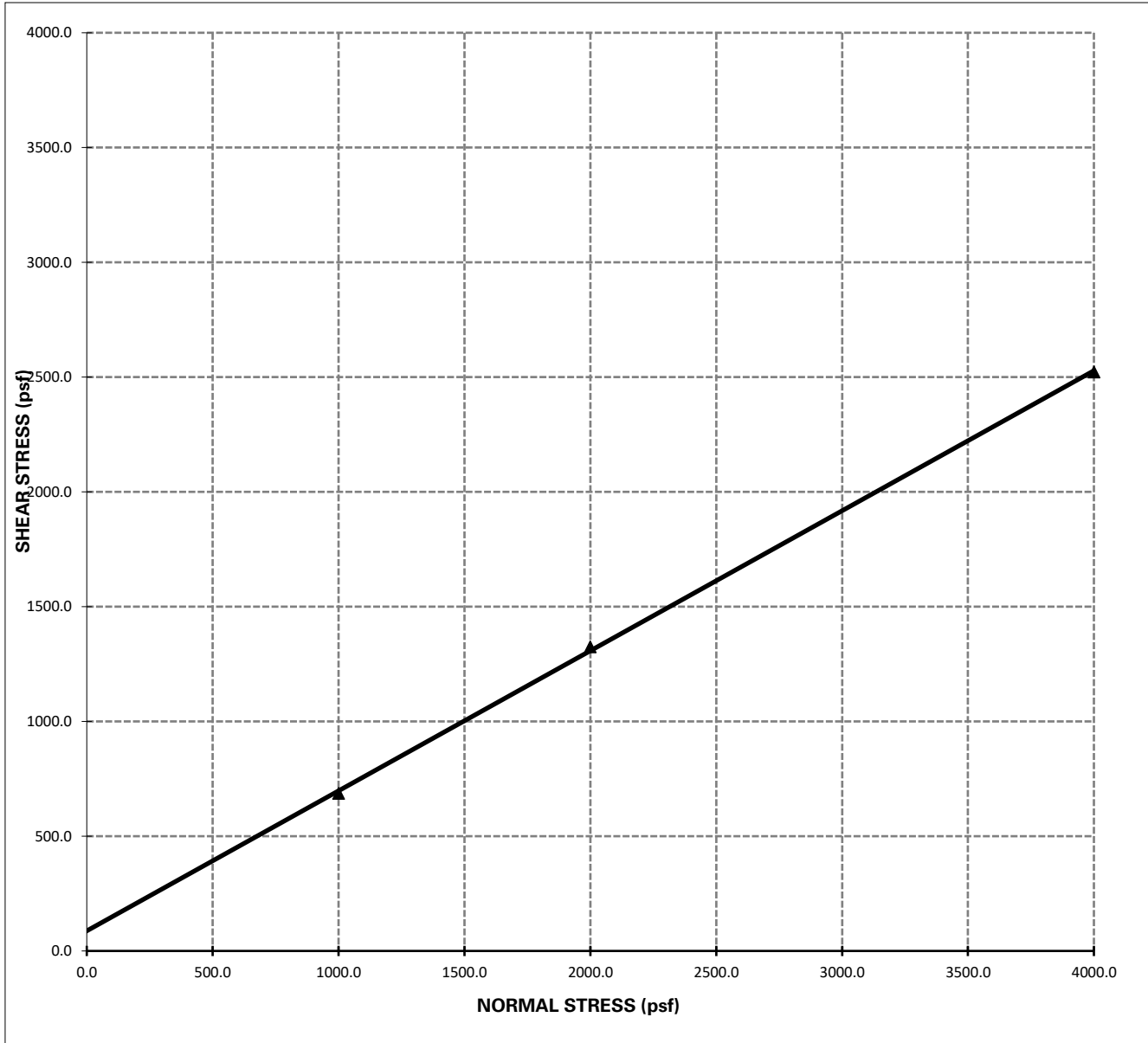
APN's 167-091-09 through -12



DIRECT SHEAR TEST

Project Name: APN's 167-091-09 through -12
Project Number: 4212-CR

Sample Location: B-3 @ 0-5 Feet
Date Tested: 3/6/2025



Shear Strength:

$\Phi = 31^{\circ}$; $C = 87$ psf

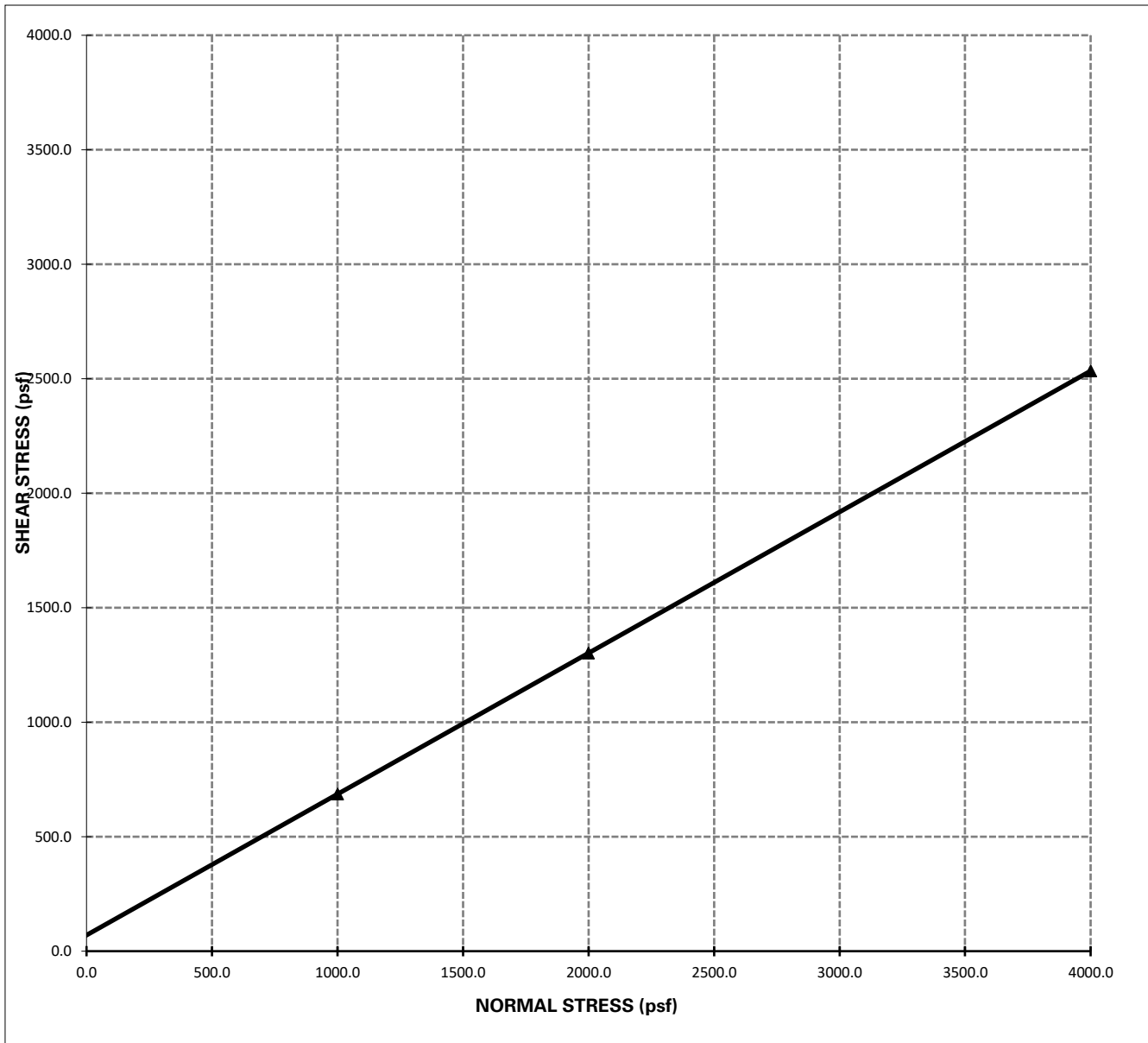
- Notes:**
- 1 - The soil specimen used in the shear box was a ring sample remolded to approximately 90% relative compaction from a bulk sample collected during the field investigation.
 - 2 - The above reflect direct shear strength at saturated conditions.
 - 3 - The tests were run at a shear rate of 0.01 in/min.



DIRECT SHEAR TEST

Project Name: APN's 167-091-09 through -12
Project Number: 4212-CR

Sample Location: B-4 @ 0-5 Feet
Date Tested: 3/6/2025



Shear Strength:

$\Phi = 32^{\circ}$; $C = 70$ psf

- Notes:**
- 1 - The soil specimen used in the shear box was a ring sample remolded to approximately 90% relative compaction from a bulk sample collected during the field investigation.
 - 2 - The above reflect direct shear strength at saturated conditions.
 - 3 - The tests were run at a shear rate of 0.01 in/min.

Report No: PTR:25-00036-S01

Proctor Report

Client: Riverside Pioneer Partners, LLC
4350 Von Karman Ave
Newport Beach CA 92660

CC:

Project: 4212-CR
APN's 167-091-09 through -12

THIS DOCUMENT SHALL NOT BE REPRODUCED EXCEPT IN FULL

Sample Details

Sample ID: 25-00036-S01

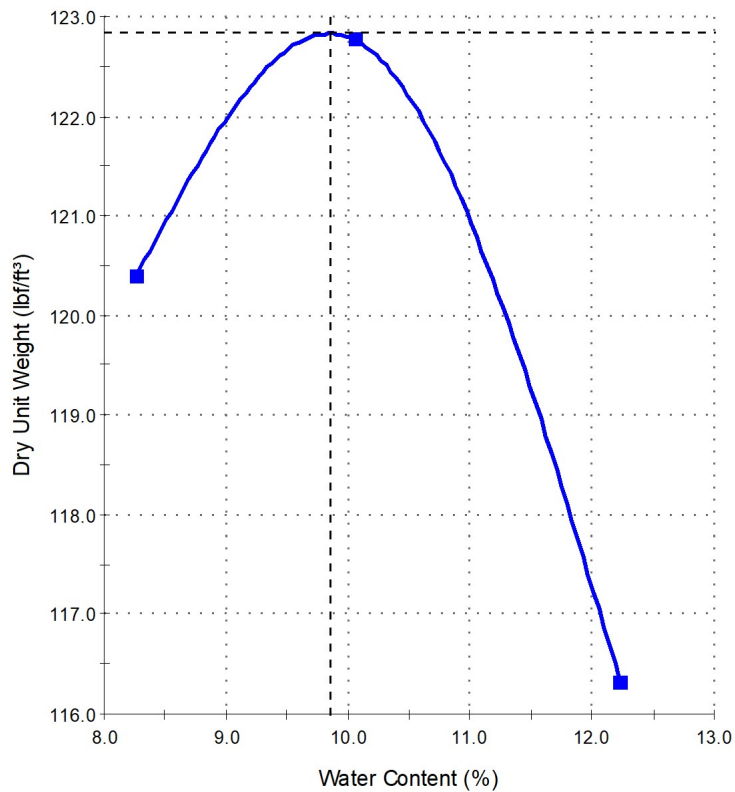
Date Sampled: 2/21/2025

Sampled By:

Material: Silty Fine to Medium SAND

Location: B-3 @ 0-5 Feet

Dry Unit Weight - Water Content Relationship



Test Results

ASTM D 1557

Maximum Dry Unit Weight (lb/ft³): 122.8

Optimum Water Content (%): 9.9

Method: A

Preparation Method: Moist

Retained Sieve No 4 (4.75mm) (%): 2

Passing Sieve No 4 (4.75mm) (%): 98

Tested By: Eduardo Cuevas

Date Tested: 2/27/2025

Comments

Report No: PTR:25-00036-S02

Proctor Report

Client: Riverside Pioneer Partners, LLC
4350 Von Karman Ave
Newport Beach CA 92660

CC:

Project: 4212-CR
APN's 167-091-09 through -12

THIS DOCUMENT SHALL NOT BE REPRODUCED EXCEPT IN FULL

Sample Details

Sample ID: 25-00036-S02

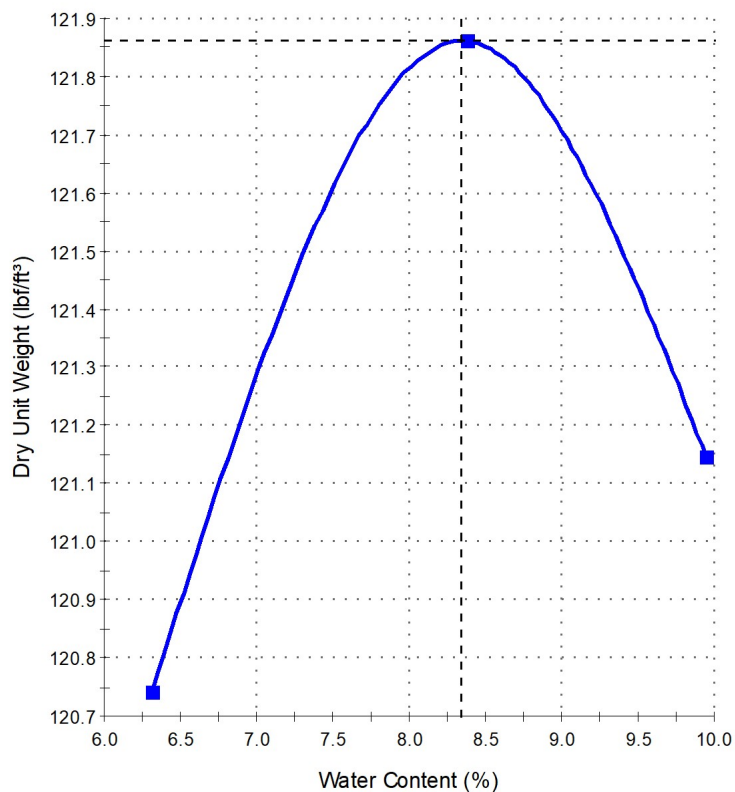
Date Sampled: 2/28/2025

Sampled By:

Material: Silty Fine to Medium SAND

Location: B-4 @ 0-5 Feet

Dry Unit Weight - Water Content Relationship



Test Results

ASTM D 1557

Maximum Dry Unit Weight (lb/ft³): 121.9

Optimum Water Content (%): 8.3

Method: A

Preparation Method: Moist

Retained Sieve No 4 (4.75mm) (%): 2

Passing Sieve No 4 (4.75mm) (%): 98

Tested By: Eduardo Cuevas

Date Tested: 2/27/2025

Comments



EXPANSION INDEX TEST

(ASTM D4829)

Client: Redlands Pioneer Partners, LLC
Project Number: 4212-CR
Project Location: APN's 167-091-09 through -12

Tested/ Checked By: AH Lab No Corona
Date Tested: 2/26/2025
Sample Source: B-3 @ 0-5 Feet
Sample Description:

Ring #: _____ Ring Dia. : 4.01" Ring Ht. 1"

DENSITY DETERMINATION

Weight of compacted sample & ring (gm)	771.2
Weight of ring (gm)	362.1
Net weight of sample (gm)	409.1
Wet Density, lb / ft ³ (C*0.3016)	123.4
Dry Density, lb / ft ³ (D/1.F)	112.7

SATURATION DETERMINATION

Moisture Content, %	9.5
Specific Gravity, assumed	2.70
Unit Wt. of Water @ 20°C, (pcf)	62.4
% Saturation	51.8

READINGS		
DATE	TIME	READING
2/26/2025		0.2350
2/26/2025		0.2350
2/27/2025		0.2350

Initial
10 min/Dry

Final

FINAL MOISTURE

Final Weight of wet sample & tare	% Moisture
788.3	13.7

EXPANSION INDEX = 0



EXPANSION INDEX TEST

(ASTM D4829)

Client: Redlands Pioneer Partners, LLC
Project Number: 4212-CR
Project Location: APN's 167-091-09 through -12

Tested/ Checked By: AH Lab No Corona
Date Tested: 2/26/2025
Sample Source: B-4 @ 0-5 Feet
Sample Description:

Ring #: _____ Ring Dia. : 4.01" Ring Ht. 1"

DENSITY DETERMINATION

Weight of compacted sample & ring (gm)	774.9
Weight of ring (gm)	365.2
Net weight of sample (gm)	409.7
Wet Density, lb / ft ³ (C*0.3016)	123.6
Dry Density, lb / ft ³ (D/1.F)	113.1

SATURATION DETERMINATION

Moisture Content, %	9.3
Specific Gravity, assumed	2.70
Unit Wt. of Water @ 20°C, (pcf)	62.4
% Saturation	51.2

READINGS

DATE	TIME	READING	
2/26/2025		0.6710	Initial
2/26/2025		0.6710	10 min/Dry
2/27/2025		0.6710	Final

FINAL MOISTURE

Final Weight of wet sample & tare	% Moisture
791.6	13.4

EXPANSION INDEX = 0



Results Only Soil Testing for APNs 167-091-09 through -12

February 28, 2025

Prepared for:

**Eddy Cuevas
GeoTek USA
1548 N. Maple Avenue
Corona, CA 92878
Ecuevas@geotekusa.com, jbrucelas@geotekusa.com**

**Project X Job#: S250227K
Client Job or PO#: 4212-CR**

Prepared by:
N. Rabatin

Respectfully Submitted,

Eduardo Hernandez, M.Sc., P.E.
Sr. Corrosion Consultant
NACE Corrosion Technologist #16592
Professional Engineer
California No. M37102
ehernandez@projectxcorrosion.com





Soil Analysis Lab Results

Client: GeoTek USA
Job Name: APNs 167-091-09 through -12
Client Job Number: 4212-CR
Project X Job Number: S250227K
February 28, 2025

	Method	ASTM D4327		ASTM D4327		ASTM G187	ASTM G51	ASTM G200	SM 4500-D	ASTM D4327	ASTM D6919	ASTM D6919	ASTM D6919	ASTM D6919	ASTM D6919	ASTM D6919	ASTM D4327	ASTM D4327
Bore# / Description	Depth	Sulfates SO ₄ ²⁻		Chlorides Cl ⁻		Resistivity As Rec'd / Minimum	pH	Redox	Sulfide S ²⁻	Nitrate NO ₃ ⁻	Ammonium NH ₄ ⁺	Lithium Li ⁺	Sodium Na ⁺	Potassium K ⁺	Magnesium Mg ²⁺	Calcium Ca ²⁺	Fluoride F ₂ ²⁻	Phosphate PO ₄ ³⁻
	(ft)	(mg/kg)	(wt%)	(mg/kg)	(wt%)	(Ω-cm)		(mV)	(mg/kg)		(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)
B-3	0-5	42.7	0.0043	23.3	0.0023	38,190	8.3	142	0.30	52.1	2.3	ND	31.9	15.0	16.3	92.4	4.6	1.1
B-4	0-5	59.9	0.0060	33.8	0.0034	67,000	8.5	129	0.27	9.5	ND	ND	47.2	10.1	16.5	98.9	7.7	7.6

Cations and Anions, except Sulfide and Bicarbonate, tested with Ion Chromatography
mg/kg = milligrams per kilogram (parts per million) of dry soil weight
ND = 0 = Not Detected | NT = Not Tested | Unk = Unknown
Chemical Analysis performed on 1:3 Soil-To-Water extract
PPM = mg/kg (soil) = mg/L (Liquid)

For AWWA 105C: 0-3mg/kg sulfide = Negative; 3-6mg/kg = trace; >6mg/kg = Positive

Note: Sometimes a bad sulfate hit is a contaminated spot. Typical fertilizers are Potassium chloride, ammonium sulfate or ammonium sulfate nitrate (ASN). So this is another reason why testing full corrosion series is good because we then have the data to see if those other ingredients are present meaning the soil sample is just fertilizer-contaminated soil. This can happen often when the soil samples collected are simply surface scoops. This is why it's best to dig in a foot, throw away the top and test the deeper stuff. Dairy farms are also notorious for these items.

If one sample pops up much more corrosive than all others, we would recommend collecting more samples surrounding the problem sample location to determine if the peak is isolated to it. This allows us to conclude it was a contaminated sample and able to declare it an outlier.

Try out our new online forms: [SOIL CORROSIVITY & THERMAL RESISTIVITY LAB REQUEST FORM](#) & [IN-SITU WENNER 4 PIN QUOTE REQUEST FORM](#)

APPENDIX C

INFILTRATION TEST DATA AND PORCHET CALCULATIONS

**Geotechnical and Infiltration Evaluation
Proposed Residential Development
APNs 167-091-09 through -12
Northeast of the 210 Freeway and West San Bernardino Avenue
Redlands, San Bernardino County, California
Project No. 4212-CR**



PERCOLATION DATA SHEET

Project: APN's 167-091-09 through -12

Job No.: 4212-CR

Test Hole No.: I-I **Tested By:** DRW

Date: 2/24/2025

Depth of Hole As Drilled: 60" **Before Test:** 60"

After Test: 60"

[illegible]

PERCOLATION DATA SHEET

Project: APN's 167-091-09 through -12

Job No.: 4212-CR

Test Hole No.: I-2 **Tested By:** DRW

Date: 2/24/2025

Depth of Hole As Drilled: 60" **Before Test:** 60"

After Test: 60"

[illegible]

PERCOLATION DATA SHEET

Project: APN's 167-091-09 through -12

Job No.: 4212-CR

Test Hole No.: I-3 **Tested By:** JR

Date: 2/21/2025

Depth of Hole As Drilled: 60" **Before Test:** 60"

After Test: 60"

[illegible]

PERCOLATION DATA SHEET

Project: APN's 167-091-09 through -12

Job No.: 4212-CR

Test Hole No.: I-4 **Tested By:** JR

Date: 2/21/2025

Depth of Hole As Drilled: 60" **Before Test:** 60"

After Test: 60"

[illegible]

Client: Redlands Pioneer Partners, LLC
Project: APNs 167-091-09 through -012
Project No: 4212-CR
Date: 2/24/2025

Boring No. I-I

Percolation to Infiltration Rate (Porchet Method)

Time Interval, Δt = 10
 Final Depth to Water, D_F = 41.75
 Test Hole Radius, r = 4
 Initial Depth to Water, D_O = 36
 Total Test Hole Depth, D_T = 60

$$\text{Equation - } I_t = \frac{\Delta H (60r)}{\Delta t (r + 2H_{avg})}$$

$H_O = D_T - D_O = 24$
 $H_F = D_T - D_F = 18.25$
 $\Delta H = \Delta D = H_O - H_F = 5.75$
 $H_{avg} = (H_O + H_F) / 2 = 21.125$

$I_t =$
2.98
 Inches per Hour

Client: Redlands Pioneer Partners, LLC
Project: APNs 167-091-09 through -012
Project No: 4212-CR
Date: 2/24/2025

Boring No. I-2

Percolation to Infiltration Rate (Porchet Method)

Time Interval, $\Delta t =$ 10
Final Depth to Water, $D_F =$ 40.25
Test Hole Radius, $r =$ 4
Initial Depth to Water, $D_O =$ 36
Total Test Hole Depth, $D_T =$ 60

Equation - $I_t = \frac{\Delta H (60r)}{\Delta t (r+2H_{avg})}$

$H_O = D_T - D_O =$ 24
 $H_F = D_T - D_F =$ 19.75
 $\Delta H = \Delta D = H_O - H_F =$ 4.25
 $H_{avg} = (H_O + H_F)/2 =$ 21.875

$I_t =$ 2.14 Inches per Hour

Client: Redlands Pioneer Partners, LLC
Project: APNs 167-091-09 through -012
Project No: 4212-CR
Date: 2/24/2025

Boring No. I-3

Percolation to Infiltration Rate (Porchet Method)

Time Interval, $\Delta t =$ 10
Final Depth to Water, $D_F =$ 44.25
Test Hole Radius, $r =$ 4
Initial Depth to Water, $D_O =$ 36
Total Test Hole Depth, $D_T =$ 60

Equation - $I_t = \frac{\Delta H (60r)}{\Delta t (r+2H_{avg})}$

$H_O = D_T - D_O =$ 24
 $H_F = D_T - D_F =$ 15.75
 $\Delta H = \Delta D = H_O - H_F =$ 8.25
 $H_{avg} = (H_O + H_F)/2 =$ 19.875

$I_t =$ 4.53 Inches per Hour

Client: Redlands Pioneer Partners, LLC
Project: APNs 167-091-09 through -012
Project No: 4212-CR
Date: 2/24/2025

Boring No. I-4

Percolation to Infiltration Rate (Porchet Method)

Time Interval, $\Delta t =$ 10
Final Depth to Water, $D_F =$ 42.5
Test Hole Radius, $r =$ 4
Initial Depth to Water, $D_O =$ 36
Total Test Hole Depth, $D_T =$ 60

Equation - $I_t = \frac{\Delta H (60r)}{\Delta t (r+2H_{avg})}$

$H_O = D_T - D_O =$ 24
 $H_F = D_T - D_F =$ 17.5
 $\Delta H = \Delta D = H_O - H_F =$ 6.5
 $H_{avg} = (H_O + H_F)/2 =$ 20.75

$I_t =$ 3.43 Inches per Hour

APPENDIX D

SEISMIC “DRY SAND” SETTLEMENT ANALYSIS

Geotechnical and Infiltration Evaluation

Proposed Residential Development

APNs 167-091-09 through -12

Northeast of the 210 Freeway and West San Bernardino Avenue

Redlands, San Bernardino County, California

Project No. 4212-CR



LIQUEFACTION ANALYSIS REPORT

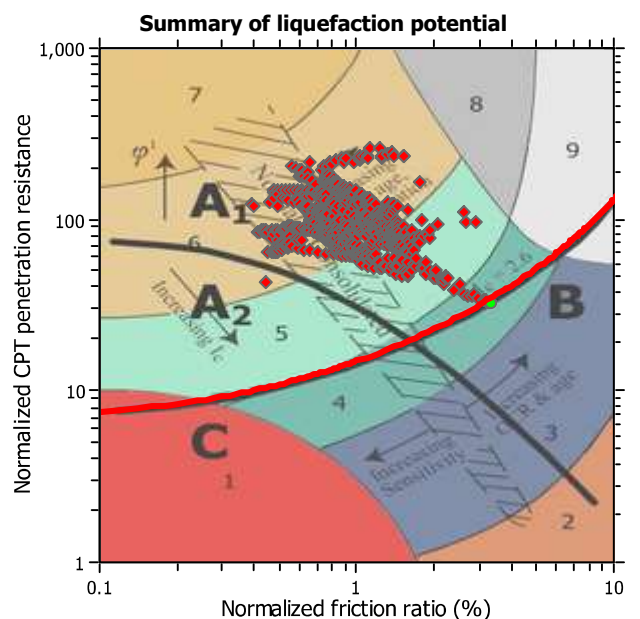
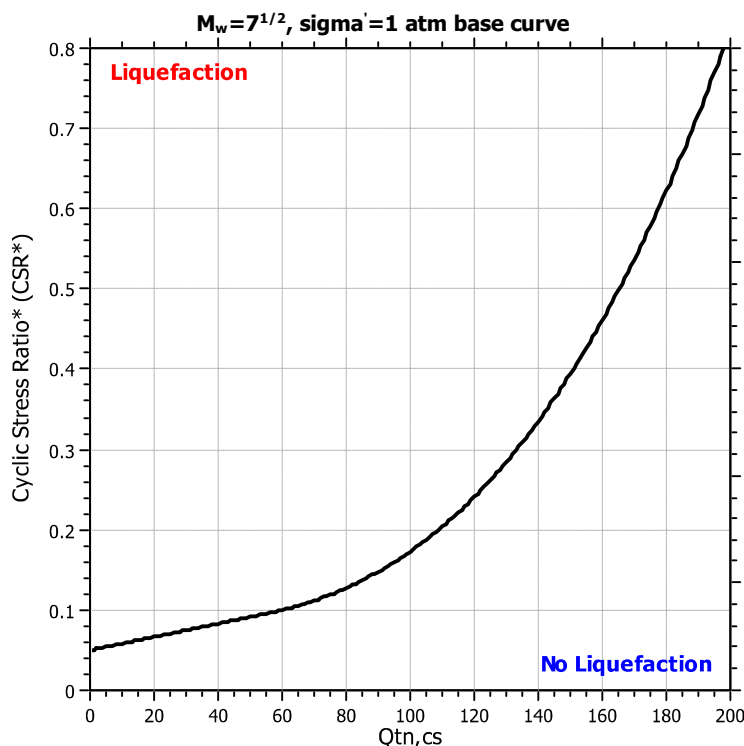
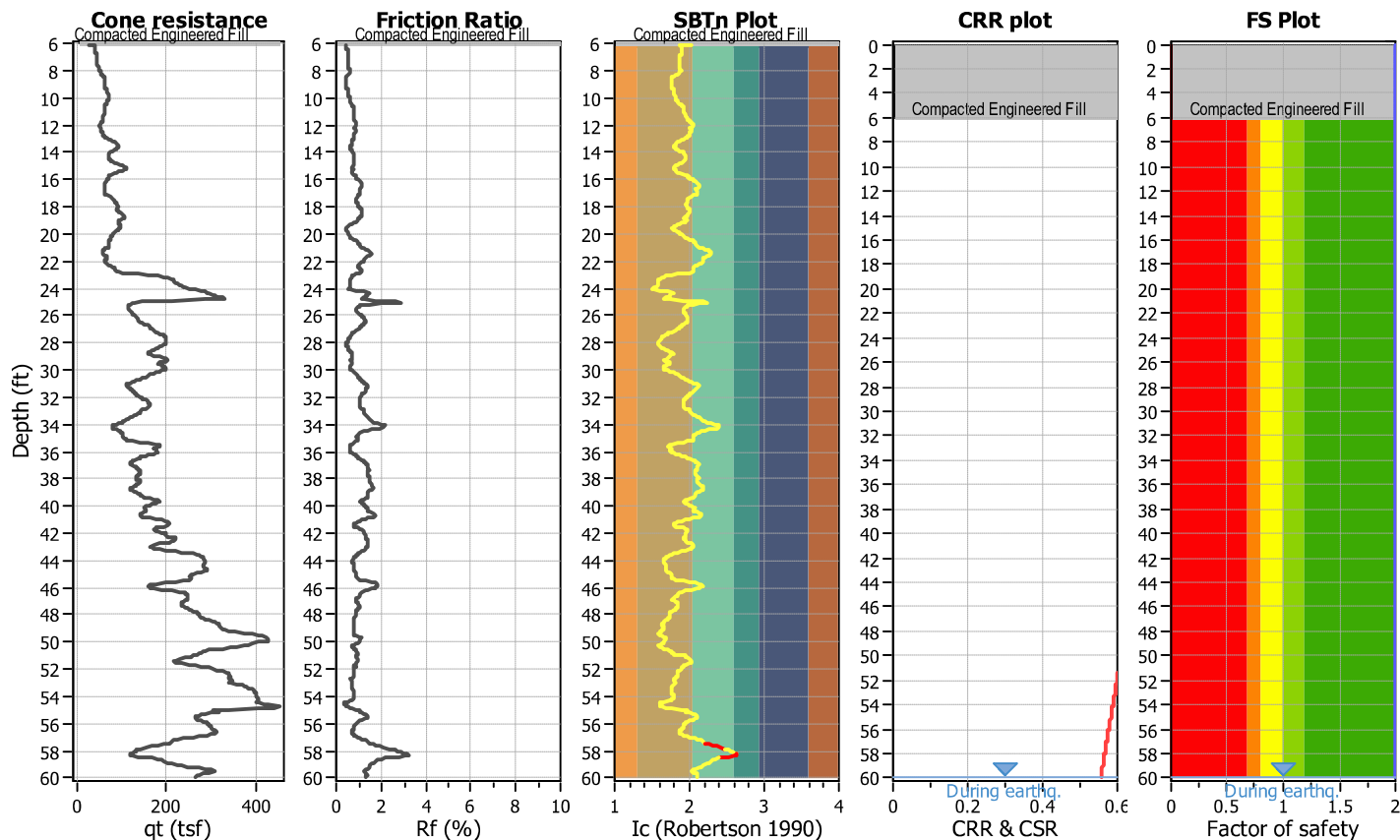
Project title : 4212-CR

Location : Northeast of the 210 Freeway and West San

CPT file : CPT-1

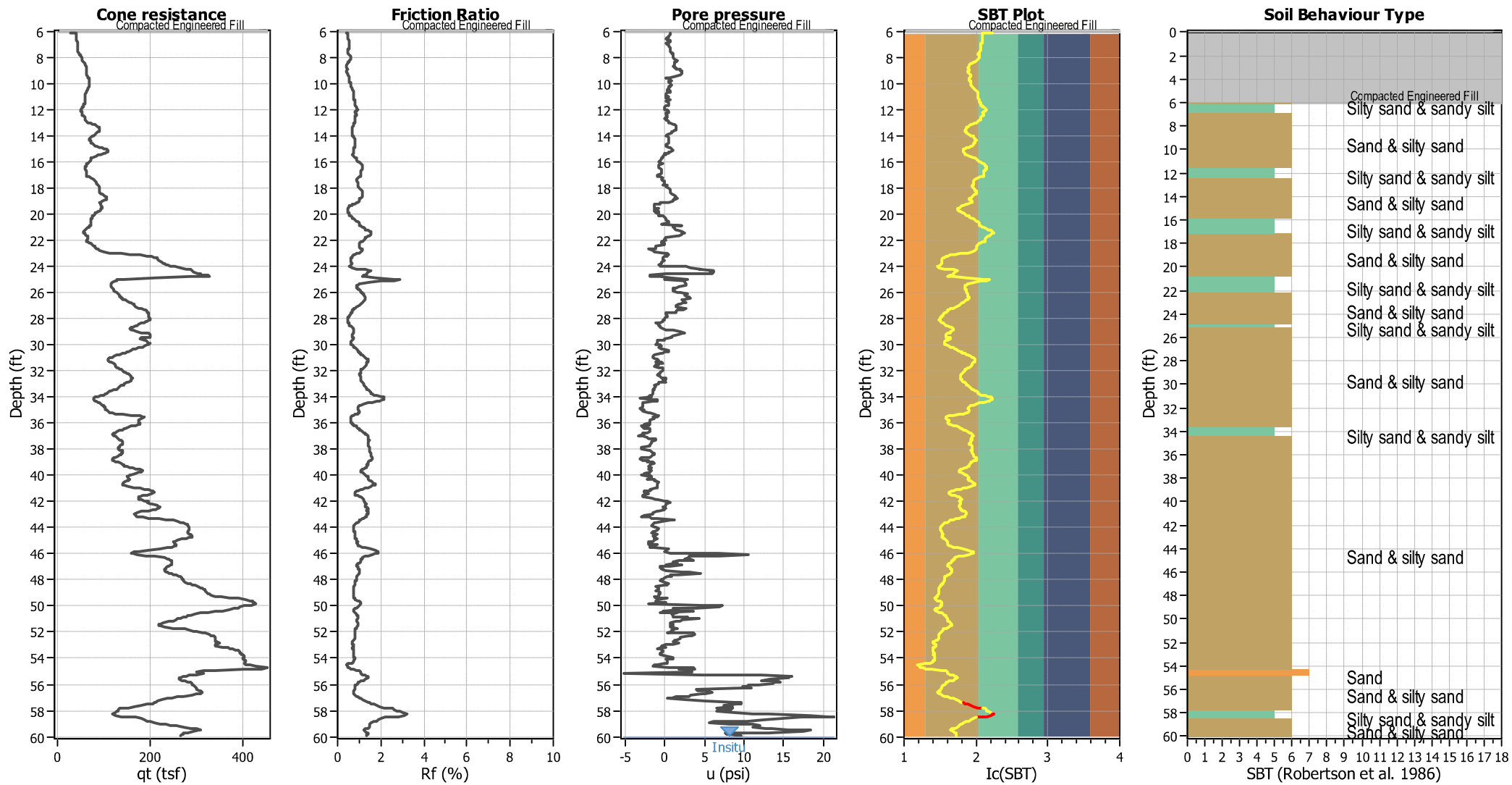
Input parameters and analysis data

Analysis method:	NCEER (1998)	G.W.T. (in-situ):	60.00 ft	Use fill:	No	Clay like behavior	
Fines correction method:	NCEER (1998)	G.W.T. (earthq.):	60.00 ft	Fill height:	N/A	applied:	Sands only
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth applied:	No
Earthquake magnitude M_w :	7.91	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	Limit depth:	N/A
Peak ground acceleration:	0.88	Unit weight calculation:	Based on SBT	K_0 applied:	Yes	MSF method:	Method based



Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading
 Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground geometry
 Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening
 Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

CPT basic interpretation plots

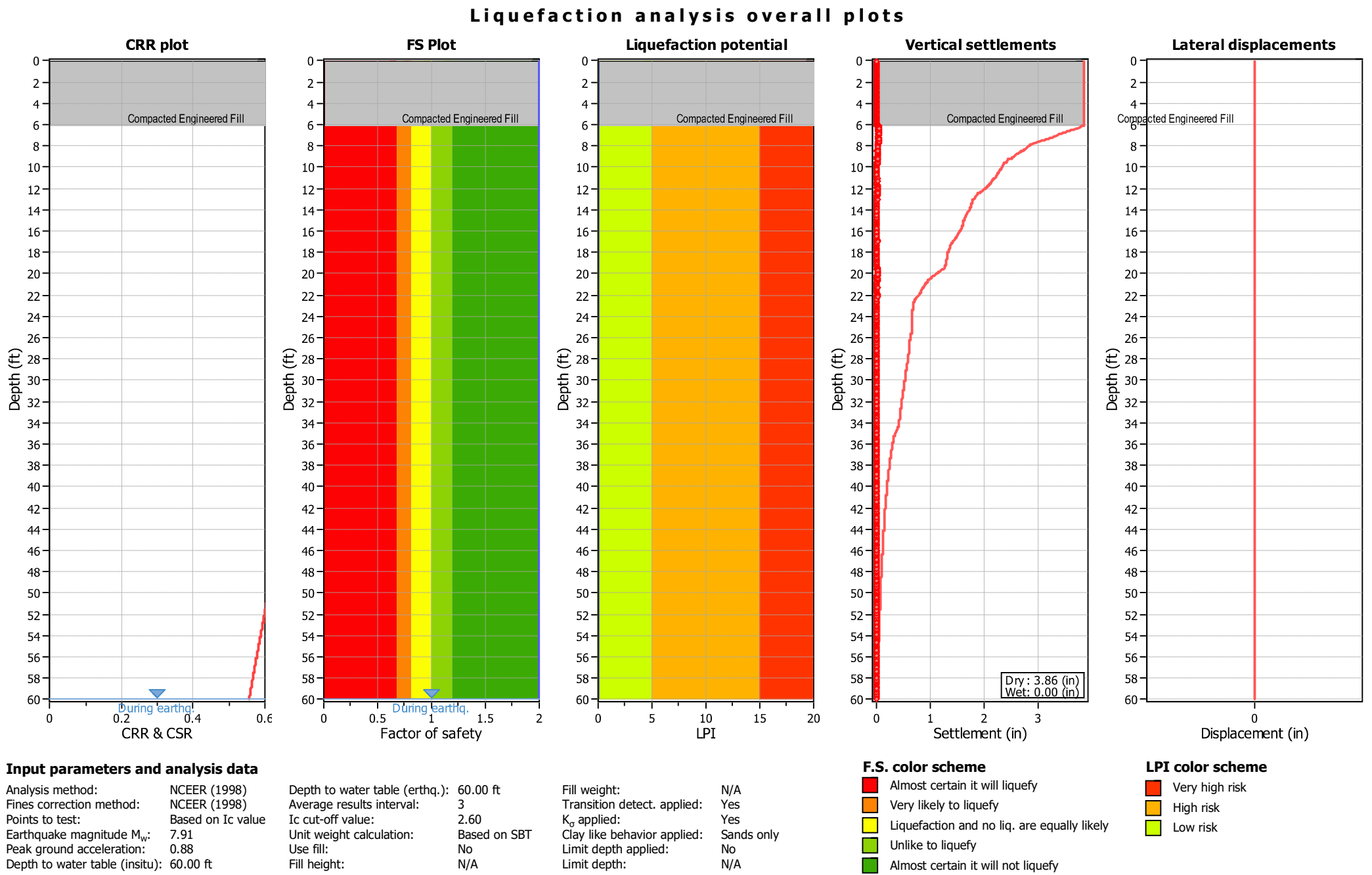


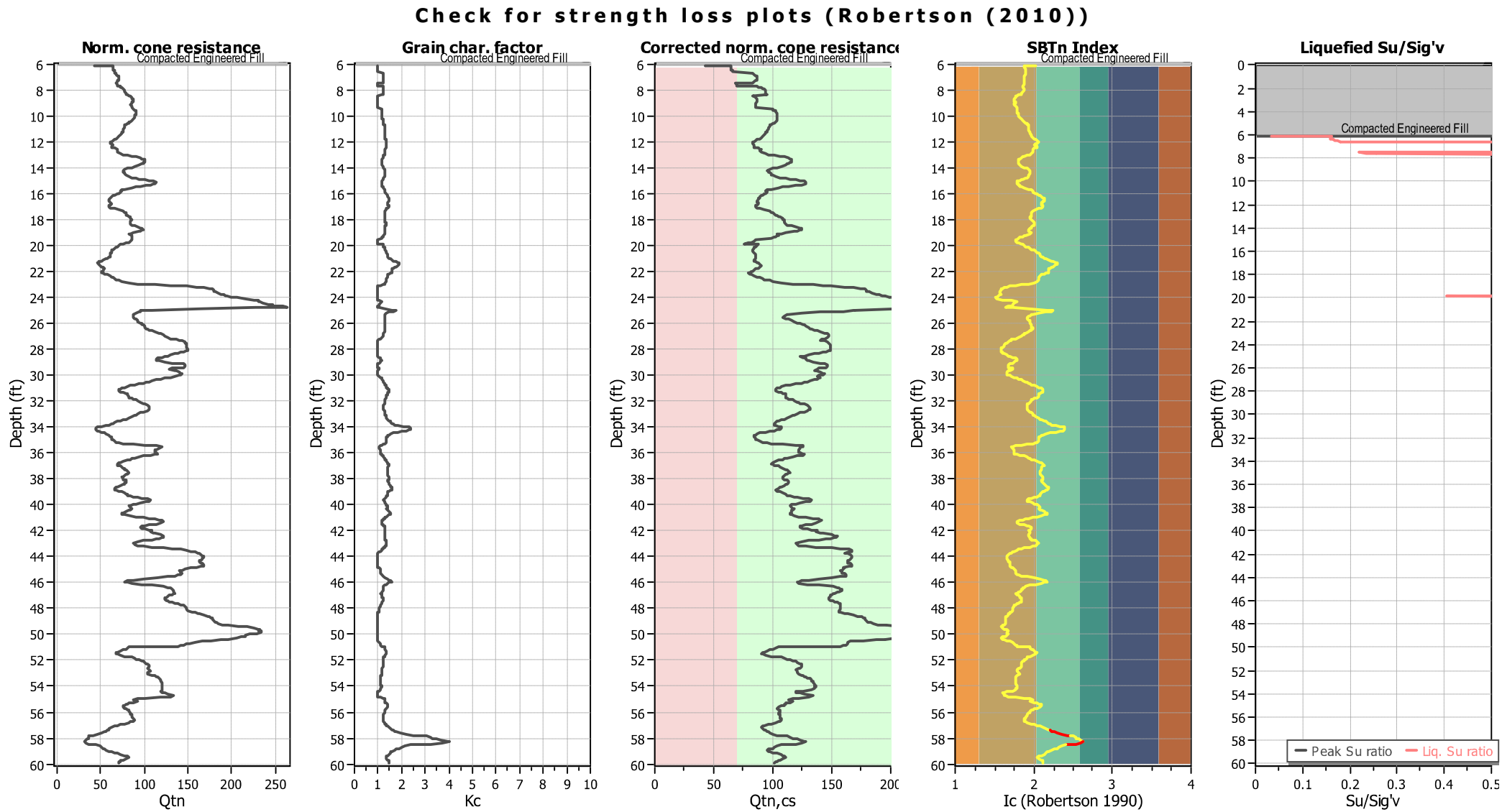
Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _o applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

SBT legend

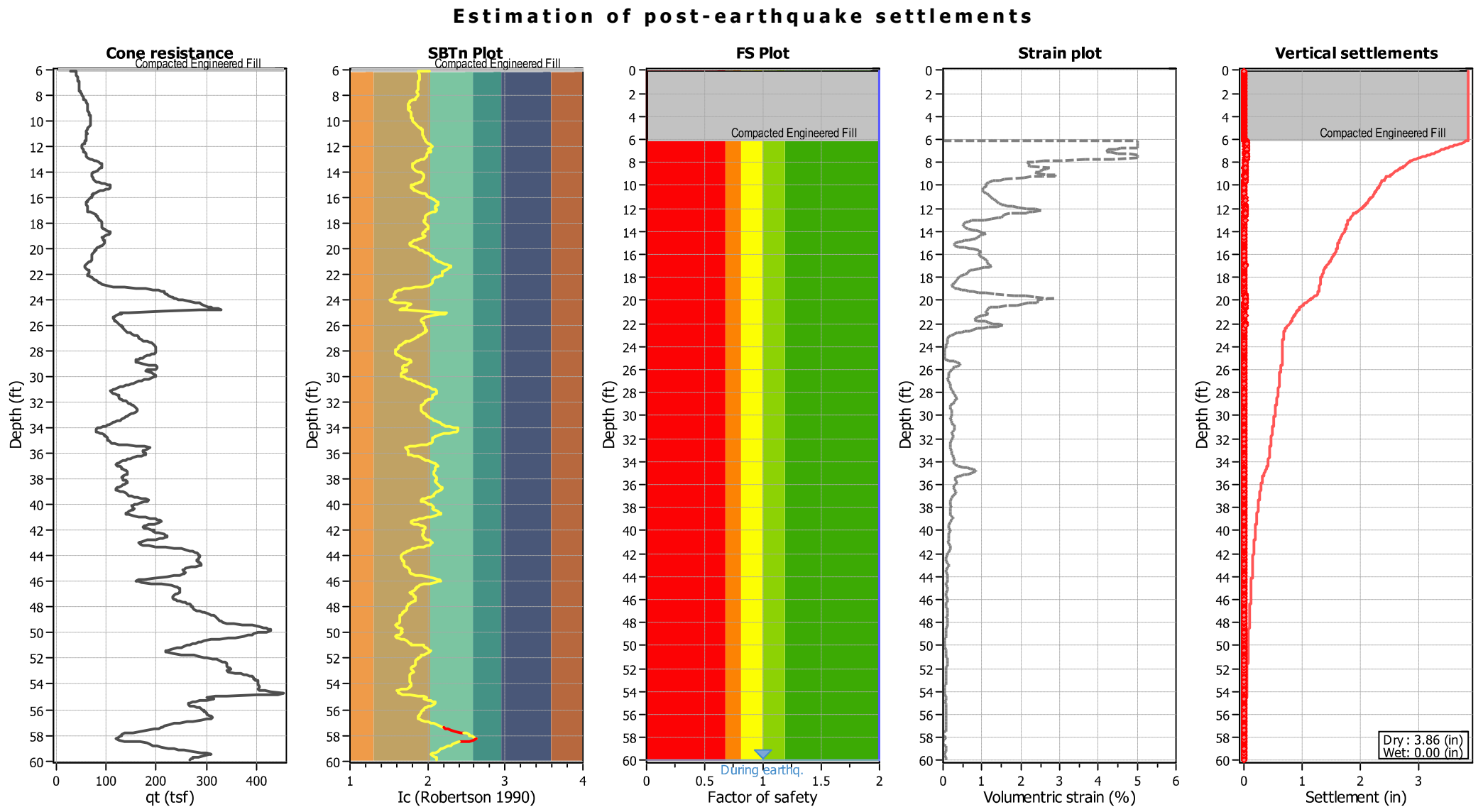
1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained





Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (earthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _g applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A



Abbreviations

- q_t : Total cone resistance (cone resistance q_c corrected for pore water effects)
- I_c : Soil Behaviour Type Index
- FS: Calculated Factor of Safety against liquefaction
- Volumetric strain: Post-liquefaction volumetric strain

:: Post-earthquake settlement of dry sands ::												
Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
0.07	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.14	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.22	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.27	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.36	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.40	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.50	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.53	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.60	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.69	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.74	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.82	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.87	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.93	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.01	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.06	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.13	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.19	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.28	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.31	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.38	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.46	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.51	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.59	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.66	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.74	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.84	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.98	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.05	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.12	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.17	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.26	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.33	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.40	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.44	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.52	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.59	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.63	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.70	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.77	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.85	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.90	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.98	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.05	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.09	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.16	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
3.23	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.30	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.37	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.41	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.48	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.58	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.64	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.74	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.88	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.01	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.09	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.15	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.24	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.33	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.42	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.47	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.56	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.61	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.70	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.75	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.79	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.89	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.93	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.03	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.07	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.15	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.21	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.30	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.35	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.40	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.47	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.53	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.62	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.74	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.85	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.99	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
6.06	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
6.12	2.03	43.02	1.00	43.02	9	315	0.65	23.625	61.15	19.28	5.00	0.040
6.18	1.88	64.41	1.00	64.41	13	388	0.65	4.363	7.46	19.28	5.00	0.032
6.26	1.89	64.25	1.00	64.25	13	392	0.65	4.264	7.28	19.28	5.00	0.051
6.31	1.89	64.22	1.00	64.22	13	394	0.65	4.189	7.14	19.28	5.00	0.030

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
6.40	1.89	64.67	1.00	64.67	13	396	0.65	4.202	7.11	19.28	5.00	0.053
6.45	1.89	64.91	1.00	64.91	13	398	0.65	4.141	6.97	19.28	5.00	0.027
6.51	1.89	65.25	1.00	65.25	13	400	0.65	4.103	6.86	19.28	5.00	0.038
6.58	1.89	65.74	1.00	65.74	13	403	0.65	4.063	6.74	19.28	5.00	0.046
6.63	1.89	66.39	1.23	81.73	16	408	0.65	3.790	4.83	19.28	5.00	0.027
6.70	1.89	67.23	1.23	82.77	17	413	0.65	3.570	4.48	19.28	5.00	0.041
6.77	1.89	68.32	1.23	84.08	17	419	0.65	3.309	4.08	19.28	4.57	0.039
6.86	1.89	69.36	1.23	85.09	17	423	0.65	3.261	3.97	19.28	4.45	0.049
6.91	1.88	70.55	1.22	86.22	17	426	0.65	3.129	3.76	19.28	4.21	0.022
6.99	1.87	71.24	1.22	86.82	17	428	0.65	3.158	3.78	19.28	4.23	0.041
7.04	1.87	71.49	1.22	87.02	17	430	0.65	3.145	3.75	19.28	4.20	0.029
7.09	1.87	71.12	1.22	86.68	17	430	0.65	3.206	3.84	19.28	4.30	0.025
7.19	1.88	70.55	1.22	86.06	17	430	0.65	3.334	4.03	19.28	4.51	0.052
7.23	1.87	70.01	1.22	85.37	17	428	0.65	3.522	4.30	19.28	4.81	0.022
7.32	1.87	69.34	1.22	84.56	17	427	0.65	3.748	4.62	19.28	5.00	0.055
7.36	1.88	68.73	1.22	83.85	17	424	0.65	3.960	4.93	19.28	5.00	0.023
7.43	1.88	68.36	1.22	83.44	17	424	0.65	4.070	5.10	19.28	5.00	0.042
7.51	1.88	68.22	1.00	68.22	14	426	0.65	4.116	6.57	19.28	5.00	0.047
7.55	1.87	69.05	1.00	69.05	14	429	0.65	3.957	6.24	19.28	5.00	0.026
7.63	1.86	70.03	1.00	70.03	14	435	0.65	3.718	5.78	19.28	5.00	0.049
7.69	1.86	71.71	1.21	86.57	17	445	0.65	3.232	3.90	19.28	4.37	0.030
7.78	1.86	73.00	1.21	88.29	17	457	0.65	2.781	3.28	19.28	3.67	0.041
7.83	1.87	74.39	1.21	90.25	18	470	0.65	2.359	2.70	19.28	3.02	0.017
7.87	1.88	75.11	1.22	91.67	18	481	0.65	2.060	2.30	19.28	2.58	0.014
7.95	1.88	75.46	1.23	92.46	18	489	0.64	1.903	2.10	19.28	2.35	0.021
8.01	1.87	77.15	1.22	93.89	19	496	0.64	1.773	1.93	19.28	2.16	0.017
8.14	1.86	77.77	1.21	93.95	19	498	0.64	1.809	1.98	19.28	2.22	0.035
8.24	1.84	79.03	1.19	94.43	19	501	0.64	1.801	1.97	19.28	2.21	0.027
8.29	1.84	79.32	1.19	94.38	18	502	0.64	1.824	2.00	19.28	2.24	0.012
8.34	1.81	81.40	1.17	94.88	18	503	0.64	1.826	2.02	19.28	2.26	0.012
8.44	1.79	82.71	1.00	82.71	16	504	0.64	1.870	2.46	19.28	2.75	0.035
8.47	1.77	84.45	1.00	84.45	16	504	0.64	1.882	2.43	19.28	2.72	0.010
8.56	1.76	85.22	1.00	85.22	16	508	0.64	1.860	2.38	19.28	2.67	0.028
8.64	1.76	86.06	1.00	86.06	16	512	0.64	1.818	2.31	19.28	2.59	0.024
8.71	1.75	86.68	1.00	86.68	16	515	0.64	1.785	2.25	19.28	2.52	0.020
8.75	1.75	87.03	1.00	87.03	17	517	0.64	1.758	2.21	19.28	2.47	0.013
8.80	1.75	86.99	1.00	86.99	17	519	0.64	1.757	2.21	19.28	2.47	0.015
8.87	1.76	86.71	1.00	86.71	17	523	0.64	1.698	2.14	19.28	2.39	0.019
8.94	1.76	86.15	1.00	86.15	16	525	0.64	1.714	2.17	19.28	2.43	0.020
9.03	1.76	85.36	1.00	85.36	16	521	0.64	1.849	2.37	19.28	2.65	0.029
9.05	1.75	84.98	1.00	84.98	16	515	0.64	1.996	2.58	19.28	2.89	0.009
9.15	1.76	84.80	1.00	84.80	16	518	0.64	1.986	2.57	19.28	2.88	0.033
9.21	1.77	85.21	1.00	85.21	16	528	0.64	1.795	2.30	19.28	2.57	0.018
9.25	1.78	85.77	1.00	85.77	16	539	0.64	1.607	2.03	19.28	2.28	0.012
9.34	1.79	85.95	1.00	85.95	17	549	0.64	1.473	1.85	19.28	2.07	0.021
9.41	1.79	87.19	1.14	99.44	19	560	0.64	1.325	1.40	19.28	1.56	0.013
9.52	1.79	88.17	1.14	100.44	19	569	0.64	1.245	1.30	19.28	1.45	0.019
9.56	1.78	89.56	1.13	101.49	20	577	0.64	1.168	1.20	19.28	1.35	0.006

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
9.62	1.79	89.82	1.14	102.12	20	582	0.64	1.125	1.15	19.28	1.29	0.009
9.65	1.79	90.33	1.14	103.00	20	588	0.64	1.070	1.08	19.28	1.21	0.006
9.75	1.79	90.31	1.15	103.43	20	594	0.64	1.041	1.04	19.28	1.17	0.014
9.80	1.80	90.25	1.15	103.85	20	598	0.64	1.014	1.01	19.28	1.13	0.006
9.85	1.80	89.97	1.16	104.06	20	601	0.64	0.998	0.99	19.28	1.11	0.006
9.94	1.81	89.56	1.16	103.98	20	604	0.64	0.997	0.99	19.28	1.11	0.012
10.02	1.81	89.31	1.16	103.78	20	606	0.64	1.005	1.00	19.28	1.12	0.012
10.04	1.81	89.29	1.17	104.12	20	609	0.64	0.982	0.97	19.28	1.09	0.002
10.13	1.82	88.61	1.18	104.16	20	613	0.64	0.965	0.95	19.28	1.06	0.012
10.22	1.83	87.85	1.19	104.17	20	618	0.64	0.944	0.92	19.28	1.03	0.010
10.26	1.84	87.16	1.19	104.00	20	620	0.64	0.936	0.91	19.28	1.02	0.004
10.31	1.85	86.06	1.20	103.60	20	623	0.64	0.927	0.90	19.28	1.01	0.006
10.39	1.87	84.62	1.22	102.85	20	626	0.64	0.925	0.90	19.28	1.01	0.011
10.44	1.88	83.19	1.23	102.03	20	627	0.64	0.925	0.91	19.28	1.01	0.006
10.53	1.90	81.90	1.23	101.06	20	628	0.64	0.939	0.93	19.28	1.04	0.011
10.58	1.91	80.56	1.24	100.06	20	628	0.64	0.954	0.95	19.28	1.06	0.006
10.67	1.92	79.25	1.25	98.94	20	627	0.64	0.982	0.98	19.28	1.10	0.012
10.72	1.93	78.19	1.25	98.01	20	626	0.64	1.008	1.02	19.28	1.14	0.006
10.77	1.93	77.60	1.26	97.46	20	625	0.64	1.026	1.04	19.28	1.17	0.008
10.84	1.93	77.18	1.26	97.05	20	626	0.64	1.039	1.06	19.28	1.19	0.010
10.90	1.93	77.03	1.26	96.86	20	627	0.64	1.047	1.07	19.28	1.20	0.010
11.04	1.93	76.61	1.26	96.35	20	628	0.64	1.072	1.10	19.28	1.23	0.019
11.05	1.93	76.43	1.26	96.21	20	628	0.64	1.074	1.11	19.28	1.24	0.002
11.11	1.94	75.78	1.26	95.65	19	629	0.64	1.088	1.13	19.28	1.26	0.010
11.18	1.94	75.07	1.27	95.03	19	629	0.64	1.105	1.15	19.28	1.29	0.010
11.22	1.95	74.31	1.27	94.36	19	628	0.64	1.127	1.18	19.28	1.32	0.006
11.36	1.96	73.35	1.27	93.48	19	629	0.64	1.156	1.22	19.28	1.36	0.023
11.41	1.96	72.76	1.28	92.99	19	629	0.64	1.168	1.24	19.28	1.38	0.008
11.45	1.97	72.27	1.28	92.58	19	629	0.64	1.179	1.25	19.28	1.40	0.006
11.51	1.97	71.61	1.28	92.01	19	629	0.64	1.198	1.28	19.28	1.43	0.010
11.56	1.98	70.86	1.29	91.37	19	629	0.64	1.219	1.31	19.28	1.46	0.009
11.64	1.98	69.97	1.30	90.61	19	628	0.64	1.245	1.35	19.28	1.51	0.015
11.69	1.99	68.98	1.30	89.79	19	627	0.64	1.277	1.39	19.28	1.56	0.009
11.76	2.00	67.62	1.31	88.60	18	624	0.64	1.333	1.47	19.28	1.64	0.013
11.83	2.02	65.70	1.32	86.90	18	618	0.64	1.431	1.60	19.28	1.79	0.015
11.92	2.04	63.67	1.34	85.34	18	615	0.64	1.516	1.72	19.28	1.92	0.021
11.95	2.06	62.12	1.36	84.55	18	614	0.64	1.536	1.74	19.28	1.95	0.008
12.01	2.06	61.25	1.36	83.38	18	608	0.64	1.664	1.92	19.28	2.15	0.015
12.10	2.05	60.81	1.35	82.35	17	602	0.64	1.809	2.13	19.28	2.38	0.025
12.15	2.02	62.26	1.33	82.60	17	599	0.64	1.889	2.24	19.28	2.51	0.016
12.24	2.01	64.07	1.31	84.14	18	608	0.64	1.751	2.05	19.28	2.29	0.025
12.27	2.02	63.85	1.32	84.36	18	613	0.64	1.676	1.95	19.28	2.18	0.008
12.37	2.04	62.46	1.34	83.68	18	617	0.64	1.665	1.93	19.28	2.16	0.027
12.42	2.03	63.25	1.34	84.67	18	625	0.64	1.549	1.77	19.28	1.99	0.011
12.47	2.01	66.52	1.31	87.39	18	639	0.64	1.377	1.54	19.28	1.72	0.010
12.57	1.99	68.96	1.30	89.44	19	650	0.64	1.274	1.40	19.28	1.56	0.020
12.61	1.99	69.12	1.30	89.67	19	653	0.64	1.251	1.37	19.28	1.53	0.006
12.69	1.99	68.77	1.30	89.44	19	655	0.64	1.248	1.36	19.28	1.53	0.015

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
12.74	1.99	68.91	1.30	89.64	19	659	0.64	1.227	1.34	19.28	1.50	0.010
12.80	1.99	69.91	1.30	90.63	19	666	0.64	1.172	1.27	19.28	1.42	0.010
12.87	1.97	72.96	1.28	93.46	19	680	0.64	1.052	1.10	19.28	1.24	0.011
12.97	1.94	76.48	1.27	96.80	20	698	0.64	0.931	0.95	19.28	1.06	0.012
13.02	1.91	81.15	1.25	101.14	20	718	0.64	0.813	0.79	19.28	0.89	0.005
13.07	1.89	86.04	1.23	105.60	21	738	0.64	0.710	0.67	19.28	0.75	0.004
13.16	1.86	90.46	1.21	109.42	22	757	0.64	0.636	0.58	19.28	0.65	0.007
13.19	1.84	94.87	1.19	112.97	22	773	0.64	0.574	0.51	19.28	0.57	0.002
13.29	1.82	97.27	1.18	114.60	22	784	0.64	0.549	0.48	19.28	0.54	0.007
13.34	1.81	99.46	1.17	115.95	23	791	0.64	0.529	0.46	19.28	0.51	0.003
13.39	1.81	100.32	1.16	116.31	23	794	0.64	0.525	0.45	19.28	0.51	0.003
13.48	1.80	100.27	1.16	116.00	22	795	0.64	0.532	0.46	19.28	0.52	0.006
13.53	1.80	100.12	1.15	115.50	22	792	0.64	0.545	0.48	19.28	0.53	0.003
13.61	1.80	99.51	1.15	114.75	22	789	0.64	0.562	0.50	19.28	0.56	0.005
13.67	1.80	98.60	1.16	113.91	22	786	0.64	0.582	0.52	19.28	0.58	0.004
13.71	1.81	96.94	1.16	112.60	22	779	0.64	0.611	0.55	19.28	0.62	0.004
13.80	1.82	93.98	1.18	110.51	22	771	0.64	0.656	0.60	19.28	0.67	0.007
13.85	1.85	89.45	1.20	107.23	21	758	0.64	0.722	0.68	19.28	0.76	0.005
13.95	1.88	84.94	1.22	103.70	21	748	0.64	0.788	0.76	19.28	0.85	0.010
13.99	1.91	80.30	1.24	99.84	20	737	0.64	0.861	0.86	19.28	0.96	0.005
14.08	1.93	77.70	1.26	97.60	20	732	0.64	0.905	0.92	19.28	1.03	0.012
14.17	1.95	75.46	1.27	95.72	20	730	0.64	0.938	0.97	19.28	1.08	0.012
14.22	1.95	75.43	1.27	95.86	20	734	0.64	0.918	0.94	19.28	1.05	0.006
14.30	1.95	75.49	1.27	96.06	20	739	0.64	0.898	0.92	19.28	1.03	0.009
14.31	1.95	76.44	1.27	97.11	20	746	0.64	0.856	0.86	19.28	0.97	0.002
14.39	1.95	77.00	1.27	97.70	20	752	0.64	0.833	0.84	19.28	0.94	0.008
14.45	1.94	77.88	1.27	98.61	20	759	0.64	0.804	0.80	19.28	0.89	0.007
14.54	1.94	78.51	1.26	99.22	20	764	0.64	0.787	0.78	19.28	0.87	0.009
14.59	1.94	79.53	1.26	100.22	20	771	0.64	0.761	0.74	19.28	0.83	0.005
14.64	1.93	81.62	1.25	102.41	21	785	0.64	0.700	0.67	19.28	0.75	0.005
14.72	1.91	85.00	1.24	105.78	21	805	0.64	0.626	0.58	19.28	0.65	0.007
14.79	1.89	89.99	1.23	110.55	22	830	0.64	0.545	0.48	19.28	0.54	0.004
14.86	1.87	94.43	1.21	114.56	23	851	0.64	0.490	0.42	19.28	0.47	0.004
14.91	1.84	100.32	1.19	119.54	23	877	0.64	0.431	0.36	19.28	0.40	0.002
15.00	1.82	105.62	1.17	123.62	24	902	0.64	0.387	0.31	19.28	0.35	0.004
15.05	1.79	110.80	1.15	127.02	25	923	0.64	0.353	0.28	19.28	0.31	0.002
15.10	1.79	113.01	1.14	128.33	25	933	0.63	0.340	0.26	19.28	0.30	0.002
15.17	1.79	112.88	1.14	128.38	25	936	0.63	0.339	0.26	19.28	0.29	0.003
15.23	1.80	110.92	1.15	127.45	25	933	0.63	0.347	0.27	19.28	0.30	0.002
15.29	1.81	107.20	1.16	124.88	24	919	0.63	0.373	0.30	19.28	0.33	0.002
15.37	1.82	102.15	1.17	119.66	23	885	0.63	0.446	0.37	19.28	0.42	0.004
15.42	1.83	96.40	1.18	113.84	22	847	0.63	0.551	0.49	19.28	0.54	0.003
15.49	1.84	90.65	1.20	108.41	21	815	0.63	0.672	0.62	19.28	0.70	0.006
15.55	1.88	85.52	1.22	104.60	21	805	0.63	0.724	0.69	19.28	0.77	0.006
15.64	1.94	77.84	1.26	98.29	20	790	0.63	0.807	0.81	19.28	0.90	0.010
15.69	1.97	74.21	1.29	95.36	20	785	0.63	0.841	0.86	19.28	0.96	0.006
15.78	1.99	72.01	1.30	93.66	19	784	0.63	0.861	0.89	19.28	1.00	0.010
15.82	1.98	73.83	1.29	95.42	20	795	0.63	0.808	0.82	19.28	0.92	0.005

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
15.91	2.00	72.59	1.30	94.68	20	799	0.63	0.799	0.81	19.28	0.91	0.010
15.97	2.02	70.84	1.32	93.69	20	802	0.63	0.791	0.81	19.28	0.91	0.006
16.01	2.04	68.63	1.35	92.41	20	802	0.63	0.797	0.82	19.28	0.92	0.005
16.11	2.06	66.47	1.37	91.20	19	802	0.63	0.810	0.84	19.28	0.94	0.011
16.16	2.09	64.36	1.40	90.16	19	800	0.63	0.828	0.86	19.28	0.96	0.006
16.23	2.10	62.76	1.42	89.39	19	798	0.63	0.848	0.88	19.28	0.99	0.009
16.29	2.12	61.33	1.45	88.65	19	795	0.63	0.874	0.91	19.28	1.02	0.008
16.36	2.13	60.33	1.46	87.99	19	792	0.63	0.903	0.95	19.28	1.06	0.009
16.43	2.13	59.79	1.46	87.45	19	789	0.63	0.929	0.98	19.28	1.10	0.009
16.52	2.13	59.58	1.46	87.04	19	788	0.63	0.949	1.01	19.28	1.13	0.012
16.56	2.12	59.90	1.45	86.93	19	788	0.63	0.956	1.02	19.28	1.14	0.006
16.61	2.11	60.81	1.43	87.06	19	790	0.63	0.954	1.02	19.28	1.14	0.007
16.70	2.10	61.73	1.41	87.24	19	792	0.63	0.952	1.02	19.28	1.14	0.012
16.74	2.09	62.12	1.41	87.29	19	793	0.63	0.954	1.03	19.28	1.15	0.005
16.84	2.10	61.46	1.41	86.84	19	793	0.63	0.971	1.05	19.28	1.17	0.014
16.89	2.10	60.67	1.42	86.37	19	791	0.63	0.989	1.07	19.28	1.20	0.007
16.93	2.11	59.93	1.43	85.93	19	789	0.63	1.008	1.09	19.28	1.23	0.006
17.02	2.11	59.53	1.44	85.61	19	789	0.63	1.022	1.11	19.28	1.25	0.014
17.07	2.10	60.73	1.42	86.17	19	794	0.63	0.997	1.08	19.28	1.21	0.007
17.16	2.06	65.11	1.37	88.97	19	813	0.63	0.895	0.95	19.28	1.07	0.011
17.26	2.02	70.42	1.32	93.16	20	837	0.63	0.781	0.80	19.28	0.90	0.011
17.35	1.99	75.04	1.30	97.19	20	858	0.63	0.699	0.69	19.28	0.78	0.008
17.39	1.97	77.22	1.29	99.33	20	872	0.63	0.652	0.63	19.28	0.71	0.004
17.49	1.97	78.36	1.28	100.51	21	882	0.63	0.625	0.60	19.28	0.67	0.008
17.54	1.97	79.22	1.28	101.57	21	893	0.63	0.595	0.57	19.28	0.63	0.004
17.63	1.97	79.77	1.28	102.29	21	902	0.63	0.574	0.54	19.28	0.61	0.007
17.67	1.97	80.74	1.28	103.46	21	912	0.63	0.547	0.51	19.28	0.57	0.003
17.74	1.96	81.98	1.28	104.84	22	925	0.63	0.519	0.48	19.28	0.53	0.004
17.80	1.96	83.39	1.28	106.45	22	939	0.63	0.489	0.44	19.28	0.49	0.004
17.86	1.96	84.64	1.28	108.06	22	955	0.63	0.457	0.40	19.28	0.45	0.003
17.95	1.96	85.21	1.28	109.03	22	969	0.63	0.433	0.38	19.28	0.42	0.005
17.99	1.97	85.13	1.29	109.59	23	983	0.63	0.409	0.35	19.28	0.40	0.002
18.08	1.99	84.44	1.30	109.45	23	993	0.63	0.397	0.34	19.28	0.38	0.004
18.13	2.00	84.03	1.30	109.58	23	1002	0.63	0.384	0.33	19.28	0.37	0.002
18.18	2.00	83.78	1.31	109.76	23	1010	0.63	0.374	0.32	19.28	0.36	0.002
18.27	2.01	83.68	1.32	110.10	23	1020	0.63	0.362	0.31	19.28	0.34	0.003
18.32	2.01	84.46	1.32	111.28	23	1034	0.63	0.345	0.29	19.28	0.32	0.002
18.41	2.01	85.80	1.31	112.74	24	1049	0.63	0.330	0.27	19.28	0.30	0.003
18.46	1.99	88.80	1.30	115.55	24	1066	0.63	0.310	0.25	19.28	0.28	0.002
18.54	1.98	92.39	1.29	119.12	25	1091	0.63	0.287	0.22	19.28	0.25	0.003
18.64	1.96	95.74	1.28	122.58	25	1117	0.63	0.265	0.20	19.28	0.23	0.003
18.67	1.95	98.02	1.27	124.74	26	1128	0.63	0.256	0.19	19.28	0.21	0.001
18.73	1.94	98.33	1.26	124.20	25	1114	0.63	0.271	0.20	19.28	0.23	0.002
18.78	1.93	97.74	1.26	122.72	25	1093	0.63	0.293	0.23	19.28	0.25	0.002
18.86	1.93	95.99	1.25	120.35	24	1072	0.63	0.319	0.25	19.28	0.28	0.002
18.91	1.93	93.73	1.26	117.91	24	1058	0.63	0.339	0.27	19.28	0.31	0.002
18.97	1.96	86.96	1.28	111.11	23	1020	0.64	0.396	0.34	19.28	0.38	0.002
19.05	1.96	84.02	1.28	107.24	22	986	0.64	0.464	0.41	19.28	0.46	0.005

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
19.12	1.95	82.02	1.27	104.01	21	950	0.64	0.553	0.52	19.28	0.58	0.005
19.18	1.90	85.13	1.24	105.35	21	931	0.64	0.614	0.58	19.28	0.64	0.004
19.23	1.88	85.38	1.22	104.16	21	905	0.64	0.707	0.68	19.28	0.76	0.004
19.30	1.85	85.50	1.20	102.75	20	880	0.64	0.817	0.81	19.28	0.90	0.008
19.37	1.82	85.82	1.18	100.92	20	853	0.64	0.970	0.99	19.28	1.11	0.008
19.46	1.80	85.83	1.15	98.86	19	830	0.64	1.132	1.20	19.28	1.34	0.015
19.55	1.78	85.70	1.00	85.70	16	813	0.64	1.280	1.62	19.28	1.81	0.020
19.59	1.78	85.53	1.00	85.53	16	805	0.64	1.362	1.73	19.28	1.94	0.010
19.63	1.78	84.36	1.00	84.36	16	795	0.64	1.466	1.89	19.28	2.12	0.008
19.72	1.79	81.96	1.00	81.96	16	784	0.64	1.604	2.14	19.28	2.39	0.027
19.78	1.81	78.71	1.00	78.71	15	775	0.64	1.733	2.40	19.28	2.69	0.020
19.84	1.83	75.62	1.00	75.62	15	772	0.64	1.782	2.56	19.28	2.87	0.020
19.91	1.86	73.13	1.21	88.18	17	771	0.64	1.813	2.14	19.28	2.40	0.022
19.96	1.88	71.19	1.22	86.87	17	771	0.64	1.830	2.18	19.28	2.45	0.013
20.06	1.90	69.49	1.23	85.72	17	774	0.64	1.817	2.19	19.28	2.45	0.028
20.14	1.91	68.13	1.25	84.82	17	777	0.64	1.786	2.16	19.28	2.42	0.023
20.16	1.93	67.01	1.26	84.27	17	784	0.64	1.701	2.05	19.28	2.30	0.005
20.24	1.95	65.95	1.27	83.73	17	792	0.64	1.628	1.96	19.28	2.20	0.022
20.29	1.98	64.60	1.29	83.19	17	803	0.64	1.511	1.82	19.28	2.03	0.011
20.38	2.00	63.35	1.31	82.72	17	814	0.64	1.422	1.70	19.28	1.91	0.022
20.42	2.03	62.44	1.33	83.13	17	834	0.64	1.251	1.47	19.28	1.65	0.008
20.52	2.05	62.03	1.35	83.88	18	853	0.64	1.120	1.29	19.28	1.44	0.016
20.54	2.06	61.78	1.37	84.66	18	868	0.64	1.026	1.16	19.28	1.30	0.004
20.61	2.08	61.35	1.39	85.03	18	879	0.64	0.974	1.09	19.28	1.22	0.010
20.70	2.08	61.37	1.39	85.47	18	887	0.64	0.937	1.04	19.28	1.16	0.012
20.75	2.09	61.07	1.40	85.61	18	892	0.64	0.916	1.01	19.28	1.13	0.007
20.83	2.09	60.52	1.41	85.21	18	892	0.64	0.930	1.03	19.28	1.15	0.011
20.89	2.11	59.29	1.43	84.87	18	893	0.64	0.926	1.02	19.28	1.14	0.007
20.96	2.13	57.80	1.47	84.89	19	899	0.64	0.904	0.99	19.28	1.10	0.010
21.02	2.16	55.74	1.52	84.81	19	901	0.64	0.901	0.97	19.28	1.09	0.007
21.07	2.19	53.15	1.59	84.44	19	896	0.64	0.934	1.00	19.28	1.11	0.007
21.16	2.22	50.58	1.67	84.23	19	890	0.65	0.979	1.03	19.28	1.15	0.012
21.20	2.25	48.57	1.75	85.22	20	891	0.65	0.978	1.00	19.28	1.12	0.006
21.30	2.28	47.19	1.84	86.63	20	897	0.65	0.954	0.94	19.28	1.06	0.012
21.34	2.29	46.71	1.89	88.19	21	907	0.65	0.909	0.87	19.28	0.98	0.004
21.39	2.29	46.98	1.89	89.01	21	916	0.65	0.870	0.83	19.28	0.93	0.006
21.47	2.29	47.66	1.88	89.48	21	925	0.65	0.833	0.79	19.28	0.88	0.008
21.53	2.27	49.29	1.82	89.69	21	938	0.65	0.784	0.75	19.28	0.84	0.006
21.62	2.25	50.98	1.76	89.48	21	948	0.65	0.754	0.73	19.28	0.81	0.009
21.66	2.22	52.95	1.68	88.84	20	952	0.65	0.741	0.73	19.28	0.82	0.004
21.75	2.21	53.62	1.63	87.46	20	945	0.65	0.778	0.79	19.28	0.88	0.009
21.81	2.19	53.79	1.60	85.92	19	933	0.65	0.834	0.87	19.28	0.97	0.007
21.85	2.19	52.84	1.60	84.34	19	917	0.65	0.913	0.97	19.28	1.09	0.005
21.95	2.19	51.97	1.60	83.15	19	906	0.65	0.979	1.06	19.28	1.19	0.014
21.99	2.19	51.37	1.60	82.02	18	895	0.65	1.047	1.15	19.28	1.29	0.007
22.08	2.18	51.31	1.57	80.43	18	882	0.65	1.143	1.30	19.28	1.45	0.016
22.13	2.15	52.82	1.51	79.81	18	878	0.65	1.181	1.37	19.28	1.53	0.009
22.22	2.13	54.84	1.47	80.34	18	884	0.65	1.149	1.34	19.28	1.50	0.017

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
22.26	2.10	57.91	1.42	82.06	18	898	0.65	1.063	1.23	19.28	1.37	0.006
22.32	2.08	60.75	1.39	84.59	18	922	0.65	0.937	1.05	19.28	1.18	0.008
22.41	2.07	63.36	1.37	86.97	19	944	0.65	0.841	0.92	19.28	1.03	0.012
22.45	2.07	64.97	1.37	89.17	19	969	0.65	0.744	0.79	19.28	0.88	0.005
22.51	2.06	66.63	1.37	91.19	19	991	0.65	0.673	0.70	19.28	0.78	0.006
22.61	2.06	67.76	1.37	92.85	20	1013	0.65	0.615	0.62	19.28	0.70	0.008
22.65	2.06	69.33	1.37	95.02	20	1038	0.65	0.554	0.54	19.28	0.61	0.003
22.71	2.06	70.82	1.37	96.96	21	1060	0.65	0.507	0.49	19.28	0.55	0.004
22.80	2.05	73.36	1.36	99.49	21	1084	0.65	0.464	0.43	19.28	0.49	0.005
22.84	2.00	82.27	1.30	107.36	22	1133	0.65	0.388	0.34	19.28	0.38	0.002
22.93	1.94	92.79	1.26	117.03	24	1182	0.65	0.331	0.27	19.28	0.30	0.003
22.97	1.84	112.00	1.19	133.55	26	1254	0.65	0.266	0.19	19.28	0.22	0.001
23.07	1.76	129.97	1.10	143.19	27	1312	0.65	0.229	0.16	19.28	0.18	0.002
23.12	1.69	148.82	1.00	148.82	28	1367	0.65	0.200	0.14	19.28	0.15	0.001
23.19	1.65	160.60	1.00	160.60	29	1401	0.65	0.186	0.12	19.28	0.13	0.001
23.25	1.62	168.81	1.00	168.81	31	1423	0.65	0.178	0.11	19.28	0.12	0.001
23.30	1.60	174.39	1.00	174.39	31	1434	0.65	0.174	0.10	19.28	0.11	0.001
23.37	1.59	177.60	1.00	177.60	32	1438	0.65	0.174	0.10	19.28	0.11	0.001
23.44	1.58	178.82	1.00	178.82	32	1436	0.65	0.176	0.10	19.28	0.11	0.001
23.49	1.57	179.65	1.00	179.65	32	1428	0.65	0.180	0.10	19.28	0.11	0.001
23.57	1.57	181.31	1.00	181.31	32	1440	0.65	0.176	0.10	19.28	0.11	0.001
23.63	1.57	184.57	1.00	184.57	33	1476	0.66	0.164	0.09	19.28	0.10	0.001
23.72	1.58	188.11	1.00	188.11	34	1513	0.66	0.153	0.08	19.28	0.09	0.001
23.81	1.57	190.66	1.00	190.66	34	1527	0.66	0.150	0.08	19.28	0.09	0.001
23.85	1.56	192.93	1.00	192.93	34	1517	0.66	0.154	0.08	19.28	0.09	0.000
23.90	1.55	194.99	1.00	194.99	35	1513	0.66	0.155	0.08	19.28	0.09	0.001
23.95	1.53	199.01	1.00	199.01	35	1514	0.66	0.156	0.08	19.28	0.09	0.001
24.03	1.51	205.48	1.00	205.48	36	1526	0.66	0.153	0.08	19.28	0.08	0.001
24.09	1.54	216.22	1.00	216.22	38	1669	0.66	0.120	0.06	19.28	0.06	0.000
24.18	1.61	226.42	1.00	226.42	41	1919	0.66	0.085	0.04	19.28	0.04	0.000
24.22	1.72	231.46	1.03	239.28	45	2281	0.66	0.059	0.02	19.28	0.03	0.000
24.35	1.78	231.31	1.13	262.06	50	2485	0.66	0.051	0.02	19.28	0.02	0.000
24.37	1.79	231.97	1.14	263.65	51	2502	0.66	0.050	0.02	19.28	0.02	0.000
24.41	1.75	236.39	1.09	257.44	49	2440	0.66	0.053	0.02	19.28	0.02	0.000
24.50	1.73	241.92	1.04	252.28	48	2415	0.66	0.054	0.02	19.28	0.02	0.000
24.55	1.73	247.26	1.05	260.74	49	2492	0.66	0.051	0.02	19.28	0.02	0.000
24.64	1.73	247.89	1.05	259.16	49	2488	0.66	0.052	0.02	19.28	0.02	0.000
24.69	1.68	259.55	1.00	259.55	48	2448	0.66	0.054	0.02	19.28	0.02	0.000
24.77	1.65	263.68	1.00	263.68	48	2381	0.66	0.057	0.02	19.28	0.02	0.000
24.81	1.73	227.41	1.04	237.22	45	2288	0.66	0.061	0.02	19.28	0.03	0.000
24.89	1.93	164.13	1.26	206.01	42	2176	0.66	0.068	0.03	19.28	0.03	0.000
24.96	2.18	109.09	1.56	170.10	38	2036	0.66	0.079	0.04	19.28	0.04	0.000
25.01	2.25	95.86	1.75	167.92	39	1979	0.66	0.084	0.04	19.28	0.04	0.000
25.10	2.22	96.37	1.66	159.65	36	1907	0.66	0.092	0.05	19.28	0.05	0.001
25.15	2.14	94.15	1.49	140.54	31	1690	0.66	0.125	0.07	19.28	0.08	0.000
25.23	2.06	90.38	1.37	123.83	26	1456	0.66	0.195	0.14	19.28	0.16	0.002
25.26	1.98	87.64	1.29	113.40	23	1266	0.66	0.316	0.26	19.28	0.29	0.001
25.33	1.97	86.97	1.28	111.76	23	1238	0.66	0.346	0.29	19.28	0.33	0.003

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
25.42	1.95	86.98	1.27	110.31	23	1200	0.66	0.393	0.34	19.28	0.38	0.004
25.47	1.93	86.99	1.26	109.29	22	1173	0.66	0.433	0.38	19.28	0.43	0.003
25.57	1.92	87.26	1.25	109.14	22	1166	0.66	0.448	0.40	19.28	0.45	0.006
25.61	1.92	89.03	1.25	111.36	22	1191	0.66	0.413	0.36	19.28	0.40	0.002
25.70	1.92	90.98	1.25	113.87	23	1221	0.66	0.376	0.32	19.28	0.36	0.004
25.75	1.93	91.67	1.26	115.40	23	1251	0.66	0.344	0.28	19.28	0.32	0.002
25.80	1.94	92.02	1.27	116.41	24	1274	0.66	0.323	0.26	19.28	0.29	0.002
25.87	1.95	92.82	1.27	117.90	24	1300	0.66	0.300	0.24	19.28	0.27	0.002
25.94	1.95	95.10	1.27	120.75	25	1333	0.66	0.276	0.21	19.28	0.24	0.002
25.99	1.96	96.86	1.27	123.34	25	1370	0.66	0.252	0.19	19.28	0.21	0.001
26.07	1.96	97.84	1.28	125.11	26	1401	0.66	0.235	0.17	19.28	0.19	0.002
26.13	1.97	99.03	1.28	127.08	26	1432	0.66	0.219	0.16	19.28	0.18	0.001
26.21	1.97	100.05	1.28	128.56	26	1455	0.66	0.209	0.15	19.28	0.17	0.002
26.26	1.97	100.98	1.29	130.01	27	1477	0.66	0.200	0.14	19.28	0.16	0.001
26.35	1.98	101.30	1.29	130.73	27	1493	0.66	0.194	0.14	19.28	0.15	0.002
26.38	1.98	101.88	1.29	131.72	27	1510	0.66	0.188	0.13	19.28	0.15	0.001
26.47	1.98	103.22	1.29	133.14	27	1524	0.66	0.184	0.13	19.28	0.14	0.002
26.54	1.97	105.08	1.28	134.89	28	1537	0.66	0.180	0.12	19.28	0.14	0.001
26.58	1.95	108.28	1.27	137.50	28	1542	0.66	0.179	0.12	19.28	0.13	0.001
26.68	1.93	111.78	1.25	140.26	28	1546	0.66	0.178	0.12	19.28	0.13	0.002
26.72	1.90	116.43	1.24	144.04	29	1553	0.66	0.177	0.11	19.28	0.13	0.001
26.82	1.88	119.37	1.23	146.43	29	1562	0.67	0.175	0.11	19.28	0.12	0.001
26.86	1.87	121.43	1.22	148.05	29	1567	0.67	0.173	0.11	19.28	0.12	0.001
26.91	1.87	122.07	1.21	148.13	29	1560	0.67	0.176	0.11	19.28	0.12	0.001
26.99	1.85	122.93	1.20	147.51	29	1537	0.67	0.186	0.12	19.28	0.13	0.001
27.04	1.81	126.57	1.16	146.72	28	1491	0.67	0.205	0.13	19.28	0.15	0.001
27.19	1.76	130.35	1.11	144.18	28	1453	0.67	0.225	0.15	19.28	0.17	0.003
27.22	1.73	134.59	1.05	141.15	27	1432	0.67	0.237	0.17	19.28	0.19	0.001
27.24	1.72	137.27	1.03	140.96	26	1439	0.67	0.233	0.17	19.28	0.19	0.000
27.32	1.71	140.59	1.00	140.59	26	1449	0.67	0.229	0.17	19.28	0.18	0.002
27.37	1.69	143.92	1.00	143.92	27	1454	0.67	0.227	0.16	19.28	0.18	0.001
27.43	1.67	146.04	1.00	146.04	27	1445	0.67	0.233	0.16	19.28	0.18	0.001
27.53	1.66	147.08	1.00	147.08	27	1429	0.67	0.243	0.17	19.28	0.19	0.002
27.57	1.64	148.09	1.00	148.09	27	1405	0.67	0.258	0.18	19.28	0.20	0.001
27.66	1.63	148.63	1.00	148.63	27	1390	0.67	0.270	0.19	19.28	0.21	0.002
27.71	1.62	148.95	1.00	148.95	27	1377	0.67	0.280	0.19	19.28	0.22	0.001
27.76	1.61	148.96	1.00	148.96	27	1366	0.67	0.289	0.20	19.28	0.23	0.001
27.85	1.61	148.69	1.00	148.69	27	1353	0.67	0.301	0.21	19.28	0.24	0.003
27.89	1.59	149.02	1.00	149.02	27	1335	0.67	0.317	0.22	19.28	0.25	0.001
27.99	1.58	149.22	1.00	149.22	27	1323	0.67	0.329	0.23	19.28	0.26	0.003
28.04	1.58	149.20	1.00	149.20	27	1314	0.67	0.339	0.24	19.28	0.27	0.002
28.09	1.58	147.82	1.00	147.82	26	1306	0.67	0.348	0.25	19.28	0.28	0.002
28.17	1.59	145.68	1.00	145.68	26	1299	0.67	0.357	0.26	19.28	0.29	0.003
28.22	1.60	142.53	1.00	142.53	26	1291	0.67	0.367	0.27	19.28	0.30	0.002
28.29	1.61	139.24	1.00	139.24	25	1284	0.67	0.376	0.29	19.28	0.32	0.002
28.36	1.63	134.88	1.00	134.88	25	1278	0.67	0.385	0.30	19.28	0.34	0.003
28.45	1.65	130.90	1.00	130.90	24	1279	0.67	0.386	0.31	19.28	0.35	0.004
28.50	1.68	127.15	1.00	127.15	24	1287	0.67	0.378	0.31	19.28	0.35	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
28.54	1.70	123.54	1.00	123.54	23	1301	0.67	0.364	0.31	19.28	0.34	0.002
28.63	1.73	119.59	1.05	126.05	24	1313	0.67	0.353	0.29	19.28	0.32	0.003
28.68	1.76	115.74	1.10	127.60	24	1325	0.67	0.343	0.27	19.28	0.30	0.002
28.77	1.78	113.63	1.13	127.88	25	1333	0.67	0.337	0.26	19.28	0.30	0.003
28.82	1.78	114.10	1.13	128.85	25	1345	0.67	0.327	0.25	19.28	0.28	0.001
28.90	1.77	117.34	1.11	130.79	25	1365	0.67	0.312	0.24	19.28	0.27	0.003
28.96	1.75	124.33	1.08	134.22	25	1402	0.67	0.284	0.21	19.28	0.24	0.002
29.05	1.73	132.47	1.05	139.71	26	1467	0.67	0.245	0.18	19.28	0.20	0.002
29.10	1.72	139.53	1.03	143.59	27	1518	0.67	0.220	0.15	19.28	0.17	0.001
29.14	1.70	144.26	1.00	144.26	27	1528	0.67	0.216	0.15	19.28	0.17	0.001
29.20	1.68	146.75	1.00	146.75	27	1507	0.67	0.226	0.16	19.28	0.18	0.001
29.28	1.67	146.67	1.00	146.67	27	1490	0.67	0.236	0.16	19.28	0.18	0.002
29.37	1.68	144.81	1.00	144.81	27	1495	0.67	0.235	0.17	19.28	0.18	0.002
29.41	1.72	134.34	1.04	139.48	26	1480	0.67	0.243	0.18	19.28	0.20	0.001
29.46	1.73	131.26	1.06	138.71	26	1468	0.67	0.251	0.18	19.28	0.20	0.001
29.55	1.74	128.81	1.07	137.29	26	1453	0.67	0.261	0.19	19.28	0.21	0.002
29.60	1.70	135.30	1.00	135.30	25	1446	0.67	0.266	0.20	19.28	0.23	0.001
29.69	1.69	136.89	1.00	136.89	25	1435	0.67	0.275	0.21	19.28	0.23	0.002
29.73	1.67	139.69	1.00	139.69	26	1428	0.67	0.280	0.21	19.28	0.23	0.001
29.82	1.65	142.52	1.00	142.52	26	1433	0.67	0.278	0.20	19.28	0.23	0.002
29.87	1.65	143.93	1.00	143.93	26	1444	0.67	0.271	0.19	19.28	0.22	0.001
29.95	1.67	142.45	1.00	142.45	26	1461	0.67	0.262	0.19	19.28	0.21	0.002
30.01	1.69	139.44	1.00	139.44	26	1477	0.67	0.254	0.19	19.28	0.21	0.002
30.05	1.72	134.71	1.03	138.37	26	1490	0.67	0.247	0.18	19.28	0.20	0.001
30.15	1.75	129.56	1.08	140.24	27	1498	0.67	0.244	0.17	19.28	0.19	0.002
30.20	1.78	123.21	1.13	139.62	27	1499	0.67	0.244	0.17	19.28	0.19	0.001
30.29	1.81	118.39	1.16	137.61	27	1495	0.67	0.247	0.18	19.28	0.20	0.002
30.33	1.84	113.54	1.19	134.99	26	1492	0.67	0.249	0.18	19.28	0.20	0.001
30.43	1.86	109.29	1.21	132.24	26	1493	0.67	0.250	0.18	19.28	0.20	0.002
30.52	1.89	105.22	1.23	129.25	26	1493	0.67	0.251	0.19	19.28	0.21	0.002
30.54	1.91	101.73	1.24	126.42	25	1490	0.67	0.254	0.19	19.28	0.21	0.001
30.61	1.92	98.82	1.25	123.85	25	1483	0.67	0.259	0.20	19.28	0.22	0.002
30.66	1.95	94.26	1.27	119.69	24	1467	0.67	0.269	0.21	19.28	0.24	0.001
30.75	1.98	89.34	1.29	115.22	24	1450	0.67	0.282	0.23	19.28	0.26	0.003
30.80	2.01	84.64	1.31	111.17	23	1434	0.67	0.293	0.25	19.28	0.27	0.002
30.84	2.04	80.39	1.34	107.86	23	1424	0.67	0.301	0.26	19.28	0.29	0.002
30.94	2.06	77.24	1.37	105.65	23	1418	0.67	0.307	0.27	19.28	0.30	0.003
30.97	2.09	73.84	1.40	103.63	22	1412	0.67	0.312	0.27	19.28	0.31	0.001
31.07	2.11	71.78	1.43	102.66	22	1412	0.67	0.314	0.28	19.28	0.31	0.004
31.12	2.12	70.35	1.45	102.34	22	1417	0.67	0.312	0.27	19.28	0.31	0.002
31.21	2.12	70.76	1.46	103.00	23	1429	0.67	0.304	0.26	19.28	0.30	0.003
31.26	2.11	72.80	1.44	104.56	23	1446	0.67	0.292	0.25	19.28	0.28	0.002
31.31	2.09	75.80	1.41	106.75	23	1468	0.67	0.278	0.23	19.28	0.26	0.002
31.39	2.08	78.50	1.39	108.88	23	1488	0.67	0.266	0.22	19.28	0.25	0.002
31.44	2.07	80.26	1.38	110.48	24	1505	0.67	0.257	0.21	19.28	0.24	0.001
31.54	2.07	80.86	1.37	111.00	24	1513	0.67	0.254	0.21	19.28	0.23	0.003
31.57	2.06	81.12	1.37	111.03	24	1511	0.67	0.256	0.21	19.28	0.23	0.001
31.63	2.05	81.73	1.36	111.03	24	1505	0.67	0.260	0.21	19.28	0.24	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
31.72	2.03	83.02	1.34	111.17	23	1492	0.67	0.269	0.22	19.28	0.25	0.003
31.79	2.01	85.23	1.32	112.34	23	1485	0.67	0.274	0.23	19.28	0.25	0.002
31.86	1.99	87.40	1.30	113.78	24	1484	0.67	0.276	0.23	19.28	0.25	0.002
31.89	1.97	90.71	1.29	116.74	24	1498	0.67	0.267	0.21	19.28	0.24	0.001
32.00	1.96	92.95	1.28	118.74	24	1509	0.67	0.262	0.21	19.28	0.23	0.003
32.04	1.95	94.81	1.27	120.45	25	1518	0.67	0.258	0.20	19.28	0.22	0.001
32.13	1.94	95.97	1.27	121.50	25	1525	0.67	0.255	0.20	19.28	0.22	0.002
32.15	1.93	98.29	1.26	123.68	25	1537	0.67	0.249	0.19	19.28	0.21	0.001
32.22	1.92	101.02	1.25	126.31	26	1554	0.67	0.241	0.18	19.28	0.20	0.002
32.29	1.91	103.38	1.25	128.76	26	1575	0.67	0.231	0.17	19.28	0.19	0.002
32.40	1.91	104.84	1.24	130.43	26	1596	0.67	0.223	0.16	19.28	0.18	0.002
32.50	1.91	105.74	1.24	131.47	26	1610	0.67	0.218	0.16	19.28	0.17	0.002
32.59	1.91	105.78	1.24	131.62	26	1617	0.67	0.216	0.15	19.28	0.17	0.002
32.64	1.91	105.42	1.25	131.37	26	1619	0.67	0.216	0.15	19.28	0.17	0.001
32.68	1.92	104.76	1.25	130.88	26	1622	0.67	0.215	0.15	19.28	0.17	0.001
32.77	1.93	103.12	1.26	129.50	26	1623	0.67	0.216	0.16	19.28	0.17	0.002
32.82	1.95	99.67	1.27	126.51	26	1616	0.67	0.219	0.16	19.28	0.18	0.001
32.91	1.97	96.00	1.28	123.23	25	1607	0.67	0.224	0.17	19.28	0.19	0.002
32.96	1.99	92.73	1.30	120.28	25	1594	0.67	0.230	0.18	19.28	0.20	0.001
33.04	2.00	91.10	1.30	118.75	25	1587	0.67	0.234	0.18	19.28	0.20	0.002
33.09	2.00	89.94	1.31	117.74	25	1584	0.67	0.236	0.18	19.28	0.21	0.001
33.14	2.01	88.27	1.32	116.50	24	1585	0.67	0.236	0.19	19.28	0.21	0.001
33.23	2.03	85.95	1.34	114.79	24	1584	0.67	0.237	0.19	19.28	0.21	0.002
33.28	2.05	82.80	1.36	112.50	24	1577	0.67	0.241	0.19	19.28	0.22	0.001
33.37	2.07	79.60	1.38	110.20	24	1567	0.67	0.247	0.20	19.28	0.23	0.002
33.41	2.10	75.68	1.42	107.56	23	1551	0.67	0.256	0.21	19.28	0.24	0.001
33.51	2.13	72.15	1.46	105.41	23	1536	0.67	0.266	0.22	19.28	0.25	0.003
33.55	2.15	68.99	1.50	103.77	23	1521	0.67	0.275	0.23	19.28	0.26	0.001
33.60	2.17	66.50	1.54	102.53	23	1508	0.67	0.283	0.24	19.28	0.27	0.002
33.73	2.19	64.13	1.58	101.52	23	1498	0.67	0.292	0.25	19.28	0.28	0.004
33.75	2.21	61.92	1.64	101.25	23	1493	0.67	0.296	0.25	19.28	0.28	0.001
33.83	2.23	59.98	1.69	101.65	23	1496	0.67	0.295	0.25	19.28	0.28	0.003
33.87	2.28	55.63	1.85	102.78	24	1490	0.67	0.299	0.24	19.28	0.27	0.001
33.97	2.34	50.67	2.07	104.65	25	1477	0.67	0.309	0.24	19.28	0.27	0.003
34.02	2.39	46.52	2.30	106.92	26	1461	0.67	0.322	0.23	19.28	0.26	0.001
34.07	2.40	45.14	2.37	106.94	26	1448	0.67	0.333	0.24	19.28	0.27	0.002
34.16	2.40	44.80	2.36	105.76	26	1437	0.67	0.344	0.25	19.28	0.28	0.003
34.20	2.39	44.81	2.32	103.76	25	1420	0.67	0.360	0.27	19.28	0.30	0.002
34.29	2.37	45.74	2.21	101.14	25	1408	0.67	0.373	0.29	19.28	0.33	0.004
34.34	2.33	47.93	2.04	97.70	23	1395	0.67	0.386	0.32	19.28	0.36	0.002
34.39	2.27	50.91	1.83	93.39	22	1371	0.67	0.413	0.37	19.28	0.42	0.002
34.46	2.22	53.95	1.66	89.36	20	1335	0.67	0.459	0.45	19.28	0.50	0.004
34.52	2.16	56.70	1.52	86.44	19	1294	0.67	0.520	0.55	19.28	0.61	0.005
34.61	2.12	58.55	1.45	85.18	19	1267	0.67	0.570	0.62	19.28	0.70	0.007
34.66	2.10	59.90	1.41	84.62	18	1247	0.67	0.611	0.68	19.28	0.76	0.004
34.75	2.08	60.94	1.39	84.44	18	1234	0.67	0.641	0.72	19.28	0.81	0.009
34.80	2.06	61.92	1.37	84.64	18	1227	0.67	0.658	0.74	19.28	0.83	0.005
34.85	2.06	62.34	1.36	84.87	18	1228	0.67	0.657	0.74	19.28	0.83	0.005

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
34.94	2.06	62.46	1.36	85.15	18	1235	0.67	0.643	0.72	19.28	0.81	0.009
34.99	2.06	63.54	1.37	86.81	18	1262	0.67	0.588	0.65	19.28	0.72	0.004
35.12	2.06	65.13	1.37	88.95	19	1296	0.67	0.530	0.57	19.28	0.63	0.010
35.17	2.05	67.60	1.36	91.59	19	1328	0.67	0.483	0.50	19.28	0.56	0.003
35.19	2.02	71.57	1.32	94.80	20	1345	0.67	0.459	0.46	19.28	0.52	0.001
35.26	1.98	76.56	1.29	98.98	20	1362	0.67	0.438	0.43	19.28	0.48	0.004
35.31	1.91	87.54	1.24	108.76	22	1398	0.67	0.398	0.36	19.28	0.40	0.002
35.40	1.83	100.35	1.19	119.11	23	1439	0.67	0.360	0.30	19.28	0.34	0.004
35.44	1.76	113.68	1.11	125.70	24	1470	0.67	0.333	0.27	19.28	0.30	0.002
35.53	1.73	120.26	1.04	125.46	24	1475	0.67	0.330	0.27	19.28	0.30	0.003
35.58	1.72	120.88	1.02	123.51	23	1461	0.67	0.343	0.29	19.28	0.32	0.002
35.63	1.73	117.69	1.04	122.61	23	1445	0.67	0.357	0.30	19.28	0.34	0.002
35.72	1.75	113.43	1.08	122.11	23	1434	0.67	0.368	0.31	19.28	0.35	0.004
35.77	1.76	111.34	1.09	121.88	23	1432	0.67	0.371	0.31	19.28	0.35	0.002
35.84	1.76	111.33	1.10	121.96	23	1434	0.67	0.370	0.31	19.28	0.35	0.003
35.92	1.75	112.54	1.09	122.18	23	1438	0.67	0.367	0.31	19.28	0.34	0.003
35.97	1.75	114.04	1.08	123.43	23	1454	0.67	0.353	0.29	19.28	0.33	0.002
36.05	1.75	114.72	1.09	125.04	24	1475	0.67	0.337	0.27	19.28	0.31	0.003
36.10	1.78	111.50	1.13	126.37	24	1501	0.67	0.318	0.25	19.28	0.28	0.002
36.20	1.82	106.48	1.17	125.08	24	1516	0.67	0.308	0.24	19.28	0.27	0.003
36.23	1.87	99.31	1.22	120.83	24	1524	0.67	0.303	0.24	19.28	0.27	0.001
36.33	1.91	94.01	1.24	116.68	23	1523	0.67	0.304	0.25	19.28	0.28	0.003
36.38	1.94	89.21	1.26	112.62	23	1518	0.67	0.309	0.26	19.28	0.29	0.002
36.43	1.96	86.51	1.27	110.27	23	1513	0.67	0.312	0.27	19.28	0.30	0.002
36.51	1.98	84.03	1.29	108.21	22	1513	0.67	0.314	0.28	19.28	0.31	0.003
36.55	2.01	80.21	1.31	105.27	22	1513	0.67	0.314	0.28	19.28	0.31	0.002
36.63	2.04	75.88	1.35	102.25	22	1514	0.67	0.314	0.29	19.28	0.32	0.003
36.70	2.08	72.32	1.38	100.14	21	1515	0.67	0.314	0.29	19.28	0.32	0.003
36.75	2.09	71.13	1.40	99.83	22	1524	0.67	0.308	0.28	19.28	0.32	0.002
36.84	2.11	69.41	1.43	99.35	22	1532	0.67	0.304	0.28	19.28	0.31	0.004
36.88	2.12	69.41	1.45	100.53	22	1558	0.67	0.288	0.26	19.28	0.29	0.001
36.98	2.13	69.67	1.46	101.78	22	1584	0.67	0.273	0.24	19.28	0.27	0.003
37.02	2.11	72.86	1.44	104.72	23	1623	0.67	0.253	0.22	19.28	0.24	0.001
37.11	2.10	74.84	1.42	106.50	23	1647	0.67	0.242	0.20	19.28	0.23	0.003
37.14	2.10	76.19	1.42	107.93	23	1666	0.67	0.234	0.19	19.28	0.22	0.001
37.20	2.10	76.64	1.42	108.89	24	1685	0.67	0.226	0.19	19.28	0.21	0.002
37.29	2.10	77.81	1.42	110.49	24	1712	0.67	0.216	0.17	19.28	0.20	0.002
37.35	2.09	79.86	1.41	112.57	24	1740	0.67	0.206	0.16	19.28	0.18	0.001
37.44	2.09	81.29	1.40	113.75	24	1755	0.67	0.202	0.16	19.28	0.18	0.002
37.48	2.08	82.21	1.39	114.05	24	1752	0.67	0.203	0.16	19.28	0.18	0.001
37.57	2.07	82.00	1.38	113.36	24	1741	0.67	0.207	0.16	19.28	0.18	0.002
37.62	2.07	81.21	1.38	112.32	24	1727	0.67	0.213	0.17	19.28	0.19	0.001
37.68	2.09	79.23	1.40	110.87	24	1718	0.66	0.216	0.18	19.28	0.20	0.001
37.76	2.10	76.83	1.42	109.39	24	1711	0.66	0.219	0.18	19.28	0.20	0.002
37.80	2.11	75.72	1.44	108.84	24	1710	0.66	0.220	0.18	19.28	0.20	0.001
37.88	2.12	74.80	1.45	108.32	24	1709	0.66	0.221	0.18	19.28	0.20	0.002
37.95	2.12	75.24	1.45	108.73	24	1716	0.66	0.219	0.18	19.28	0.20	0.002
38.03	2.12	75.79	1.44	109.36	24	1728	0.66	0.215	0.17	19.28	0.19	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
38.08	2.11	77.55	1.43	110.90	24	1748	0.66	0.208	0.17	19.28	0.19	0.001
38.13	2.10	79.23	1.42	112.63	24	1772	0.66	0.200	0.16	19.28	0.18	0.001
38.22	2.10	79.85	1.42	113.66	25	1793	0.66	0.194	0.15	19.28	0.17	0.002
38.27	2.11	79.32	1.44	113.88	25	1804	0.66	0.191	0.15	19.28	0.17	0.001
38.35	2.12	77.93	1.45	113.12	25	1801	0.66	0.192	0.15	19.28	0.17	0.002
38.40	2.13	76.71	1.46	112.26	25	1793	0.66	0.195	0.15	19.28	0.17	0.001
38.45	2.14	75.07	1.48	110.90	24	1777	0.66	0.200	0.16	19.28	0.18	0.001
38.54	2.15	72.98	1.50	109.34	24	1760	0.66	0.206	0.16	19.28	0.18	0.002
38.58	2.17	69.42	1.55	107.40	24	1737	0.66	0.215	0.17	19.28	0.19	0.001
38.67	2.19	67.04	1.58	106.21	24	1723	0.66	0.221	0.18	19.28	0.20	0.002
38.73	2.19	65.51	1.60	104.50	23	1697	0.66	0.231	0.19	19.28	0.21	0.001
38.82	2.18	65.93	1.56	102.85	23	1672	0.66	0.243	0.21	19.28	0.23	0.003
38.87	2.15	67.41	1.51	101.96	23	1653	0.66	0.251	0.22	19.28	0.24	0.001
38.92	2.13	70.09	1.47	103.07	23	1664	0.66	0.247	0.21	19.28	0.24	0.002
38.99	2.11	73.88	1.43	105.77	23	1695	0.66	0.234	0.20	19.28	0.22	0.002
39.05	2.09	77.40	1.40	108.69	23	1728	0.66	0.221	0.18	19.28	0.20	0.001
39.14	2.08	79.53	1.39	110.57	24	1751	0.66	0.213	0.17	19.28	0.19	0.002
39.20	2.08	80.17	1.39	111.34	24	1764	0.66	0.209	0.17	19.28	0.19	0.001
39.25	2.07	81.56	1.38	112.63	24	1779	0.66	0.204	0.16	19.28	0.18	0.001
39.33	2.06	83.95	1.37	114.71	24	1801	0.66	0.197	0.15	19.28	0.17	0.002
39.39	2.04	87.64	1.34	117.83	25	1826	0.66	0.190	0.15	19.28	0.16	0.001
39.45	2.01	91.68	1.32	120.96	25	1838	0.66	0.186	0.14	19.28	0.16	0.001
39.52	1.98	97.15	1.29	125.29	26	1846	0.66	0.185	0.14	19.28	0.15	0.001
39.57	1.94	103.13	1.26	130.19	26	1849	0.66	0.184	0.13	19.28	0.15	0.001
39.66	1.92	106.94	1.25	133.36	27	1854	0.66	0.183	0.13	19.28	0.14	0.002
39.71	1.92	105.62	1.25	132.20	27	1852	0.66	0.184	0.13	19.28	0.15	0.001
39.80	1.95	101.39	1.27	128.49	26	1844	0.66	0.186	0.13	19.28	0.15	0.002
39.83	1.98	95.35	1.29	123.23	25	1831	0.66	0.190	0.14	19.28	0.16	0.001
39.93	2.01	90.82	1.31	119.36	25	1819	0.66	0.194	0.15	19.28	0.17	0.002
39.98	2.03	86.79	1.34	116.16	25	1810	0.66	0.197	0.15	19.28	0.17	0.001
40.05	2.05	84.43	1.36	114.78	24	1816	0.66	0.196	0.15	19.28	0.17	0.001
40.12	2.07	83.15	1.38	114.49	24	1832	0.66	0.191	0.15	19.28	0.17	0.002
40.16	2.07	83.91	1.38	115.72	25	1855	0.66	0.185	0.14	19.28	0.16	0.001
40.26	2.07	85.30	1.37	117.04	25	1872	0.66	0.181	0.14	19.28	0.15	0.002
40.35	2.06	86.23	1.37	117.90	25	1884	0.66	0.178	0.14	19.28	0.15	0.002
40.38	2.07	84.82	1.38	117.36	25	1892	0.66	0.176	0.13	19.28	0.15	0.001
40.44	2.10	82.05	1.42	116.28	25	1902	0.66	0.174	0.13	19.28	0.15	0.001
40.49	2.13	78.60	1.47	115.63	25	1918	0.66	0.170	0.13	19.28	0.14	0.001
40.58	2.16	75.96	1.52	115.35	26	1929	0.66	0.168	0.13	19.28	0.14	0.001
40.63	2.17	74.59	1.54	115.06	26	1929	0.66	0.168	0.12	19.28	0.14	0.001
40.77	2.16	74.86	1.53	114.67	25	1926	0.66	0.169	0.13	19.28	0.14	0.002
40.79	2.14	78.00	1.48	115.43	25	1927	0.66	0.169	0.13	19.28	0.14	0.000
40.86	2.09	83.90	1.41	118.20	26	1941	0.66	0.166	0.12	19.28	0.14	0.001
40.91	2.04	90.75	1.35	122.42	26	1951	0.66	0.164	0.12	19.28	0.13	0.001
40.95	1.99	98.97	1.30	128.43	27	1955	0.65	0.163	0.12	19.28	0.13	0.001
41.05	1.94	105.86	1.26	133.78	27	1948	0.65	0.165	0.11	19.28	0.13	0.001
41.09	1.89	112.72	1.23	138.69	28	1922	0.65	0.171	0.12	19.28	0.13	0.001
41.17	1.85	117.00	1.20	140.61	28	1883	0.65	0.181	0.12	19.28	0.14	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
41.23	1.81	120.60	1.17	140.93	27	1836	0.65	0.194	0.13	19.28	0.15	0.001
41.28	1.79	122.38	1.14	139.09	27	1786	0.65	0.211	0.15	19.28	0.17	0.001
41.41	1.79	119.25	1.14	136.09	26	1753	0.65	0.223	0.16	19.28	0.18	0.003
41.50	1.82	112.90	1.17	132.51	26	1739	0.65	0.229	0.17	19.28	0.19	0.002
41.55	1.87	105.39	1.22	128.11	25	1753	0.65	0.224	0.17	19.28	0.19	0.001
41.60	1.91	100.41	1.24	124.81	25	1775	0.65	0.216	0.16	19.28	0.18	0.001
41.69	1.94	97.43	1.26	123.12	25	1810	0.65	0.204	0.16	19.28	0.17	0.002
41.74	1.96	96.40	1.28	123.39	25	1858	0.65	0.190	0.14	19.28	0.16	0.001
41.82	1.97	97.42	1.28	125.15	26	1899	0.65	0.179	0.13	19.28	0.15	0.002
41.88	1.96	101.11	1.28	129.15	26	1942	0.65	0.169	0.12	19.28	0.13	0.001
41.97	1.95	105.12	1.27	133.20	27	1976	0.65	0.162	0.11	19.28	0.13	0.001
42.00	1.94	109.32	1.26	137.78	28	2024	0.65	0.152	0.10	19.28	0.11	0.000
42.06	1.96	107.56	1.27	137.12	28	2060	0.65	0.146	0.10	19.28	0.11	0.001
42.13	1.96	109.97	1.28	140.31	29	2113	0.65	0.137	0.09	19.28	0.10	0.001
42.22	1.96	112.03	1.28	142.94	29	2156	0.65	0.131	0.08	19.28	0.09	0.001
42.27	1.93	119.28	1.26	150.03	30	2205	0.65	0.124	0.08	19.28	0.08	0.000
42.36	1.93	121.92	1.25	152.83	31	2234	0.65	0.121	0.07	19.28	0.08	0.001
42.41	1.93	122.74	1.26	154.58	31	2282	0.65	0.115	0.07	19.28	0.08	0.000
42.50	1.95	121.65	1.27	154.19	31	2308	0.65	0.112	0.07	19.28	0.07	0.001
42.54	1.96	119.48	1.28	152.40	31	2309	0.65	0.112	0.07	19.28	0.07	0.000
42.59	1.96	117.01	1.28	149.33	31	2266	0.65	0.117	0.07	19.28	0.08	0.000
42.67	1.97	113.16	1.28	144.90	30	2215	0.65	0.124	0.08	19.28	0.09	0.001
42.73	1.98	108.25	1.29	139.58	29	2161	0.65	0.131	0.08	19.28	0.09	0.001
42.79	2.02	98.38	1.32	130.16	27	2093	0.65	0.142	0.10	19.28	0.11	0.001
42.86	2.04	91.98	1.35	123.81	26	2031	0.65	0.153	0.11	19.28	0.12	0.001
42.95	2.06	87.56	1.36	119.45	25	1986	0.65	0.162	0.12	19.28	0.14	0.002
42.98	2.04	89.38	1.34	120.07	25	1970	0.65	0.166	0.12	19.28	0.14	0.001
43.05	2.04	89.81	1.34	120.31	25	1970	0.65	0.166	0.12	19.28	0.14	0.001
43.14	2.02	91.67	1.33	121.81	26	1979	0.65	0.164	0.12	19.28	0.14	0.001
43.19	1.99	98.86	1.30	128.20	27	2017	0.65	0.157	0.11	19.28	0.12	0.001
43.28	1.94	107.95	1.27	136.63	28	2063	0.65	0.148	0.10	19.28	0.11	0.001
43.32	1.88	121.88	1.23	149.46	30	2127	0.65	0.138	0.09	19.28	0.10	0.000
43.42	1.85	131.82	1.20	157.82	31	2171	0.64	0.131	0.08	19.28	0.09	0.001
43.44	1.81	141.75	1.16	164.74	32	2205	0.64	0.127	0.07	19.28	0.08	0.000
43.51	1.78	147.82	1.13	166.93	32	2205	0.64	0.127	0.07	19.28	0.08	0.001
43.60	1.75	153.20	1.09	166.96	32	2193	0.64	0.128	0.07	19.28	0.08	0.001
43.65	1.72	158.20	1.04	164.39	31	2169	0.64	0.132	0.08	19.28	0.09	0.000
43.73	1.70	161.13	1.00	161.13	30	2144	0.64	0.136	0.08	19.28	0.09	0.001
43.78	1.69	163.75	1.00	163.75	30	2121	0.64	0.139	0.08	19.28	0.09	0.001
43.83	1.67	165.39	1.00	165.39	31	2097	0.64	0.143	0.09	19.28	0.10	0.001
43.92	1.66	166.65	1.00	166.65	31	2082	0.64	0.146	0.09	19.28	0.10	0.001
43.97	1.66	167.49	1.00	167.49	31	2073	0.64	0.148	0.09	19.28	0.10	0.001
44.10	1.66	167.39	1.00	167.39	31	2076	0.64	0.148	0.09	19.28	0.10	0.002
44.11	1.66	167.02	1.00	167.02	31	2083	0.64	0.146	0.09	19.28	0.10	0.000
44.20	1.66	166.00	1.00	166.00	31	2090	0.64	0.145	0.09	19.28	0.10	0.001
44.25	1.67	164.70	1.00	164.70	30	2100	0.64	0.144	0.09	19.28	0.10	0.001
44.33	1.68	163.61	1.00	163.61	30	2113	0.64	0.142	0.09	19.28	0.10	0.001
44.41	1.69	163.14	1.00	163.14	30	2125	0.64	0.140	0.09	19.28	0.10	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
44.43	1.69	163.40	1.00	163.40	30	2138	0.64	0.138	0.08	19.28	0.09	0.000
44.52	1.69	164.01	1.00	164.01	30	2152	0.64	0.136	0.08	19.28	0.09	0.001
44.57	1.69	165.78	1.00	165.78	31	2175	0.64	0.133	0.08	19.28	0.09	0.001
44.66	1.69	167.32	1.00	167.32	31	2191	0.64	0.131	0.08	19.28	0.09	0.001
44.70	1.69	167.37	1.00	167.37	31	2194	0.64	0.130	0.08	19.28	0.09	0.000
44.75	1.69	164.46	1.00	164.46	31	2180	0.64	0.132	0.08	19.28	0.09	0.001
44.84	1.71	159.45	1.01	160.33	30	2165	0.64	0.135	0.08	19.28	0.09	0.001
44.89	1.73	152.68	1.05	160.65	30	2150	0.64	0.137	0.08	19.28	0.09	0.001
44.98	1.76	146.07	1.09	159.82	30	2138	0.64	0.139	0.08	19.28	0.09	0.001
45.03	1.77	143.07	1.11	158.74	30	2129	0.64	0.141	0.09	19.28	0.10	0.001
45.12	1.77	140.78	1.12	157.70	30	2122	0.64	0.142	0.09	19.28	0.10	0.001
45.16	1.77	142.19	1.11	157.95	30	2122	0.64	0.142	0.09	19.28	0.10	0.000
45.25	1.77	142.19	1.12	158.81	30	2139	0.64	0.139	0.08	19.28	0.09	0.001
45.34	1.77	143.77	1.12	160.49	31	2164	0.64	0.136	0.08	19.28	0.09	0.001
45.37	1.79	142.07	1.14	161.91	31	2199	0.64	0.131	0.08	19.28	0.09	0.000
45.44	1.81	139.12	1.17	162.30	32	2237	0.64	0.126	0.07	19.28	0.08	0.001
45.48	1.85	134.68	1.20	161.64	32	2294	0.64	0.119	0.07	19.28	0.08	0.000
45.54	1.91	126.30	1.25	157.46	32	2383	0.63	0.109	0.06	19.28	0.07	0.001
45.63	2.00	114.80	1.31	149.82	31	2467	0.63	0.102	0.06	19.28	0.07	0.001
45.69	2.08	99.62	1.39	138.66	30	2439	0.63	0.104	0.06	19.28	0.07	0.000
45.80	2.14	87.50	1.49	130.35	29	2363	0.63	0.112	0.07	19.28	0.08	0.001
45.82	2.16	81.27	1.53	123.95	27	2258	0.63	0.124	0.08	19.28	0.09	0.000
45.95	2.18	78.07	1.56	121.81	27	2230	0.63	0.127	0.09	19.28	0.10	0.002
45.96	2.16	79.42	1.52	120.84	27	2205	0.63	0.131	0.09	19.28	0.10	0.000
46.00	2.14	80.86	1.49	120.23	26	2185	0.63	0.134	0.10	19.28	0.11	0.001
46.07	2.10	85.57	1.42	121.47	26	2173	0.63	0.135	0.10	19.28	0.11	0.001
46.13	2.02	97.29	1.33	129.08	27	2188	0.63	0.133	0.09	19.28	0.10	0.001
46.23	1.96	109.26	1.27	139.21	28	2219	0.63	0.129	0.08	19.28	0.09	0.001
46.27	1.89	122.52	1.23	150.93	30	2256	0.63	0.125	0.08	19.28	0.09	0.000
46.35	1.87	126.84	1.22	154.49	31	2266	0.63	0.123	0.07	19.28	0.08	0.001
46.41	1.86	129.58	1.21	156.45	31	2266	0.63	0.123	0.07	19.28	0.08	0.001
46.46	1.85	131.24	1.20	157.64	31	2269	0.63	0.123	0.07	19.28	0.08	0.001
46.53	1.85	132.27	1.20	158.38	31	2272	0.63	0.123	0.07	19.28	0.08	0.001
46.59	1.84	132.72	1.19	157.95	31	2252	0.63	0.125	0.07	19.28	0.08	0.001
46.68	1.82	132.79	1.18	156.41	30	2208	0.63	0.131	0.08	19.28	0.09	0.001
46.73	1.80	133.78	1.15	154.29	30	2145	0.63	0.141	0.09	19.28	0.10	0.001
46.82	1.78	134.48	1.13	152.49	29	2105	0.63	0.147	0.09	19.28	0.10	0.001
46.86	1.78	134.17	1.12	150.72	29	2075	0.63	0.153	0.10	19.28	0.11	0.001
46.92	1.79	131.50	1.14	149.74	29	2073	0.63	0.153	0.10	19.28	0.11	0.001
47.01	1.81	128.70	1.16	149.14	29	2087	0.63	0.151	0.10	19.28	0.11	0.001
47.05	1.82	126.41	1.17	148.53	29	2102	0.63	0.148	0.10	19.28	0.11	0.001
47.13	1.83	124.76	1.19	147.90	29	2114	0.63	0.146	0.09	19.28	0.11	0.001
47.19	1.84	123.52	1.19	147.34	29	2122	0.63	0.145	0.09	19.28	0.10	0.001
47.25	1.85	123.55	1.20	147.96	29	2143	0.63	0.142	0.09	19.28	0.10	0.001
47.36	1.85	124.51	1.20	149.17	29	2164	0.63	0.138	0.09	19.28	0.10	0.001
47.38	1.84	126.60	1.19	151.16	30	2185	0.63	0.135	0.08	19.28	0.09	0.000
47.47	1.83	128.89	1.19	152.91	30	2196	0.62	0.134	0.08	19.28	0.09	0.001
47.51	1.82	132.23	1.17	155.08	30	2204	0.62	0.133	0.08	19.28	0.09	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
47.61	1.80	135.14	1.16	156.37	30	2202	0.62	0.133	0.08	19.28	0.09	0.001
47.65	1.78	139.02	1.13	157.14	30	2188	0.62	0.135	0.08	19.28	0.09	0.000
47.75	1.76	141.81	1.11	156.97	30	2176	0.62	0.137	0.08	19.28	0.09	0.001
47.77	1.75	144.93	1.08	156.76	30	2169	0.62	0.138	0.09	19.28	0.10	0.000
47.84	1.74	146.59	1.07	156.94	30	2174	0.62	0.138	0.09	19.28	0.10	0.001
47.93	1.74	147.73	1.06	156.99	30	2178	0.62	0.137	0.09	19.28	0.10	0.001
47.98	1.74	148.18	1.06	157.18	30	2182	0.62	0.137	0.08	19.28	0.10	0.001
48.07	1.74	148.29	1.06	157.09	30	2183	0.62	0.136	0.08	19.28	0.10	0.001
48.12	1.73	148.95	1.05	156.78	30	2182	0.62	0.137	0.09	19.28	0.10	0.001
48.20	1.72	150.52	1.04	156.14	29	2179	0.62	0.137	0.09	19.28	0.10	0.001
48.25	1.71	154.11	1.01	155.51	29	2186	0.62	0.136	0.09	19.28	0.10	0.001
48.34	1.70	157.59	1.00	157.59	29	2198	0.62	0.134	0.08	19.28	0.09	0.001
48.39	1.69	160.74	1.00	160.74	30	2212	0.62	0.133	0.08	19.28	0.09	0.001
48.43	1.68	163.68	1.00	163.68	30	2223	0.62	0.131	0.08	19.28	0.09	0.000
48.52	1.67	167.75	1.00	167.75	31	2234	0.62	0.130	0.08	19.28	0.09	0.001
48.58	1.65	172.26	1.00	172.26	32	2246	0.62	0.128	0.07	19.28	0.08	0.001
48.63	1.65	175.23	1.00	175.23	32	2259	0.62	0.127	0.07	19.28	0.08	0.001
48.71	1.64	176.81	1.00	176.81	32	2271	0.62	0.125	0.07	19.28	0.08	0.001
48.76	1.64	177.46	1.00	177.46	32	2281	0.62	0.124	0.07	19.28	0.08	0.000
48.85	1.64	177.67	1.00	177.67	33	2288	0.62	0.123	0.07	19.28	0.08	0.001
48.90	1.65	178.14	1.00	178.14	33	2300	0.62	0.122	0.07	19.28	0.08	0.000
48.99	1.65	179.15	1.00	179.15	33	2315	0.62	0.120	0.07	19.28	0.07	0.001
49.02	1.65	180.76	1.00	180.76	33	2333	0.62	0.118	0.06	19.28	0.07	0.000
49.09	1.64	182.79	1.00	182.79	33	2343	0.62	0.117	0.06	19.28	0.07	0.001
49.15	1.63	185.01	1.00	185.01	34	2354	0.62	0.116	0.06	19.28	0.07	0.001
49.25	1.63	188.74	1.00	188.74	34	2379	0.61	0.114	0.06	19.28	0.07	0.001
49.29	1.61	196.76	1.00	196.76	36	2439	0.61	0.108	0.05	19.28	0.06	0.000
49.36	1.60	206.82	1.00	206.82	37	2527	0.61	0.100	0.05	19.28	0.05	0.000
49.43	1.59	216.88	1.00	216.88	39	2616	0.61	0.094	0.04	19.28	0.05	0.000
49.48	1.60	224.98	1.00	224.98	41	2736	0.61	0.086	0.04	19.28	0.04	0.000
49.57	1.62	230.39	1.00	230.39	42	2887	0.61	0.078	0.03	19.28	0.04	0.000
49.62	1.66	232.05	1.00	232.05	43	3071	0.61	0.070	0.03	19.28	0.03	0.000
49.71	1.68	230.71	1.00	230.71	43	3178	0.61	0.066	0.03	19.28	0.03	0.000
49.76	1.68	233.73	1.00	233.73	43	3191	0.61	0.065	0.03	19.28	0.03	0.000
49.89	1.66	234.46	1.00	234.46	43	3142	0.61	0.067	0.03	19.28	0.03	0.000
49.94	1.66	229.59	1.00	229.59	42	3062	0.61	0.070	0.03	19.28	0.03	0.000
50.00	1.67	220.88	1.00	220.88	41	2988	0.61	0.073	0.03	19.28	0.03	0.000
50.07	1.66	215.55	1.00	215.55	40	2878	0.61	0.079	0.03	19.28	0.04	0.000
50.17	1.63	215.82	1.00	215.82	39	2771	0.61	0.084	0.04	19.28	0.04	0.001
50.21	1.61	210.92	1.00	210.92	38	2621	0.61	0.094	0.04	19.28	0.05	0.000
50.31	1.59	205.36	1.00	205.36	37	2506	0.61	0.103	0.05	19.28	0.06	0.001
50.34	1.60	195.54	1.00	195.54	35	2425	0.61	0.110	0.06	19.28	0.06	0.000
50.40	1.63	187.86	1.00	187.86	34	2405	0.61	0.112	0.06	19.28	0.07	0.000
50.49	1.66	176.46	1.00	176.46	33	2384	0.61	0.114	0.06	19.28	0.07	0.001
50.54	1.69	166.34	1.00	166.34	31	2359	0.61	0.117	0.07	19.28	0.08	0.000
50.60	1.72	157.58	1.03	163.08	31	2341	0.61	0.119	0.07	19.28	0.08	0.001
50.68	1.74	152.00	1.08	163.58	31	2342	0.61	0.119	0.07	19.28	0.08	0.001
50.72	1.76	148.63	1.10	163.22	31	2341	0.61	0.119	0.07	19.28	0.08	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
50.82	1.77	145.38	1.12	162.19	31	2336	0.61	0.119	0.07	19.28	0.08	0.001
50.86	1.78	141.81	1.13	159.93	31	2312	0.61	0.122	0.07	19.28	0.08	0.000
50.95	1.79	138.32	1.14	157.47	30	2288	0.61	0.125	0.08	19.28	0.09	0.001
51.00	1.95	82.43	1.27	104.92	21	2761	0.60	0.085	0.08	19.28	0.09	0.001
51.09	1.95	80.69	1.27	102.73	21	2709	0.60	0.089	0.08	19.28	0.09	0.001
51.14	1.96	78.45	1.28	100.20	21	2656	0.60	0.092	0.09	19.28	0.10	0.001
51.19	1.98	75.70	1.29	97.78	20	2630	0.60	0.094	0.09	19.28	0.10	0.001
51.26	2.01	72.02	1.31	94.65	20	2597	0.60	0.096	0.10	19.28	0.11	0.001
51.36	2.03	69.40	1.33	92.50	19	2573	0.60	0.098	0.10	19.28	0.11	0.001
51.38	2.04	68.10	1.34	91.23	19	2548	0.60	0.100	0.10	19.28	0.12	0.000
51.46	2.03	67.92	1.34	90.74	19	2534	0.60	0.101	0.11	19.28	0.12	0.001
51.53	2.02	68.85	1.32	91.14	19	2530	0.60	0.101	0.11	19.28	0.12	0.001
51.58	1.99	72.52	1.30	94.11	19	2566	0.60	0.099	0.10	19.28	0.11	0.001
51.69	1.95	77.62	1.27	98.84	20	2639	0.60	0.093	0.09	19.28	0.10	0.001
51.74	1.93	82.40	1.26	103.53	21	2716	0.60	0.088	0.08	19.28	0.09	0.001
51.78	1.92	85.94	1.25	107.19	22	2785	0.60	0.084	0.08	19.28	0.09	0.000
51.88	1.91	88.31	1.24	109.63	22	2835	0.60	0.082	0.07	19.28	0.08	0.001
51.91	1.89	91.25	1.23	112.53	23	2887	0.60	0.079	0.07	19.28	0.08	0.000
51.99	1.88	93.56	1.23	114.71	23	2926	0.60	0.077	0.07	19.28	0.07	0.001
52.06	1.87	96.51	1.22	117.31	23	2966	0.60	0.075	0.06	19.28	0.07	0.001
52.11	1.86	99.10	1.21	119.52	24	3001	0.60	0.074	0.06	19.28	0.07	0.000
52.20	1.85	101.04	1.20	121.19	24	3033	0.60	0.073	0.06	19.28	0.07	0.001
52.24	1.84	102.74	1.20	122.89	24	3072	0.60	0.071	0.06	19.28	0.06	0.000
52.33	1.84	104.01	1.19	124.09	24	3101	0.60	0.070	0.06	19.28	0.06	0.001
52.43	1.84	104.77	1.19	124.72	24	3118	0.60	0.069	0.05	19.28	0.06	0.001
52.52	1.84	104.89	1.19	124.75	24	3122	0.60	0.069	0.05	19.28	0.06	0.001
52.57	1.83	104.61	1.18	123.48	24	3078	0.60	0.071	0.06	19.28	0.06	0.000
52.65	1.81	104.43	1.16	121.50	24	3013	0.60	0.074	0.06	19.28	0.07	0.001
52.70	1.79	104.70	1.14	119.35	23	2945	0.60	0.077	0.06	19.28	0.07	0.000
52.76	1.78	105.38	1.13	119.09	23	2938	0.59	0.077	0.07	19.28	0.07	0.000
52.84	1.78	106.54	1.13	120.21	23	2970	0.59	0.076	0.06	19.28	0.07	0.001
52.89	1.80	104.53	1.15	120.46	23	2991	0.59	0.075	0.06	19.28	0.07	0.000
52.98	1.81	103.29	1.17	120.60	23	3014	0.59	0.074	0.06	19.28	0.07	0.001
53.02	1.82	103.99	1.17	121.66	24	3046	0.59	0.072	0.06	19.28	0.07	0.000
53.12	1.79	108.12	1.15	123.98	24	3089	0.59	0.071	0.06	19.28	0.06	0.001
53.16	1.78	111.68	1.13	125.68	24	3125	0.59	0.069	0.06	19.28	0.06	0.000
53.25	1.77	113.14	1.12	126.56	24	3152	0.59	0.068	0.05	19.28	0.06	0.001
53.28	1.77	114.61	1.12	128.12	25	3193	0.59	0.067	0.05	19.28	0.06	0.000
53.39	1.77	116.25	1.12	129.68	25	3238	0.59	0.065	0.05	19.28	0.06	0.001
53.44	1.77	117.78	1.11	131.28	25	3282	0.59	0.064	0.05	19.28	0.05	0.000
53.48	1.77	118.85	1.12	132.65	25	3319	0.59	0.063	0.05	19.28	0.05	0.000
53.58	1.77	118.93	1.12	133.38	26	3344	0.59	0.062	0.05	19.28	0.05	0.001
53.62	1.78	119.27	1.12	133.90	26	3360	0.59	0.062	0.05	19.28	0.05	0.000
53.68	1.77	119.65	1.12	134.02	26	3367	0.59	0.061	0.05	19.28	0.05	0.000
53.75	1.77	120.43	1.12	134.52	26	3384	0.59	0.061	0.05	19.28	0.05	0.000
53.85	1.77	120.49	1.12	135.09	26	3405	0.59	0.060	0.04	19.28	0.05	0.001
53.90	1.78	120.50	1.13	135.66	26	3424	0.59	0.060	0.04	19.28	0.05	0.000
53.95	1.78	120.07	1.13	136.01	26	3438	0.59	0.059	0.04	19.28	0.05	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
54.04	1.79	119.86	1.14	136.08	26	3447	0.59	0.059	0.04	19.28	0.05	0.001
54.09	1.78	120.11	1.13	135.93	26	3445	0.59	0.059	0.04	19.28	0.05	0.000
54.17	1.78	120.38	1.13	135.50	26	3438	0.59	0.060	0.04	19.28	0.05	0.001
54.22	1.77	120.73	1.12	135.20	26	3433	0.59	0.060	0.04	19.28	0.05	0.000
54.27	1.77	120.35	1.11	133.49	25	3393	0.59	0.061	0.05	19.28	0.05	0.000
54.35	1.73	119.62	1.05	126.16	24	3240	0.59	0.066	0.05	19.28	0.06	0.001
54.41	1.67	119.29	1.00	119.29	22	2975	0.59	0.076	0.07	19.28	0.08	0.000
54.48	1.61	119.93	1.00	119.93	22	2799	0.59	0.085	0.08	19.28	0.09	0.001
54.55	1.60	123.27	1.00	123.27	22	2836	0.58	0.083	0.07	19.28	0.08	0.001
54.64	1.63	126.26	1.00	126.26	23	3003	0.58	0.075	0.06	19.28	0.07	0.001
54.69	1.62	133.61	1.00	133.61	24	3147	0.58	0.069	0.05	19.28	0.06	0.000
54.73	1.64	133.59	1.00	133.59	24	3243	0.58	0.066	0.05	19.28	0.06	0.000
54.81	1.68	130.06	1.00	130.06	24	3311	0.58	0.063	0.05	19.28	0.06	0.001
54.87	1.76	118.44	1.10	130.62	25	3360	0.58	0.062	0.05	19.28	0.05	0.000
54.96	1.88	99.72	1.23	122.19	24	3304	0.58	0.064	0.05	19.28	0.06	0.001
55.00	1.94	92.64	1.26	117.17	24	3312	0.58	0.064	0.05	19.28	0.06	0.000
55.08	1.98	87.91	1.29	113.48	23	3299	0.58	0.064	0.05	19.28	0.06	0.001
55.12	1.96	92.07	1.28	117.54	24	3373	0.58	0.062	0.05	19.28	0.06	0.000
55.23	2.00	87.57	1.31	114.29	24	3372	0.58	0.062	0.05	19.28	0.06	0.001
55.25	2.04	82.73	1.35	111.47	24	3377	0.58	0.062	0.05	19.28	0.06	0.000
55.36	2.07	80.58	1.37	110.74	24	3397	0.58	0.061	0.05	19.28	0.06	0.001
55.41	2.09	78.39	1.41	110.33	24	3416	0.58	0.061	0.05	19.28	0.06	0.000
55.45	2.10	76.89	1.42	109.21	24	3391	0.58	0.061	0.05	19.28	0.06	0.000
55.54	2.10	75.70	1.42	107.34	23	3338	0.58	0.063	0.05	19.28	0.06	0.001
55.59	2.08	75.78	1.39	105.30	23	3259	0.58	0.065	0.06	19.28	0.06	0.000
55.68	2.05	77.00	1.35	104.16	22	3189	0.58	0.068	0.06	19.28	0.07	0.001
55.73	2.01	79.19	1.32	104.19	22	3129	0.58	0.070	0.06	19.28	0.07	0.000
55.82	1.98	80.92	1.29	104.74	22	3099	0.58	0.071	0.06	19.28	0.07	0.001
55.87	1.97	82.44	1.28	105.78	22	3099	0.58	0.071	0.06	19.28	0.07	0.000
55.92	1.96	83.02	1.28	105.90	22	3083	0.58	0.072	0.07	19.28	0.07	0.000
56.00	1.94	83.49	1.26	105.59	22	3043	0.58	0.074	0.07	19.28	0.08	0.001
56.09	1.93	83.88	1.25	105.14	21	2997	0.58	0.076	0.07	19.28	0.08	0.001
56.15	1.92	84.37	1.25	105.21	21	2980	0.58	0.076	0.07	19.28	0.08	0.001
56.18	1.91	84.98	1.24	105.73	21	2987	0.58	0.076	0.07	19.28	0.08	0.000
56.24	1.91	85.27	1.24	105.98	21	2993	0.58	0.076	0.07	19.28	0.08	0.001
56.30	1.91	85.29	1.24	105.99	21	2997	0.58	0.076	0.07	19.28	0.08	0.001
56.39	1.91	85.53	1.24	106.09	21	2997	0.57	0.076	0.07	19.28	0.08	0.001
56.44	1.89	86.57	1.23	106.66	21	2989	0.57	0.076	0.07	19.28	0.08	0.000
56.52	1.88	87.81	1.22	107.35	21	2983	0.57	0.076	0.07	19.28	0.08	0.001
56.59	1.87	88.58	1.22	107.72	21	2979	0.57	0.077	0.07	19.28	0.08	0.001
56.63	1.87	88.24	1.22	107.32	21	2971	0.57	0.077	0.07	19.28	0.08	0.000
56.72	1.88	87.32	1.22	106.52	21	2964	0.57	0.077	0.07	19.28	0.08	0.001
56.77	1.90	84.67	1.23	104.53	21	2955	0.57	0.078	0.07	19.28	0.08	0.000
56.86	1.92	81.69	1.25	102.32	21	2956	0.57	0.078	0.07	19.28	0.08	0.001
56.90	1.97	76.80	1.28	98.58	20	2946	0.57	0.078	0.08	19.28	0.09	0.000
56.99	2.00	73.02	1.31	95.74	20	2934	0.57	0.079	0.08	19.28	0.09	0.001
57.03	2.04	69.15	1.35	93.18	20	2920	0.57	0.079	0.08	19.28	0.09	0.000
57.09	2.08	66.04	1.39	91.57	20	2912	0.57	0.080	0.08	19.28	0.09	0.001

:: Post-earthquake settlement of dry sands :: (continued)

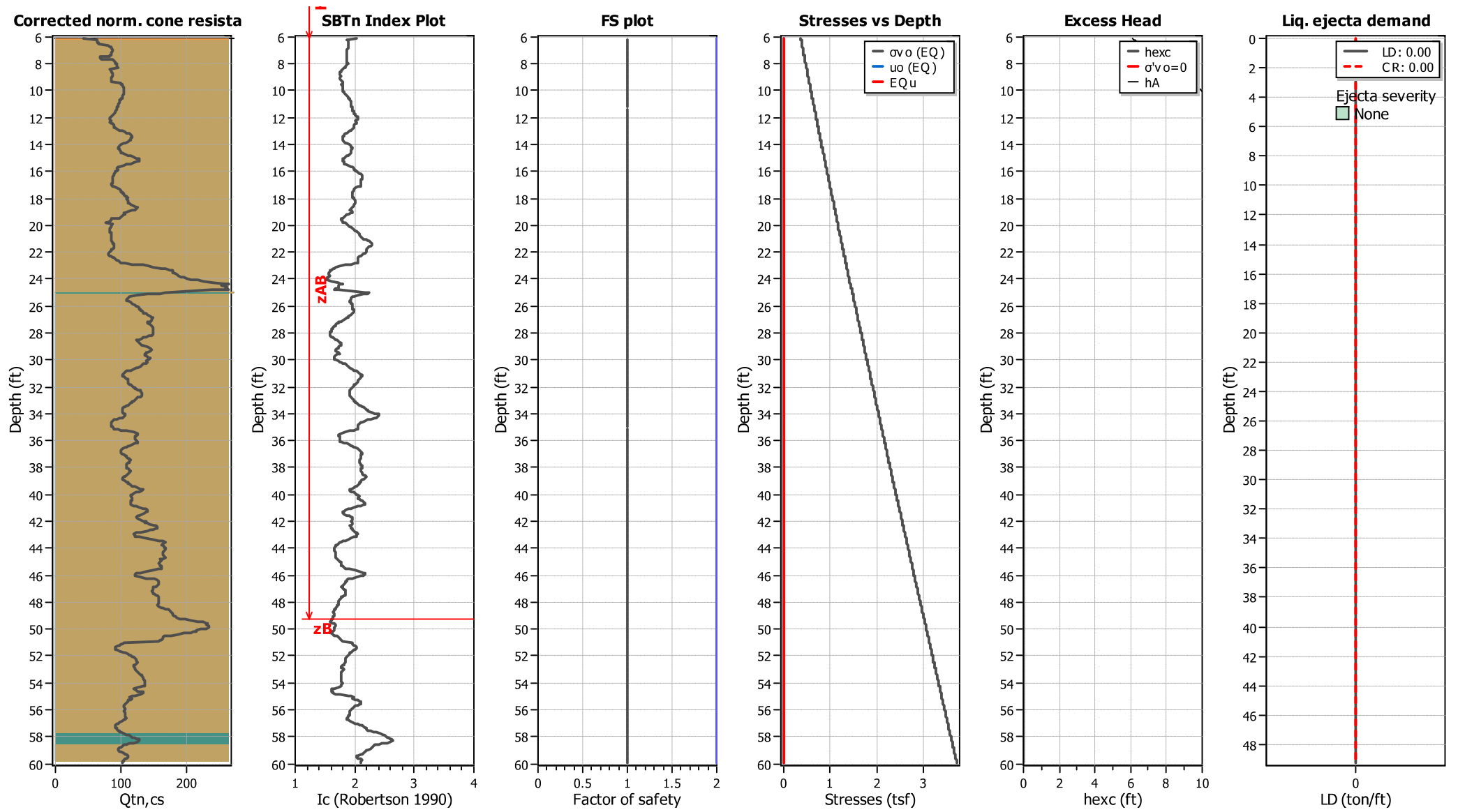
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
57.18	2.11	62.89	1.44	90.55	20	2912	0.57	0.080	0.08	19.28	0.09	0.001
57.22	2.16	60.10	1.52	91.16	20	2940	0.57	0.079	0.08	19.28	0.09	0.000
57.34	2.18	58.90	1.57	92.38	21	2977	0.57	0.077	0.07	19.28	0.08	0.001
57.37	2.20	58.39	1.61	93.82	21	3014	0.57	0.075	0.07	19.28	0.08	0.000
57.41	2.21	57.85	1.63	94.35	21	3026	0.57	0.075	0.07	19.28	0.08	0.000
57.50	2.22	56.74	1.68	95.30	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.55	2.26	53.96	1.79	96.77	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.64	2.31	50.28	1.95	98.15	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.69	2.36	45.89	2.19	100.44	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.78	2.42	41.88	2.48	103.71	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.87	2.47	38.72	2.79	107.88	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.88	2.51	36.73	3.04	111.72	0	0	0.57	0.000	0.00	0.00	0.00	0.000
57.96	2.53	35.76	3.20	114.60	30	2851	0.57	0.083	0.05	19.28	0.06	0.001
58.04	2.54	35.60	3.28	116.83	31	2880	0.57	0.082	0.05	19.28	0.05	0.001
58.09	2.57	34.63	3.47	120.27	32	2892	0.57	0.081	0.05	19.28	0.05	0.000
58.18	2.59	33.45	3.69	123.31	33	2892	0.57	0.081	0.04	19.28	0.05	0.001
58.23	2.62	32.00	3.96	126.73	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.29	2.63	31.75	4.02	127.74	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.36	2.59	33.71	3.70	124.59	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.46	2.54	36.67	3.26	119.59	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.50	2.46	41.49	2.71	112.40	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.55	2.40	45.28	2.38	107.64	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.60	2.34	49.15	2.11	103.55	0	0	0.56	0.000	0.00	0.00	0.00	0.000
58.69	2.32	50.84	1.99	101.36	24	3133	0.56	0.071	0.06	19.28	0.06	0.001
58.77	2.30	52.01	1.91	99.47	23	3126	0.56	0.071	0.06	19.28	0.07	0.001
58.83	2.29	52.51	1.87	98.18	23	3113	0.56	0.072	0.06	19.28	0.07	0.000
58.87	2.26	54.27	1.78	96.56	22	3112	0.56	0.072	0.06	19.28	0.07	0.000
58.97	2.23	56.72	1.69	95.64	22	3133	0.56	0.071	0.06	19.28	0.07	0.001
59.01	2.19	60.13	1.60	96.02	22	3183	0.56	0.069	0.06	19.28	0.07	0.000
59.09	2.17	63.13	1.55	97.98	22	3264	0.56	0.066	0.06	19.28	0.07	0.001
59.15	2.16	65.88	1.53	100.58	22	3359	0.56	0.063	0.06	19.28	0.06	0.000
59.20	2.15	68.97	1.51	103.97	23	3477	0.56	0.060	0.05	19.28	0.06	0.000
59.29	2.13	72.81	1.47	106.82	23	3577	0.56	0.057	0.05	19.28	0.05	0.001
59.33	2.09	77.35	1.41	108.84	23	3623	0.56	0.056	0.05	19.28	0.05	0.000
59.41	2.05	80.98	1.36	110.17	23	3625	0.56	0.056	0.05	19.28	0.05	0.000
59.47	2.03	82.75	1.34	110.70	23	3610	0.56	0.057	0.05	19.28	0.05	0.000
59.52	2.04	82.41	1.34	110.41	23	3607	0.56	0.057	0.05	19.28	0.05	0.000
59.60	2.06	78.92	1.37	108.19	23	3586	0.56	0.057	0.05	19.28	0.05	0.001
59.67	2.09	75.58	1.41	106.38	23	3563	0.56	0.058	0.05	19.28	0.05	0.000
59.73	2.11	72.92	1.44	104.96	23	3535	0.56	0.059	0.05	19.28	0.06	0.000
59.80	2.11	72.31	1.44	104.00	23	3507	0.56	0.059	0.05	19.28	0.06	0.001
59.84	2.11	71.44	1.44	102.55	22	3460	0.56	0.061	0.05	19.28	0.06	0.000
59.94	2.11	70.55	1.44	101.49	22	3431	0.56	0.061	0.05	19.28	0.06	0.001

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
Total estimated settlement: 3.86												

Abbreviations

- Q_{tn}: Equivalent clean sand normalized cone resistance
- K_c: Fines correction factor
- Q_{tn,cs}: Post-liquefaction volumetric strain
- G_{max}: Small strain shear modulus
- CSR: Soil cyclic stress ratio
- γ: Cyclic shear strain
- e_{vol(15)}: Volumetric strain after 15 cycles
- N_c: Equivalent number of cycles
- e_v: Volumetric strain
- Settle.: Calculated settlement

Ejecta Severity Estimation



LIQUEFACTION ANALYSIS REPORT

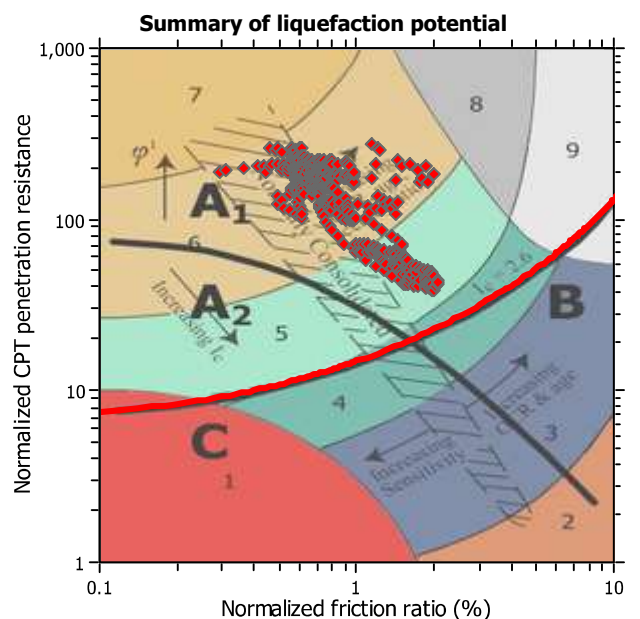
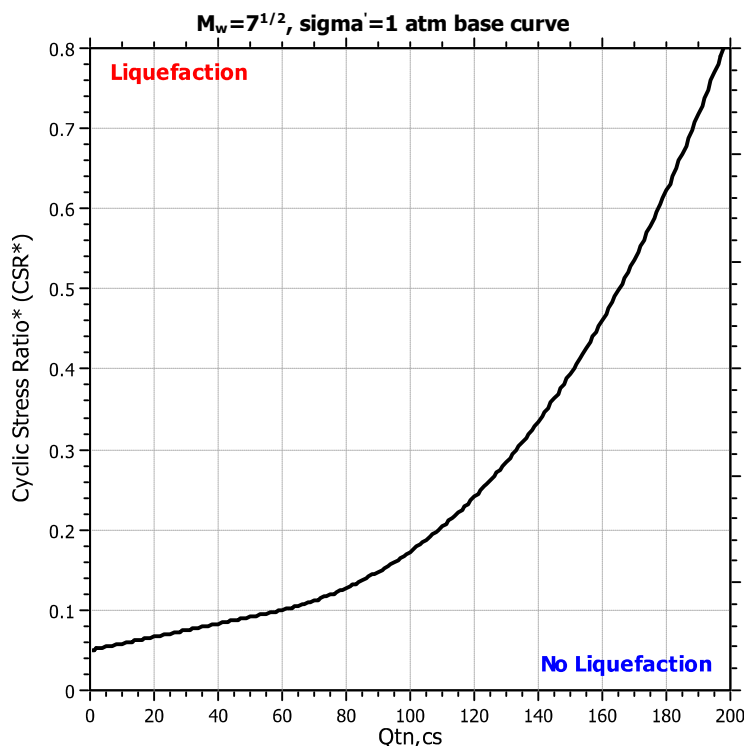
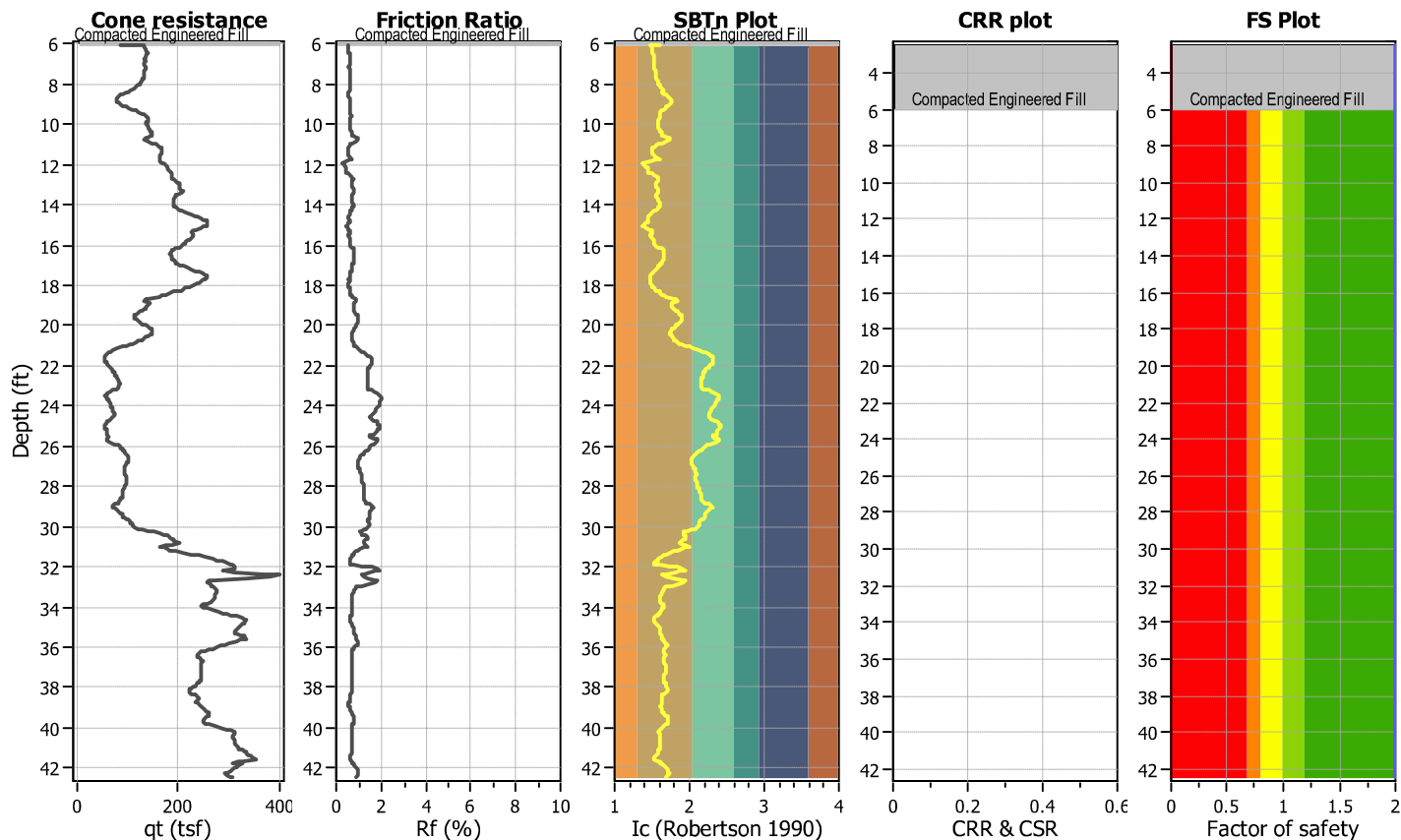
Project title : 4212-CR

Location : Northeast of the 210 Freeway and West San

CPT file : CPT-2

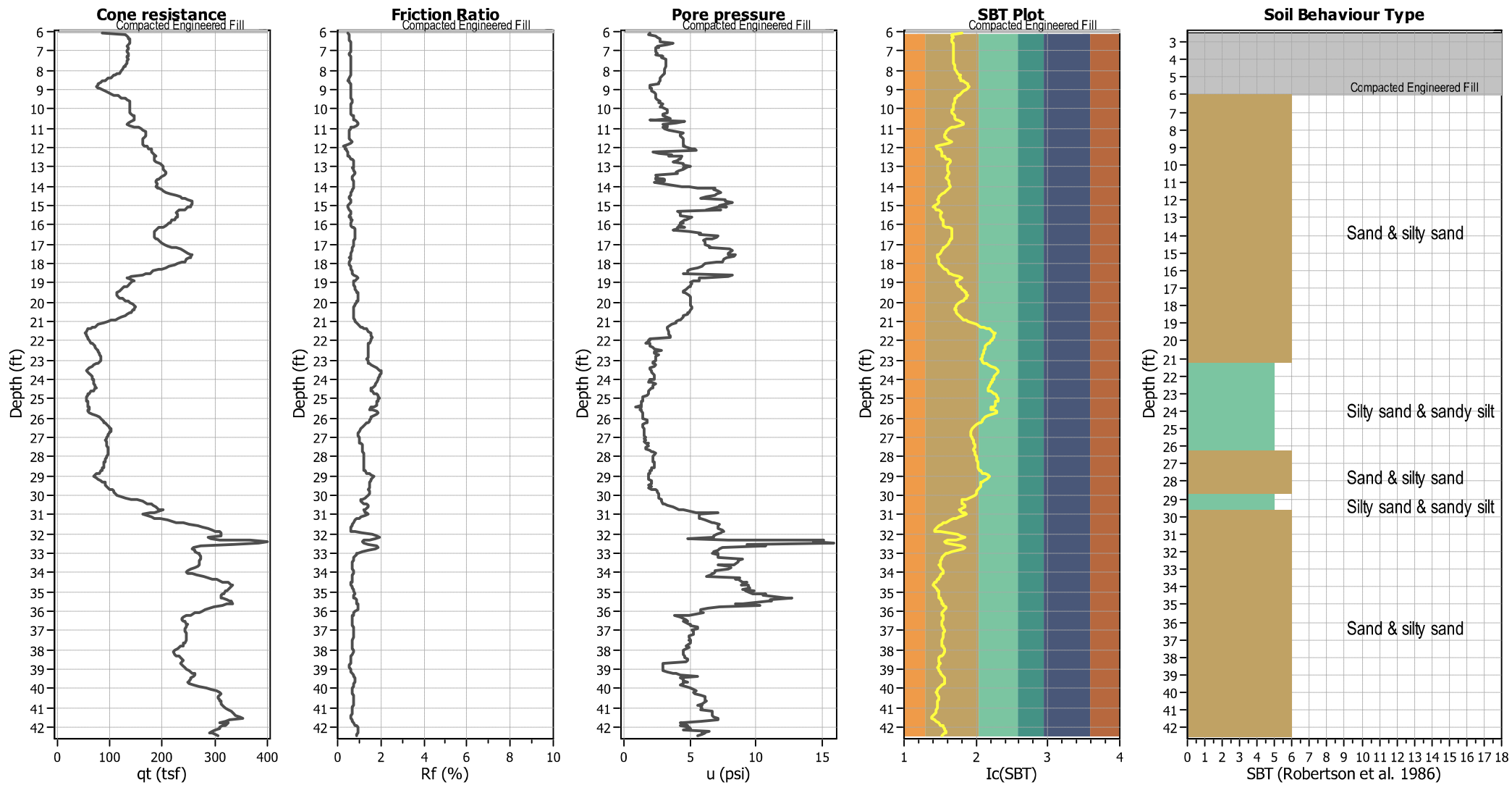
Input parameters and analysis data

Analysis method:	NCEER (1998)	G.W.T. (in-situ):	60.00 ft	Use fill:	No	Clay like behavior	
Fines correction method:	NCEER (1998)	G.W.T. (earthq.):	60.00 ft	Fill height:	N/A	applied:	Sands only
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth applied:	No
Earthquake magnitude M_w :	7.91	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	Limit depth:	N/A
Peak ground acceleration:	0.88	Unit weight calculation:	Based on SBT	K_g applied:	Yes	MSF method:	Method based



Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading
 Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground geometry
 Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening
 Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

CPT basic interpretation plots

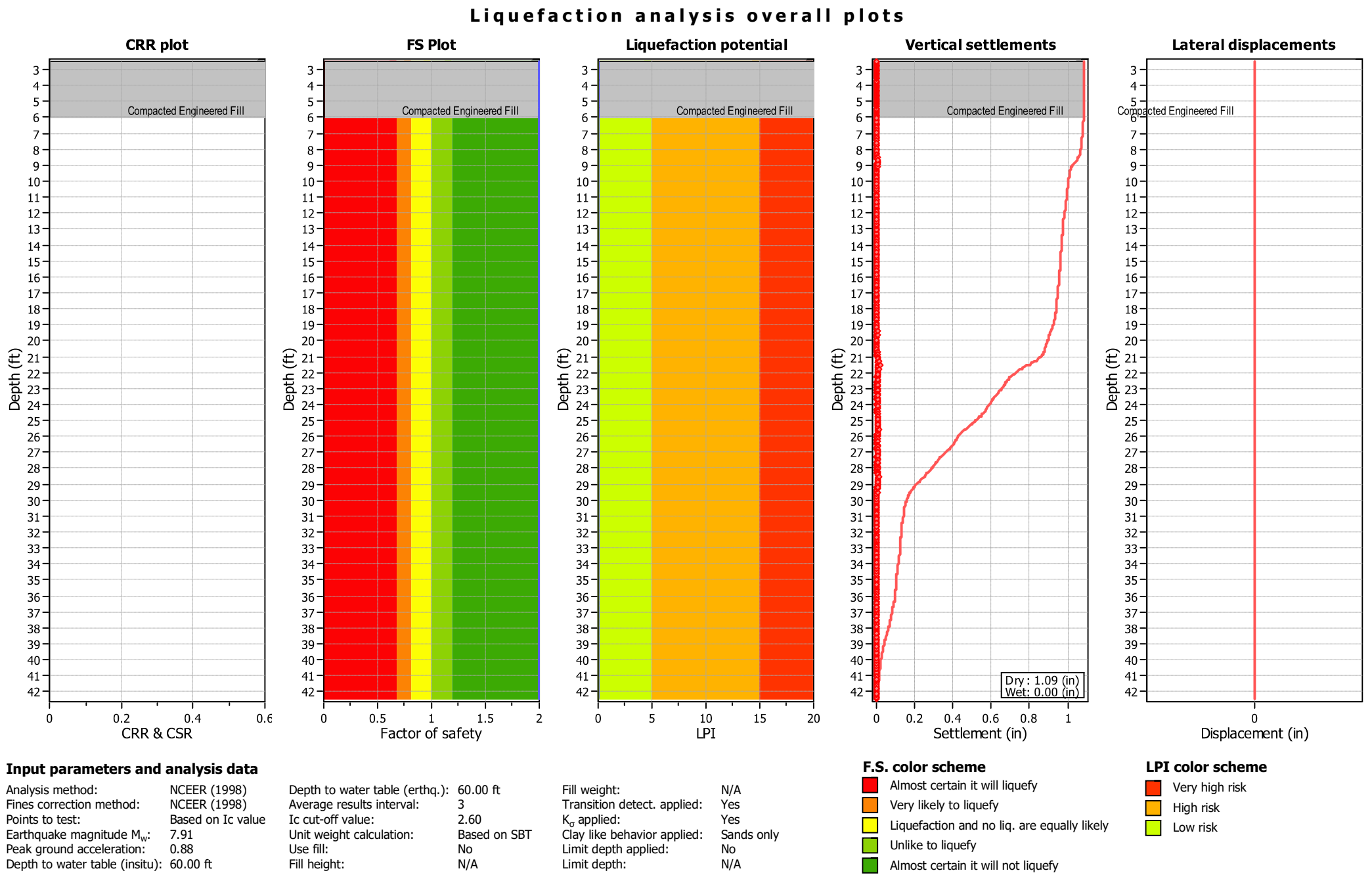


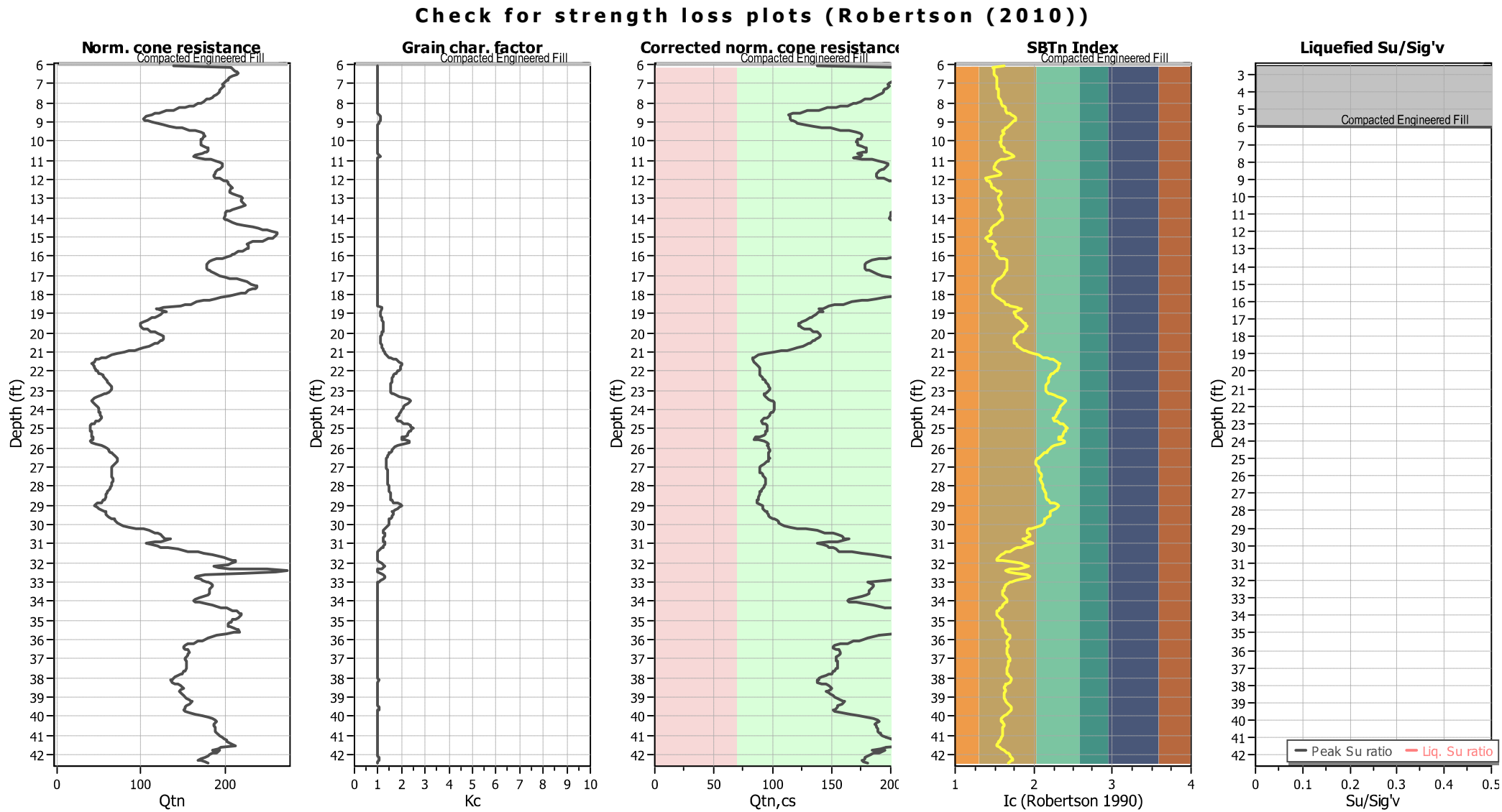
Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K ₀ applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

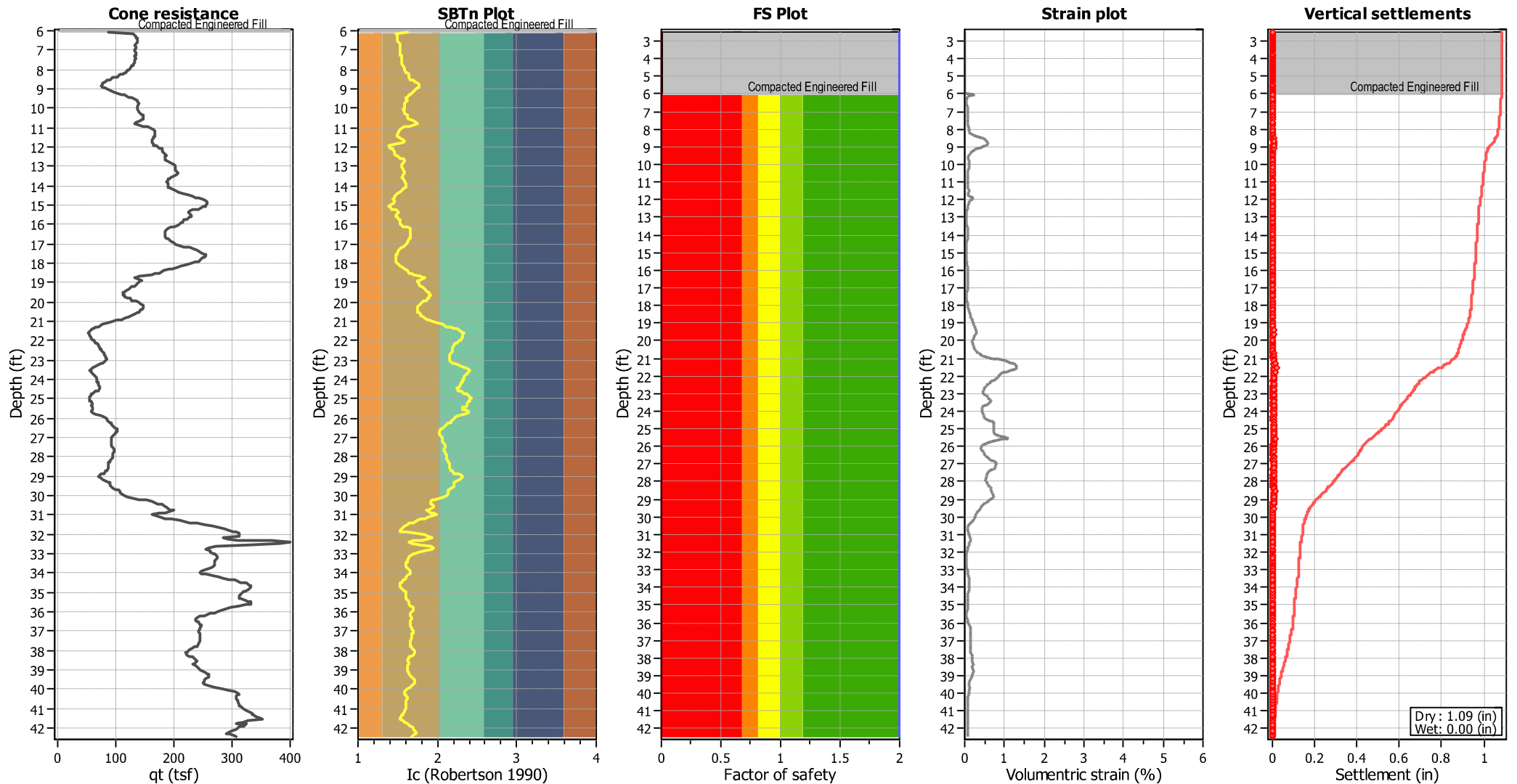




Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _g applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

Estimation of post-earthquake settlements



Abbreviations

- q_t : Total cone resistance (cone resistance q_c corrected for pore water effects)
- I_c : Soil Behaviour Type Index
- FS: Calculated Factor of Safety against liquefaction
- Volumetric strain: Post-liquefaction volumetric strain

:: Post-earthquake settlement of dry sands ::												
Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
2.51	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.63	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.74	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.92	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.97	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.06	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.10	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.16	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.24	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.35	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.43	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.52	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.55	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.61	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.71	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.75	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.84	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.89	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.98	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.03	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.07	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.17	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.21	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.30	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.34	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.40	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.49	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.53	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.63	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.67	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.76	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.81	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.91	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.00	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.09	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.13	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.23	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.28	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.32	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.46	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.50	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.53	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.59	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
5.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.71	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.79	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.88	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.92	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.97	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
6.07	1.61	138.25	1.00	138.25	25	598	0.65	0.306	0.23	19.28	0.26	0.003
6.11	1.48	207.91	1.00	207.91	36	765	0.65	0.112	0.05	19.28	0.06	0.000
6.19	1.49	209.46	1.00	209.46	37	787	0.65	0.104	0.05	19.28	0.06	0.001
6.30	1.50	210.67	1.00	210.67	37	806	0.65	0.099	0.05	19.28	0.05	0.001
6.34	1.50	213.02	1.00	213.02	37	813	0.65	0.097	0.05	19.28	0.05	0.000
6.38	1.49	214.67	1.00	214.67	37	819	0.65	0.096	0.05	19.28	0.05	0.000
6.43	1.49	215.63	1.00	215.63	38	825	0.65	0.095	0.04	19.28	0.05	0.000
6.50	1.50	214.42	1.00	214.42	37	834	0.65	0.094	0.04	19.28	0.05	0.000
6.58	1.52	210.05	1.00	210.05	37	837	0.65	0.095	0.05	19.28	0.05	0.000
6.66	1.53	205.38	1.00	205.38	36	834	0.65	0.098	0.05	19.28	0.05	0.001
6.71	1.53	203.34	1.00	203.34	36	830	0.65	0.102	0.05	19.28	0.06	0.000
6.81	1.52	203.60	1.00	203.60	36	830	0.65	0.104	0.05	19.28	0.06	0.001
6.85	1.52	202.97	1.00	202.97	36	831	0.65	0.105	0.05	19.28	0.06	0.000
6.89	1.52	201.45	1.00	201.45	35	830	0.65	0.107	0.05	19.28	0.06	0.000
6.99	1.53	198.63	1.00	198.63	35	828	0.65	0.111	0.06	19.28	0.06	0.001
7.04	1.53	198.04	1.00	198.04	35	827	0.65	0.113	0.06	19.28	0.06	0.000
7.10	1.52	198.42	1.00	198.42	35	827	0.65	0.115	0.06	19.28	0.07	0.001
7.17	1.53	197.55	1.00	197.55	35	831	0.65	0.115	0.06	19.28	0.07	0.001
7.26	1.53	196.27	1.00	196.27	35	838	0.65	0.115	0.06	19.28	0.07	0.001
7.29	1.54	194.83	1.00	194.83	35	846	0.65	0.112	0.06	19.28	0.07	0.000
7.36	1.55	194.36	1.00	194.36	34	851	0.65	0.112	0.06	19.28	0.07	0.000
7.45	1.55	193.51	1.00	193.51	34	854	0.65	0.113	0.06	19.28	0.07	0.001
7.49	1.55	192.34	1.00	192.34	34	848	0.65	0.117	0.06	19.28	0.07	0.000
7.59	1.55	189.88	1.00	189.88	34	844	0.65	0.123	0.07	19.28	0.07	0.001
7.63	1.55	187.50	1.00	187.50	33	839	0.65	0.127	0.07	19.28	0.08	0.000
7.70	1.56	185.01	1.00	185.01	33	837	0.65	0.130	0.07	19.28	0.08	0.001
7.77	1.56	182.34	1.00	182.34	32	834	0.65	0.134	0.07	19.28	0.08	0.001
7.81	1.57	178.57	1.00	178.57	32	828	0.65	0.139	0.08	19.28	0.09	0.001
7.95	1.58	174.10	1.00	174.10	31	825	0.64	0.146	0.09	19.28	0.10	0.002
8.00	1.59	170.91	1.00	170.91	31	820	0.64	0.152	0.09	19.28	0.10	0.001
8.01	1.59	167.93	1.00	167.93	30	813	0.64	0.157	0.10	19.28	0.11	0.000
8.14	1.61	163.25	1.00	163.25	30	809	0.64	0.165	0.10	19.28	0.12	0.002
8.18	1.62	157.77	1.00	157.77	29	800	0.64	0.174	0.11	19.28	0.13	0.001
8.23	1.64	153.11	1.00	153.11	28	794	0.64	0.182	0.12	19.28	0.14	0.001
8.28	1.65	146.02	1.00	146.02	27	768	0.64	0.211	0.15	19.28	0.17	0.001
8.37	1.64	139.00	1.00	139.00	25	729	0.64	0.270	0.20	19.28	0.23	0.003
8.41	1.64	129.56	1.00	129.56	24	684	0.64	0.365	0.30	19.28	0.33	0.002
8.51	1.66	122.62	1.00	122.62	23	659	0.64	0.449	0.39	19.28	0.44	0.005
8.55	1.69	116.79	1.00	116.79	22	652	0.64	0.482	0.44	19.28	0.49	0.003
8.60	1.71	112.81	1.01	113.96	21	646	0.64	0.510	0.47	19.28	0.53	0.003
8.69	1.74	106.84	1.08	114.97	22	638	0.64	0.560	0.51	19.28	0.57	0.006
8.76	1.76	104.37	1.10	114.78	22	635	0.64	0.583	0.52	19.28	0.59	0.005

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
8.81	1.77	103.15	1.11	114.79	22	636	0.64	0.590	0.53	19.28	0.59	0.003
8.89	1.76	105.30	1.10	115.81	22	646	0.64	0.556	0.49	19.28	0.55	0.005
8.95	1.75	107.54	1.09	117.48	22	658	0.64	0.512	0.45	19.28	0.50	0.003
9.00	1.74	112.97	1.06	119.79	23	680	0.64	0.443	0.38	19.28	0.43	0.003
9.08	1.71	119.66	1.01	120.87	23	704	0.64	0.382	0.33	19.28	0.37	0.004
9.13	1.69	126.75	1.00	126.75	24	730	0.64	0.329	0.27	19.28	0.30	0.002
9.20	1.67	134.22	1.00	134.22	25	762	0.64	0.275	0.21	19.28	0.24	0.002
9.28	1.66	141.71	1.00	141.71	26	796	0.64	0.233	0.17	19.28	0.19	0.002
9.32	1.64	149.19	1.00	149.19	27	826	0.64	0.203	0.14	19.28	0.16	0.001
9.41	1.62	156.95	1.00	156.95	28	847	0.64	0.187	0.12	19.28	0.14	0.001
9.46	1.61	164.83	1.00	164.83	30	880	0.64	0.164	0.10	19.28	0.11	0.001
9.53	1.61	171.02	1.00	171.02	31	921	0.64	0.142	0.08	19.28	0.09	0.001
9.60	1.61	174.37	1.00	174.37	32	946	0.64	0.131	0.08	19.28	0.08	0.001
9.65	1.60	175.46	1.00	175.46	32	943	0.64	0.134	0.08	19.28	0.09	0.001
9.74	1.59	175.92	1.00	175.92	32	929	0.64	0.143	0.08	19.28	0.09	0.001
9.78	1.58	175.58	1.00	175.58	31	922	0.64	0.149	0.09	19.28	0.10	0.000
9.87	1.59	173.90	1.00	173.90	31	924	0.64	0.150	0.09	19.28	0.10	0.001
9.91	1.59	172.14	1.00	172.14	31	922	0.64	0.153	0.09	19.28	0.10	0.001
10.01	1.59	171.07	1.00	171.07	31	920	0.64	0.157	0.09	19.28	0.10	0.001
10.06	1.58	171.90	1.00	171.90	31	910	0.64	0.165	0.10	19.28	0.11	0.001
10.19	1.58	172.04	1.00	172.04	31	921	0.64	0.162	0.10	19.28	0.11	0.002
10.24	1.59	175.30	1.00	175.30	32	958	0.64	0.143	0.08	19.28	0.09	0.001
10.33	1.62	177.78	1.00	177.78	32	1005	0.64	0.124	0.07	19.28	0.08	0.001
10.38	1.63	179.85	1.00	179.85	33	1032	0.64	0.114	0.06	19.28	0.07	0.000
10.47	1.63	179.63	1.00	179.63	33	1041	0.64	0.113	0.06	19.28	0.07	0.001
10.54	1.64	179.05	1.00	179.05	33	1048	0.64	0.112	0.06	19.28	0.07	0.001
10.57	1.66	177.18	1.00	177.18	33	1066	0.64	0.107	0.06	19.28	0.07	0.000
10.64	1.70	172.41	1.00	172.41	32	1088	0.64	0.102	0.06	19.28	0.06	0.001
10.70	1.74	165.37	1.06	175.97	33	1094	0.64	0.101	0.05	19.28	0.06	0.000
10.79	1.74	162.82	1.07	174.62	33	1088	0.64	0.105	0.06	19.28	0.06	0.001
10.83	1.69	167.87	1.00	167.87	31	1064	0.64	0.113	0.07	19.28	0.07	0.000
10.93	1.63	176.01	1.00	176.01	32	1047	0.64	0.121	0.07	19.28	0.08	0.001
10.98	1.58	184.09	1.00	184.09	33	1025	0.64	0.131	0.07	19.28	0.08	0.001
11.03	1.55	189.37	1.00	189.37	34	1021	0.64	0.133	0.07	19.28	0.08	0.000
11.12	1.53	193.96	1.00	193.96	34	1023	0.64	0.135	0.07	19.28	0.08	0.001
11.16	1.51	197.68	1.00	197.68	35	1019	0.64	0.138	0.07	19.28	0.08	0.000
11.25	1.50	197.62	1.00	197.62	35	1010	0.64	0.144	0.07	19.28	0.08	0.001
11.35	1.50	196.42	1.00	196.42	34	1003	0.64	0.150	0.08	19.28	0.09	0.001
11.38	1.50	195.46	1.00	195.46	34	1000	0.64	0.153	0.08	19.28	0.09	0.000
11.44	1.50	194.23	1.00	194.23	34	1000	0.64	0.154	0.08	19.28	0.09	0.001
11.49	1.51	192.47	1.00	192.47	34	1000	0.64	0.156	0.08	19.28	0.09	0.001
11.58	1.52	190.25	1.00	190.25	33	1012	0.64	0.152	0.08	19.28	0.09	0.001
11.63	1.56	188.89	1.00	188.89	34	1053	0.64	0.133	0.07	19.28	0.08	0.000
11.72	1.59	187.87	1.00	187.87	34	1097	0.64	0.119	0.06	19.28	0.07	0.001
11.77	1.56	187.44	1.00	187.44	33	1052	0.64	0.138	0.07	19.28	0.08	0.001
11.85	1.48	187.79	1.00	187.79	33	954	0.64	0.197	0.11	19.28	0.12	0.001
11.90	1.38	189.23	1.00	189.23	32	849	0.64	0.319	0.18	19.28	0.20	0.001
11.95	1.38	193.93	1.00	193.93	33	875	0.64	0.283	0.16	19.28	0.18	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
12.05	1.41	198.62	1.00	198.62	34	930	0.64	0.225	0.12	19.28	0.13	0.002
12.09	1.43	203.13	1.00	203.13	35	982	0.64	0.184	0.10	19.28	0.11	0.000
12.15	1.44	204.17	1.00	204.17	35	1007	0.64	0.170	0.09	19.28	0.10	0.001
12.22	1.46	203.72	1.00	203.72	35	1023	0.64	0.163	0.08	19.28	0.09	0.001
12.28	1.45	206.04	1.00	206.04	35	1035	0.64	0.158	0.08	19.28	0.09	0.001
12.38	1.46	207.56	1.00	207.56	36	1050	0.64	0.153	0.08	19.28	0.09	0.001
12.42	1.47	208.88	1.00	208.88	36	1072	0.64	0.144	0.07	19.28	0.08	0.000
12.47	1.50	207.05	1.00	207.05	36	1110	0.64	0.129	0.06	19.28	0.07	0.000
12.56	1.54	205.72	1.00	205.72	36	1158	0.64	0.115	0.06	19.28	0.06	0.001
12.60	1.57	205.46	1.00	205.46	37	1213	0.64	0.100	0.05	19.28	0.05	0.000
12.70	1.59	206.08	1.00	206.08	37	1250	0.64	0.093	0.04	19.28	0.05	0.001
12.75	1.59	208.60	1.00	208.60	37	1267	0.64	0.091	0.04	19.28	0.05	0.000
12.84	1.57	214.83	1.00	214.83	38	1277	0.64	0.090	0.04	19.28	0.05	0.001
12.89	1.56	219.49	1.00	219.49	39	1284	0.64	0.089	0.04	19.28	0.04	0.000
12.94	1.56	222.10	1.00	222.10	39	1304	0.64	0.086	0.04	19.28	0.04	0.000
13.02	1.56	220.30	1.00	220.30	39	1299	0.64	0.088	0.04	19.28	0.04	0.000
13.07	1.55	219.73	1.00	219.73	39	1288	0.64	0.090	0.04	19.28	0.05	0.000
13.14	1.56	218.98	1.00	218.98	39	1299	0.64	0.089	0.04	19.28	0.04	0.000
13.21	1.57	220.40	1.00	220.40	39	1332	0.64	0.084	0.04	19.28	0.04	0.000
13.30	1.59	222.55	1.00	222.55	40	1376	0.64	0.078	0.03	19.28	0.04	0.000
13.35	1.58	224.38	1.00	224.38	40	1381	0.64	0.078	0.03	19.28	0.04	0.000
13.39	1.57	220.21	1.00	220.21	39	1342	0.64	0.085	0.04	19.28	0.04	0.000
13.46	1.56	213.90	1.00	213.90	38	1286	0.64	0.096	0.04	19.28	0.05	0.000
13.53	1.55	208.59	1.00	208.59	37	1246	0.64	0.106	0.05	19.28	0.06	0.000
13.62	1.56	205.19	1.00	205.19	37	1248	0.64	0.107	0.05	19.28	0.06	0.001
13.67	1.58	202.84	1.00	202.84	36	1255	0.64	0.106	0.05	19.28	0.06	0.000
13.72	1.58	200.60	1.00	200.60	36	1258	0.64	0.106	0.05	19.28	0.06	0.000
13.80	1.59	199.84	1.00	199.84	36	1266	0.64	0.105	0.05	19.28	0.06	0.001
13.85	1.60	199.90	1.00	199.90	36	1279	0.64	0.103	0.05	19.28	0.06	0.000
13.99	1.60	198.63	1.00	198.63	36	1290	0.64	0.102	0.05	19.28	0.06	0.001
14.04	1.61	198.54	1.00	198.54	36	1294	0.64	0.102	0.05	19.28	0.06	0.000
14.07	1.60	199.99	1.00	199.99	36	1290	0.64	0.103	0.05	19.28	0.06	0.000
14.13	1.58	202.45	1.00	202.45	36	1287	0.64	0.104	0.05	19.28	0.06	0.000
14.18	1.57	206.40	1.00	206.40	37	1287	0.64	0.105	0.05	19.28	0.06	0.000
14.26	1.55	209.76	1.00	209.76	37	1289	0.64	0.106	0.05	19.28	0.06	0.001
14.31	1.53	215.10	1.00	215.10	38	1288	0.64	0.107	0.05	19.28	0.06	0.000
14.37	1.50	223.50	1.00	223.50	39	1295	0.64	0.106	0.05	19.28	0.05	0.000
14.48	1.48	233.12	1.00	233.12	40	1310	0.64	0.104	0.04	19.28	0.05	0.001
14.55	1.46	240.94	1.00	240.94	41	1327	0.64	0.101	0.04	19.28	0.05	0.000
14.58	1.45	245.65	1.00	245.65	42	1345	0.64	0.098	0.04	19.28	0.04	0.000
14.64	1.45	250.56	1.00	250.56	43	1368	0.64	0.094	0.04	19.28	0.04	0.000
14.73	1.45	255.81	1.00	255.81	44	1397	0.64	0.090	0.04	19.28	0.04	0.000
14.77	1.45	261.76	1.00	261.76	45	1444	0.64	0.083	0.03	19.28	0.04	0.000
14.87	1.45	262.16	1.00	262.16	45	1450	0.64	0.083	0.03	19.28	0.03	0.000
14.92	1.43	261.40	1.00	261.40	45	1409	0.64	0.090	0.03	19.28	0.04	0.000
15.01	1.40	258.68	1.00	258.68	44	1342	0.64	0.104	0.04	19.28	0.05	0.000
15.05	1.38	258.01	1.00	258.01	43	1308	0.64	0.112	0.04	19.28	0.05	0.000
15.10	1.40	252.64	1.00	252.64	43	1316	0.63	0.111	0.04	19.28	0.05	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
15.19	1.42	245.59	1.00	245.59	42	1323	0.63	0.110	0.05	19.28	0.05	0.001
15.24	1.47	233.62	1.00	233.62	40	1334	0.63	0.108	0.05	19.28	0.05	0.000
15.31	1.49	228.96	1.00	228.96	40	1357	0.63	0.104	0.05	19.28	0.05	0.000
15.36	1.51	225.96	1.00	225.96	40	1359	0.63	0.104	0.05	19.28	0.05	0.000
15.45	1.49	228.06	1.00	228.06	40	1354	0.63	0.107	0.05	19.28	0.05	0.001
15.49	1.48	228.68	1.00	228.68	40	1337	0.63	0.111	0.05	19.28	0.05	0.000
15.59	1.48	227.95	1.00	227.95	40	1337	0.63	0.112	0.05	19.28	0.06	0.001
15.62	1.49	225.64	1.00	225.64	39	1347	0.63	0.110	0.05	19.28	0.05	0.000
15.71	1.52	220.85	1.00	220.85	39	1364	0.63	0.108	0.05	19.28	0.05	0.001
15.77	1.53	215.33	1.00	215.33	38	1359	0.63	0.110	0.05	19.28	0.06	0.000
15.86	1.53	211.33	1.00	211.33	37	1338	0.63	0.116	0.05	19.28	0.06	0.001
15.91	1.52	208.73	1.00	208.73	37	1307	0.63	0.124	0.06	19.28	0.07	0.000
15.96	1.52	206.77	1.00	206.77	36	1300	0.63	0.127	0.06	19.28	0.07	0.000
16.04	1.54	202.79	1.00	202.79	36	1306	0.63	0.127	0.06	19.28	0.07	0.001
16.09	1.57	196.62	1.00	196.62	35	1321	0.63	0.123	0.06	19.28	0.07	0.000
16.14	1.62	189.66	1.00	189.66	34	1353	0.63	0.116	0.06	19.28	0.07	0.000
16.23	1.65	184.50	1.00	184.50	34	1379	0.63	0.111	0.06	19.28	0.07	0.001
16.28	1.66	181.28	1.00	181.28	33	1373	0.63	0.113	0.06	19.28	0.07	0.000
16.37	1.66	179.64	1.00	179.64	33	1352	0.63	0.119	0.07	19.28	0.07	0.001
16.41	1.65	178.41	1.00	178.41	33	1334	0.63	0.124	0.07	19.28	0.08	0.000
16.50	1.66	178.21	1.00	178.21	33	1347	0.63	0.122	0.07	19.28	0.08	0.001
16.60	1.66	177.71	1.00	177.71	33	1357	0.63	0.121	0.07	19.28	0.07	0.001
16.65	1.66	178.04	1.00	178.04	33	1360	0.63	0.121	0.07	19.28	0.07	0.000
16.69	1.65	179.36	1.00	179.36	33	1361	0.63	0.121	0.07	19.28	0.07	0.000
16.74	1.64	181.90	1.00	181.90	33	1365	0.63	0.121	0.07	19.28	0.07	0.000
16.83	1.64	184.27	1.00	184.27	34	1372	0.63	0.120	0.06	19.28	0.07	0.001
16.88	1.63	186.67	1.00	186.67	34	1377	0.63	0.119	0.06	19.28	0.07	0.000
16.96	1.62	189.62	1.00	189.62	34	1384	0.63	0.119	0.06	19.28	0.07	0.001
17.05	1.61	193.10	1.00	193.10	35	1393	0.63	0.118	0.06	19.28	0.07	0.001
17.07	1.59	200.02	1.00	200.02	36	1411	0.63	0.114	0.06	19.28	0.06	0.000
17.17	1.57	206.20	1.00	206.20	37	1430	0.63	0.111	0.05	19.28	0.06	0.001
17.20	1.55	214.23	1.00	214.23	38	1450	0.63	0.107	0.05	19.28	0.06	0.000
17.27	1.54	220.01	1.00	220.01	39	1461	0.63	0.105	0.05	19.28	0.05	0.000
17.34	1.52	226.70	1.00	226.70	40	1474	0.63	0.104	0.05	19.28	0.05	0.000
17.44	1.50	232.14	1.00	232.14	41	1482	0.63	0.103	0.04	19.28	0.05	0.001
17.51	1.49	236.51	1.00	236.51	41	1486	0.63	0.103	0.04	19.28	0.05	0.000
17.54	1.47	238.97	1.00	238.97	41	1475	0.63	0.105	0.04	19.28	0.05	0.000
17.61	1.47	238.29	1.00	238.29	41	1466	0.63	0.108	0.05	19.28	0.05	0.000
17.71	1.47	236.27	1.00	236.27	41	1458	0.63	0.111	0.05	19.28	0.05	0.001
17.73	1.47	233.07	1.00	233.07	40	1452	0.63	0.112	0.05	19.28	0.05	0.000
17.80	1.48	228.93	1.00	228.93	40	1443	0.63	0.115	0.05	19.28	0.06	0.001
17.89	1.48	224.47	1.00	224.47	39	1423	0.63	0.121	0.05	19.28	0.06	0.001
17.94	1.48	219.40	1.00	219.40	38	1393	0.64	0.129	0.06	19.28	0.07	0.000
17.98	1.49	212.40	1.00	212.40	37	1360	0.64	0.139	0.07	19.28	0.07	0.000
18.07	1.50	204.99	1.00	204.99	36	1339	0.64	0.148	0.07	19.28	0.08	0.001
18.12	1.53	195.60	1.00	195.60	35	1326	0.64	0.153	0.08	19.28	0.09	0.001
18.25	1.56	188.37	1.00	188.37	34	1324	0.64	0.155	0.08	19.28	0.09	0.001
18.26	1.59	181.00	1.00	181.00	32	1318	0.64	0.158	0.09	19.28	0.10	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
18.32	1.59	175.24	1.00	175.24	32	1293	0.64	0.169	0.10	19.28	0.11	0.001
18.40	1.62	166.87	1.00	166.87	30	1269	0.64	0.181	0.11	19.28	0.12	0.001
18.49	1.64	159.47	1.00	159.47	29	1248	0.64	0.194	0.12	19.28	0.14	0.002
18.53	1.67	152.81	1.00	152.81	28	1248	0.64	0.194	0.13	19.28	0.14	0.001
18.59	1.69	147.87	1.00	147.87	28	1247	0.64	0.196	0.13	19.28	0.15	0.001
18.67	1.79	126.87	1.14	144.87	28	1215	0.64	0.216	0.14	19.28	0.16	0.002
18.72	1.82	121.71	1.17	143.00	28	1214	0.64	0.218	0.15	19.28	0.16	0.001
18.77	1.83	117.63	1.19	139.56	27	1194	0.64	0.233	0.16	19.28	0.18	0.001
18.86	1.76	129.90	1.10	143.09	27	1204	0.64	0.228	0.16	19.28	0.18	0.002
18.90	1.75	127.63	1.09	139.32	26	1175	0.64	0.251	0.18	19.28	0.20	0.001
19.00	1.76	125.88	1.09	137.76	26	1164	0.64	0.262	0.19	19.28	0.21	0.002
19.04	1.77	122.63	1.11	136.01	26	1150	0.64	0.275	0.20	19.28	0.23	0.001
19.15	1.78	119.68	1.12	134.54	26	1142	0.64	0.286	0.21	19.28	0.24	0.003
19.18	1.79	116.56	1.14	133.08	26	1133	0.64	0.296	0.22	19.28	0.25	0.001
19.23	1.81	113.20	1.16	131.46	25	1127	0.64	0.304	0.23	19.28	0.25	0.001
19.31	1.84	108.07	1.19	128.46	25	1119	0.64	0.316	0.24	19.28	0.27	0.003
19.43	1.86	103.10	1.21	124.96	25	1111	0.64	0.328	0.25	19.28	0.28	0.004
19.46	1.88	100.10	1.23	122.75	24	1108	0.64	0.333	0.26	19.28	0.29	0.001
19.50	1.89	99.19	1.23	122.35	24	1115	0.64	0.327	0.26	19.28	0.29	0.001
19.61	1.90	98.70	1.24	122.30	25	1125	0.64	0.319	0.25	19.28	0.28	0.004
19.65	1.91	99.03	1.24	122.99	25	1137	0.64	0.308	0.24	19.28	0.27	0.001
19.69	1.90	100.91	1.24	124.99	25	1152	0.64	0.294	0.22	19.28	0.25	0.001
19.78	1.90	103.00	1.23	127.07	25	1168	0.64	0.282	0.21	19.28	0.24	0.003
19.82	1.87	107.63	1.22	131.16	26	1187	0.64	0.266	0.19	19.28	0.22	0.001
19.93	1.85	111.32	1.20	134.03	26	1201	0.65	0.257	0.18	19.28	0.21	0.003
19.97	1.83	116.40	1.18	137.51	27	1215	0.65	0.248	0.17	19.28	0.20	0.001
20.05	1.81	120.04	1.16	139.27	27	1221	0.65	0.246	0.17	19.28	0.19	0.002
20.11	1.78	123.87	1.13	140.43	27	1225	0.65	0.244	0.17	19.28	0.19	0.001
20.15	1.77	126.50	1.11	140.21	27	1222	0.65	0.247	0.17	19.28	0.19	0.001
20.24	1.75	127.53	1.09	138.90	26	1215	0.65	0.255	0.18	19.28	0.20	0.002
20.33	1.75	126.90	1.08	136.95	26	1203	0.65	0.267	0.20	19.28	0.22	0.002
20.42	1.75	125.39	1.08	135.45	26	1192	0.65	0.279	0.21	19.28	0.23	0.002
20.43	1.75	123.26	1.09	133.90	25	1178	0.65	0.292	0.22	19.28	0.24	0.000
20.52	1.76	120.38	1.10	132.23	25	1165	0.65	0.307	0.23	19.28	0.26	0.003
20.56	1.77	117.40	1.11	130.29	25	1149	0.65	0.326	0.25	19.28	0.28	0.002
20.61	1.78	113.18	1.13	127.91	25	1131	0.65	0.349	0.27	19.28	0.30	0.002
20.70	1.79	109.24	1.15	125.31	24	1114	0.65	0.374	0.30	19.28	0.33	0.004
20.75	1.82	103.37	1.17	121.26	24	1089	0.65	0.411	0.34	19.28	0.38	0.002
20.84	1.84	97.73	1.20	116.91	23	1068	0.65	0.451	0.38	19.28	0.43	0.005
20.89	1.87	92.11	1.22	112.18	22	1046	0.65	0.497	0.44	19.28	0.49	0.003
20.93	1.90	86.34	1.24	106.94	21	1021	0.65	0.555	0.51	19.28	0.57	0.003
21.02	1.94	79.96	1.26	100.99	21	996	0.65	0.629	0.61	19.28	0.68	0.008
21.07	1.99	72.52	1.30	94.17	20	966	0.65	0.730	0.75	19.28	0.84	0.005
21.16	2.06	64.41	1.36	87.76	19	940	0.65	0.839	0.91	19.28	1.02	0.010
21.26	2.12	57.91	1.45	83.91	18	921	0.65	0.940	1.05	19.28	1.17	0.015
21.30	2.17	53.08	1.55	82.47	18	910	0.65	1.009	1.12	19.28	1.25	0.006
21.35	2.21	50.55	1.63	82.43	19	906	0.65	1.038	1.13	19.28	1.26	0.007
21.39	2.25	47.07	1.76	82.88	19	898	0.65	1.093	1.15	19.28	1.29	0.007

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
21.53	2.30	43.88	1.91	83.72	20	890	0.65	1.161	1.19	19.28	1.33	0.022
21.56	2.32	41.97	2.02	84.85	20	886	0.65	1.195	1.18	19.28	1.32	0.005
21.63	2.33	42.05	2.03	85.53	20	893	0.65	1.154	1.13	19.28	1.26	0.009
21.67	2.32	42.90	2.02	86.67	21	908	0.65	1.062	1.02	19.28	1.15	0.007
21.77	2.31	44.20	1.98	87.47	21	926	0.65	0.973	0.93	19.28	1.05	0.012
21.81	2.31	44.94	1.97	88.49	21	939	0.65	0.909	0.86	19.28	0.96	0.005
21.88	2.30	45.85	1.94	88.96	21	951	0.65	0.861	0.81	19.28	0.91	0.008
21.95	2.29	46.80	1.90	89.15	21	960	0.65	0.826	0.78	19.28	0.88	0.007
22.00	2.27	48.61	1.83	88.87	21	971	0.65	0.789	0.76	19.28	0.85	0.005
22.09	2.25	50.32	1.76	88.48	20	979	0.65	0.764	0.75	19.28	0.84	0.009
22.14	2.23	52.24	1.70	88.60	20	990	0.65	0.729	0.72	19.28	0.80	0.005
22.22	2.21	54.11	1.65	89.09	20	1003	0.66	0.690	0.68	19.28	0.76	0.008
22.27	2.20	55.57	1.62	89.87	20	1016	0.66	0.654	0.64	19.28	0.72	0.004
22.31	2.19	56.79	1.60	90.90	20	1030	0.66	0.616	0.60	19.28	0.67	0.003
22.41	2.19	57.50	1.60	91.76	21	1043	0.66	0.588	0.57	19.28	0.63	0.007
22.45	2.19	58.29	1.59	92.76	21	1057	0.66	0.558	0.53	19.28	0.59	0.003
22.52	2.19	59.21	1.58	93.60	21	1069	0.66	0.534	0.50	19.28	0.56	0.005
22.60	2.18	60.18	1.57	94.41	21	1081	0.66	0.512	0.48	19.28	0.54	0.005
22.64	2.17	61.82	1.54	95.35	21	1094	0.66	0.488	0.45	19.28	0.51	0.002
22.73	2.16	62.95	1.52	95.99	21	1105	0.66	0.473	0.44	19.28	0.49	0.005
22.77	2.15	64.34	1.51	96.92	21	1116	0.66	0.454	0.42	19.28	0.47	0.002
22.87	2.15	64.84	1.50	97.32	21	1124	0.66	0.446	0.41	19.28	0.46	0.005
22.92	2.15	65.03	1.50	97.37	21	1126	0.66	0.444	0.41	19.28	0.46	0.003
23.01	2.15	63.69	1.51	96.39	21	1118	0.66	0.461	0.43	19.28	0.48	0.005
23.10	2.17	61.76	1.54	95.16	21	1107	0.66	0.486	0.45	19.28	0.51	0.006
23.14	2.19	58.92	1.59	93.97	21	1092	0.66	0.517	0.48	19.28	0.54	0.002
23.19	2.22	56.41	1.65	93.28	21	1080	0.66	0.544	0.51	19.28	0.57	0.004
23.23	2.26	52.60	1.77	93.09	21	1064	0.66	0.584	0.54	19.28	0.60	0.003
23.31	2.30	48.82	1.92	93.66	22	1050	0.66	0.626	0.56	19.28	0.62	0.006
23.37	2.35	44.81	2.12	95.18	23	1033	0.66	0.677	0.58	19.28	0.64	0.005
23.47	2.38	42.32	2.29	96.81	24	1026	0.66	0.707	0.58	19.28	0.65	0.007
23.51	2.40	41.25	2.38	98.27	24	1027	0.66	0.708	0.56	19.28	0.63	0.003
23.57	2.41	41.56	2.40	99.76	25	1041	0.66	0.667	0.52	19.28	0.58	0.004
23.64	2.39	42.93	2.35	100.71	25	1063	0.66	0.612	0.47	19.28	0.53	0.005
23.70	2.38	44.82	2.26	101.35	25	1086	0.66	0.558	0.43	19.28	0.48	0.003
23.76	2.36	46.84	2.16	101.31	24	1106	0.66	0.519	0.41	19.28	0.46	0.004
23.84	2.34	48.40	2.09	101.16	24	1120	0.66	0.495	0.39	19.28	0.44	0.004
23.93	2.33	49.21	2.05	101.06	24	1129	0.66	0.483	0.39	19.28	0.43	0.005
23.97	2.33	49.52	2.04	101.03	24	1132	0.66	0.478	0.38	19.28	0.43	0.002
24.02	2.32	49.79	2.02	100.66	24	1133	0.66	0.479	0.39	19.28	0.43	0.003
24.11	2.31	50.38	1.98	99.59	24	1132	0.66	0.484	0.40	19.28	0.44	0.005
24.18	2.30	50.93	1.93	98.32	23	1128	0.66	0.494	0.41	19.28	0.46	0.004
24.25	2.29	51.25	1.90	97.44	23	1126	0.66	0.502	0.43	19.28	0.48	0.004
24.30	2.29	51.76	1.87	97.03	23	1127	0.66	0.502	0.43	19.28	0.48	0.002
24.34	2.27	52.90	1.82	96.46	22	1130	0.66	0.498	0.43	19.28	0.49	0.003
24.44	2.26	53.70	1.77	95.25	22	1127	0.66	0.509	0.45	19.28	0.51	0.006
24.48	2.25	52.85	1.76	92.76	21	1101	0.66	0.564	0.52	19.28	0.58	0.003
24.57	2.26	50.40	1.80	90.68	21	1073	0.66	0.638	0.60	19.28	0.67	0.007

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
24.62	2.30	46.73	1.93	90.09	21	1047	0.66	0.716	0.67	19.28	0.75	0.005
24.72	2.34	43.82	2.09	91.64	22	1041	0.66	0.745	0.67	19.28	0.75	0.008
24.76	2.37	41.62	2.24	93.37	23	1036	0.66	0.764	0.66	19.28	0.74	0.004
24.80	2.40	40.18	2.35	94.58	23	1032	0.66	0.781	0.65	19.28	0.73	0.004
24.91	2.40	39.69	2.39	94.78	23	1032	0.66	0.789	0.66	19.28	0.73	0.010
24.96	2.42	38.60	2.47	95.37	24	1026	0.66	0.814	0.67	19.28	0.74	0.004
25.00	2.41	39.13	2.43	94.93	23	1030	0.66	0.803	0.66	19.28	0.74	0.004
25.09	2.41	39.43	2.41	94.84	23	1035	0.66	0.791	0.66	19.28	0.73	0.008
25.14	2.38	40.98	2.29	93.96	23	1045	0.66	0.758	0.64	19.28	0.72	0.004
25.24	2.38	41.51	2.25	93.48	23	1049	0.67	0.750	0.64	19.28	0.72	0.008
25.28	2.38	41.30	2.27	93.75	23	1051	0.67	0.748	0.64	19.28	0.71	0.004
25.35	2.38	41.09	2.26	92.80	23	1044	0.67	0.776	0.67	19.28	0.75	0.006
25.42	2.36	41.29	2.16	89.23	22	1022	0.67	0.865	0.79	19.28	0.89	0.007
25.46	2.32	42.24	2.02	85.45	20	1002	0.67	0.957	0.94	19.28	1.05	0.005
25.54	2.32	42.31	2.00	84.55	20	997	0.67	0.986	0.98	19.28	1.10	0.010
25.60	2.35	41.19	2.13	87.79	21	1015	0.67	0.907	0.85	19.28	0.95	0.007
25.68	2.39	39.50	2.35	92.68	23	1038	0.67	0.822	0.70	19.28	0.79	0.007
25.74	2.39	41.14	2.32	95.65	23	1077	0.67	0.696	0.58	19.28	0.64	0.005
25.82	2.34	46.06	2.08	95.86	23	1124	0.67	0.577	0.49	19.28	0.55	0.005
25.89	2.27	52.72	1.81	95.61	22	1170	0.67	0.489	0.43	19.28	0.48	0.004
25.94	2.23	57.15	1.68	96.13	22	1197	0.67	0.448	0.40	19.28	0.45	0.003
26.01	2.21	59.44	1.63	96.91	22	1214	0.67	0.425	0.38	19.28	0.43	0.004
26.07	2.19	60.96	1.60	97.41	22	1225	0.67	0.412	0.37	19.28	0.41	0.003
26.16	2.18	62.15	1.57	97.35	22	1228	0.67	0.410	0.37	19.28	0.41	0.004
26.22	2.16	63.46	1.53	96.90	22	1224	0.67	0.417	0.38	19.28	0.43	0.003
26.26	2.14	64.94	1.48	96.22	21	1214	0.67	0.433	0.40	19.28	0.45	0.002
26.35	2.12	66.41	1.44	95.84	21	1206	0.67	0.448	0.43	19.28	0.48	0.005
26.39	2.09	68.32	1.41	96.06	21	1200	0.67	0.458	0.44	19.28	0.49	0.003
26.45	2.07	69.89	1.38	96.48	21	1196	0.67	0.466	0.45	19.28	0.50	0.003
26.53	2.05	71.47	1.36	96.99	21	1192	0.67	0.476	0.46	19.28	0.51	0.005
26.58	2.04	72.34	1.34	96.94	20	1182	0.67	0.496	0.48	19.28	0.54	0.003
26.67	2.03	72.45	1.33	96.37	20	1169	0.67	0.521	0.51	19.28	0.57	0.006
26.71	2.02	71.79	1.33	95.13	20	1151	0.67	0.560	0.56	19.28	0.63	0.003
26.81	2.02	70.70	1.33	93.76	20	1138	0.67	0.593	0.60	19.28	0.68	0.008
26.84	2.03	69.23	1.33	92.18	19	1124	0.67	0.628	0.65	19.28	0.73	0.002
26.91	2.04	67.56	1.34	90.76	19	1117	0.67	0.648	0.68	19.28	0.76	0.006
26.99	2.05	65.81	1.36	89.46	19	1113	0.67	0.663	0.70	19.28	0.79	0.008
27.05	2.06	65.06	1.37	89.12	19	1116	0.67	0.658	0.70	19.28	0.78	0.005
27.13	2.07	64.70	1.37	88.93	19	1118	0.67	0.657	0.70	19.28	0.78	0.007
27.24	2.07	64.79	1.38	89.12	19	1123	0.67	0.648	0.69	19.28	0.77	0.010
27.27	2.07	64.73	1.38	89.46	19	1132	0.67	0.629	0.66	19.28	0.74	0.003
27.32	2.08	64.72	1.40	90.39	19	1151	0.67	0.587	0.61	19.28	0.68	0.003
27.37	2.09	64.64	1.41	91.20	20	1167	0.67	0.554	0.56	19.28	0.63	0.004
27.44	2.10	64.93	1.42	92.01	20	1181	0.67	0.529	0.53	19.28	0.60	0.005
27.50	2.09	65.99	1.41	92.83	20	1190	0.67	0.515	0.51	19.28	0.58	0.004
27.59	2.08	66.98	1.40	93.57	20	1199	0.67	0.503	0.50	19.28	0.56	0.006
27.64	2.08	67.24	1.40	93.92	20	1205	0.67	0.495	0.49	19.28	0.55	0.003
27.73	2.09	66.75	1.41	93.84	20	1209	0.67	0.490	0.48	19.28	0.54	0.006

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
27.78	2.10	66.28	1.42	93.84	20	1214	0.67	0.484	0.48	19.28	0.53	0.003
27.86	2.10	65.74	1.42	93.67	20	1217	0.67	0.482	0.47	19.28	0.53	0.005
27.92	2.11	65.39	1.43	93.53	20	1218	0.67	0.482	0.47	19.28	0.53	0.004
27.97	2.11	64.62	1.44	93.03	20	1215	0.67	0.488	0.48	19.28	0.54	0.003
28.05	2.12	63.93	1.45	92.44	20	1211	0.67	0.498	0.49	19.28	0.55	0.006
28.10	2.12	62.78	1.46	91.50	20	1203	0.67	0.515	0.51	19.28	0.58	0.003
28.18	2.13	61.61	1.47	90.50	20	1194	0.67	0.534	0.54	19.28	0.60	0.006
28.27	2.14	60.61	1.48	89.67	20	1186	0.67	0.551	0.56	19.28	0.63	0.006
28.29	2.14	59.84	1.49	89.11	20	1180	0.67	0.564	0.58	19.28	0.65	0.002
28.38	2.15	59.38	1.49	88.74	20	1179	0.67	0.571	0.59	19.28	0.66	0.008
28.43	2.15	58.98	1.50	88.45	20	1176	0.67	0.577	0.59	19.28	0.67	0.003
28.56	2.15	58.58	1.51	88.17	19	1177	0.67	0.582	0.60	19.28	0.67	0.011
28.61	2.16	58.00	1.51	87.86	19	1175	0.67	0.589	0.61	19.28	0.68	0.004
28.70	2.16	56.99	1.53	87.41	19	1172	0.67	0.598	0.62	19.28	0.69	0.007
28.75	2.19	54.92	1.58	86.74	19	1164	0.67	0.618	0.64	19.28	0.72	0.004
28.84	2.21	52.58	1.64	86.49	20	1160	0.67	0.632	0.65	19.28	0.73	0.008
28.89	2.27	47.98	1.82	87.13	20	1149	0.67	0.660	0.65	19.28	0.73	0.004
28.95	2.30	46.07	1.93	89.05	21	1158	0.67	0.642	0.61	19.28	0.68	0.005
29.03	2.33	44.94	2.02	90.92	22	1169	0.67	0.619	0.56	19.28	0.63	0.006
29.08	2.30	47.93	1.91	91.44	21	1197	0.67	0.561	0.51	19.28	0.58	0.004
29.18	2.27	50.08	1.83	91.68	21	1215	0.67	0.529	0.49	19.28	0.55	0.006
29.21	2.24	53.64	1.72	92.35	21	1241	0.67	0.486	0.45	19.28	0.51	0.002
29.31	2.22	56.38	1.65	93.29	21	1264	0.67	0.455	0.42	19.28	0.48	0.005
29.36	2.20	58.56	1.62	94.58	21	1286	0.67	0.426	0.39	19.28	0.44	0.003
29.40	2.20	59.07	1.62	95.53	22	1300	0.67	0.409	0.37	19.28	0.42	0.002
29.50	2.20	59.11	1.63	96.09	22	1310	0.67	0.400	0.36	19.28	0.41	0.005
29.55	2.20	60.11	1.61	96.76	22	1321	0.67	0.388	0.35	19.28	0.39	0.002
29.62	2.18	62.54	1.56	97.82	22	1340	0.67	0.370	0.33	19.28	0.37	0.003
29.68	2.16	65.48	1.52	99.52	22	1363	0.67	0.347	0.31	19.28	0.35	0.002
29.73	2.14	68.18	1.49	101.57	22	1390	0.67	0.324	0.28	19.28	0.32	0.002
29.81	2.14	69.44	1.49	103.13	23	1413	0.67	0.307	0.26	19.28	0.30	0.003
29.87	2.14	70.30	1.49	104.45	23	1433	0.67	0.293	0.25	19.28	0.28	0.002
29.95	2.14	71.28	1.48	105.40	23	1448	0.67	0.284	0.24	19.28	0.27	0.003
30.01	2.11	74.92	1.43	107.37	23	1466	0.67	0.274	0.23	19.28	0.25	0.002
30.10	2.07	79.43	1.38	109.92	24	1481	0.67	0.266	0.22	19.28	0.24	0.003
30.13	2.02	86.22	1.33	114.57	24	1498	0.67	0.256	0.21	19.28	0.23	0.001
30.19	1.97	93.49	1.29	120.35	25	1513	0.67	0.248	0.19	19.28	0.21	0.001
30.25	1.93	102.07	1.25	127.99	26	1543	0.67	0.234	0.17	19.28	0.19	0.001
30.32	1.91	110.34	1.24	137.20	28	1633	0.67	0.197	0.13	19.28	0.15	0.001
30.46	1.91	115.91	1.24	144.25	29	1724	0.67	0.169	0.11	19.28	0.12	0.002
30.47	1.94	120.31	1.26	151.61	31	1852	0.67	0.139	0.08	19.28	0.09	0.000
30.55	1.94	122.72	1.27	155.41	32	1918	0.67	0.127	0.07	19.28	0.08	0.001
30.60	1.94	124.91	1.27	158.19	32	1954	0.67	0.122	0.07	19.28	0.08	0.001
30.65	1.92	127.37	1.25	159.20	32	1924	0.67	0.127	0.07	19.28	0.08	0.000
30.74	1.90	129.13	1.24	159.64	32	1897	0.67	0.132	0.08	19.28	0.08	0.001
30.77	1.87	135.39	1.21	164.44	33	1903	0.67	0.131	0.07	19.28	0.08	0.000
30.87	1.92	124.21	1.25	155.04	31	1877	0.67	0.136	0.08	19.28	0.09	0.001
30.93	1.95	116.43	1.27	147.84	30	1847	0.67	0.143	0.09	19.28	0.10	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
30.97	1.99	106.30	1.30	138.01	29	1786	0.67	0.157	0.10	19.28	0.11	0.001
31.05	1.94	112.85	1.26	142.37	29	1763	0.67	0.163	0.10	19.28	0.12	0.001
31.11	1.88	119.22	1.22	145.98	29	1721	0.67	0.175	0.11	19.28	0.13	0.001
31.20	1.84	123.63	1.19	147.73	29	1691	0.67	0.185	0.12	19.28	0.13	0.002
31.24	1.79	132.32	1.14	151.26	29	1681	0.67	0.189	0.12	19.28	0.13	0.001
31.31	1.74	143.48	1.07	153.48	29	1697	0.67	0.184	0.12	19.28	0.13	0.001
31.41	1.69	156.20	1.00	156.20	29	1714	0.67	0.180	0.12	19.28	0.13	0.002
31.43	1.65	167.52	1.00	167.52	31	1730	0.67	0.175	0.10	19.28	0.12	0.000
31.51	1.62	175.45	1.00	175.45	32	1751	0.67	0.170	0.10	19.28	0.11	0.001
31.57	1.59	185.10	1.00	185.10	33	1782	0.67	0.162	0.09	19.28	0.10	0.001
31.66	1.57	192.31	1.00	192.31	34	1806	0.67	0.157	0.08	19.28	0.09	0.001
31.70	1.55	199.40	1.00	199.40	35	1824	0.67	0.153	0.08	19.28	0.09	0.000
31.80	1.53	203.67	1.00	203.67	36	1821	0.67	0.154	0.08	19.28	0.09	0.001
31.84	1.52	209.07	1.00	209.07	37	1861	0.67	0.146	0.07	19.28	0.08	0.000
31.89	1.60	212.83	1.00	212.83	38	2097	0.67	0.109	0.05	19.28	0.06	0.000
31.97	1.72	212.46	1.04	220.68	42	2482	0.67	0.076	0.03	19.28	0.04	0.000
32.03	1.84	208.00	1.19	247.22	48	2858	0.67	0.058	0.02	19.28	0.02	0.000
32.13	1.89	204.84	1.23	251.46	50	3035	0.67	0.053	0.02	19.28	0.02	0.000
32.17	1.93	186.20	1.26	234.59	48	2963	0.67	0.055	0.02	19.28	0.02	0.000
32.22	1.88	192.59	1.22	235.49	47	2830	0.67	0.060	0.02	19.28	0.02	0.000
32.30	1.81	206.25	1.17	240.42	47	2751	0.67	0.063	0.02	19.28	0.03	0.000
32.36	1.69	250.55	1.00	250.55	47	2798	0.67	0.061	0.02	19.28	0.02	0.000
32.42	1.64	274.41	1.00	274.41	50	2865	0.67	0.059	0.02	19.28	0.02	0.000
32.50	1.68	260.06	1.00	260.06	48	2863	0.67	0.059	0.02	19.28	0.02	0.000
32.58	1.78	222.21	1.12	249.75	48	2830	0.67	0.060	0.02	19.28	0.02	0.000
32.63	1.92	176.50	1.25	220.11	44	2755	0.67	0.064	0.02	19.28	0.03	0.000
32.68	1.95	166.30	1.27	210.87	43	2721	0.67	0.065	0.03	19.28	0.03	0.000
32.74	1.93	164.47	1.26	206.41	42	2619	0.67	0.070	0.03	19.28	0.03	0.000
32.82	1.85	171.88	1.20	205.94	40	2437	0.67	0.081	0.03	19.28	0.04	0.000
32.94	1.78	173.98	1.12	195.46	37	2230	0.67	0.099	0.05	19.28	0.05	0.001
32.97	1.71	178.55	1.02	181.49	34	2090	0.67	0.114	0.06	19.28	0.07	0.000
33.01	1.69	180.54	1.00	180.54	34	2044	0.67	0.121	0.06	19.28	0.07	0.000
33.09	1.67	182.70	1.00	182.70	34	2009	0.67	0.127	0.07	19.28	0.08	0.001
33.14	1.65	185.26	1.00	185.26	34	1975	0.67	0.132	0.07	19.28	0.08	0.000
33.23	1.64	185.19	1.00	185.19	34	1943	0.67	0.139	0.07	19.28	0.08	0.001
33.33	1.63	184.02	1.00	184.02	33	1910	0.67	0.145	0.08	19.28	0.09	0.001
33.37	1.62	182.60	1.00	182.60	33	1883	0.67	0.151	0.08	19.28	0.09	0.000
33.42	1.61	182.30	1.00	182.30	33	1869	0.67	0.155	0.08	19.28	0.09	0.001
33.47	1.61	182.18	1.00	182.18	33	1855	0.67	0.158	0.09	19.28	0.10	0.000
33.58	1.61	181.65	1.00	181.65	33	1847	0.67	0.161	0.09	19.28	0.10	0.001
33.61	1.60	181.19	1.00	181.19	33	1838	0.67	0.163	0.09	19.28	0.10	0.000
33.68	1.61	179.31	1.00	179.31	32	1829	0.67	0.166	0.09	19.28	0.10	0.001
33.74	1.62	175.73	1.00	175.73	32	1817	0.67	0.169	0.10	19.28	0.11	0.001
33.80	1.64	170.35	1.00	170.35	31	1805	0.67	0.173	0.10	19.28	0.11	0.001
33.89	1.65	165.24	1.00	165.24	30	1800	0.67	0.175	0.11	19.28	0.12	0.001
33.95	1.66	163.09	1.00	163.09	30	1800	0.67	0.175	0.11	19.28	0.12	0.001
34.02	1.66	164.29	1.00	164.29	30	1807	0.67	0.174	0.11	19.28	0.12	0.001
34.06	1.64	170.32	1.00	170.32	31	1828	0.67	0.169	0.10	19.28	0.11	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
34.16	1.62	177.13	1.00	177.13	32	1859	0.67	0.161	0.09	19.28	0.10	0.001
34.21	1.61	185.04	1.00	185.04	33	1903	0.67	0.152	0.08	19.28	0.09	0.001
34.29	1.60	190.07	1.00	190.07	34	1936	0.67	0.146	0.08	19.28	0.09	0.001
34.33	1.59	195.51	1.00	195.51	35	1966	0.67	0.140	0.07	19.28	0.08	0.000
34.39	1.57	201.93	1.00	201.93	36	1987	0.67	0.137	0.07	19.28	0.08	0.000
34.48	1.55	208.63	1.00	208.63	37	2005	0.67	0.134	0.06	19.28	0.07	0.001
34.53	1.53	214.92	1.00	214.92	38	2019	0.67	0.132	0.06	19.28	0.07	0.000
34.62	1.52	218.48	1.00	218.48	38	2032	0.67	0.130	0.06	19.28	0.07	0.001
34.67	1.53	219.99	1.00	219.99	39	2052	0.67	0.127	0.06	19.28	0.06	0.000
34.76	1.54	219.22	1.00	219.22	39	2075	0.67	0.124	0.06	19.28	0.06	0.001
34.78	1.55	217.45	1.00	217.45	39	2098	0.67	0.121	0.06	19.28	0.06	0.000
34.85	1.57	215.40	1.00	215.40	38	2120	0.67	0.118	0.05	19.28	0.06	0.000
34.94	1.58	212.56	1.00	212.56	38	2139	0.67	0.116	0.05	19.28	0.06	0.001
34.99	1.60	210.01	1.00	210.01	38	2153	0.67	0.115	0.05	19.28	0.06	0.000
35.08	1.60	207.74	1.00	207.74	38	2154	0.67	0.115	0.05	19.28	0.06	0.001
35.13	1.61	206.15	1.00	206.15	37	2144	0.67	0.117	0.06	19.28	0.06	0.000
35.17	1.60	204.85	1.00	204.85	37	2122	0.67	0.119	0.06	19.28	0.06	0.000
35.30	1.60	204.28	1.00	204.28	37	2113	0.67	0.121	0.06	19.28	0.07	0.001
35.32	1.61	206.17	1.00	206.17	37	2165	0.67	0.115	0.05	19.28	0.06	0.000
35.40	1.62	210.55	1.00	210.55	38	2247	0.67	0.105	0.05	19.28	0.05	0.001
35.45	1.64	214.48	1.00	214.48	39	2329	0.67	0.098	0.04	19.28	0.05	0.000
35.51	1.64	216.62	1.00	216.62	40	2357	0.67	0.095	0.04	19.28	0.05	0.000
35.60	1.63	217.47	1.00	217.47	40	2365	0.67	0.095	0.04	19.28	0.05	0.001
35.64	1.65	210.76	1.00	210.76	39	2336	0.67	0.098	0.04	19.28	0.05	0.000
35.73	1.67	202.56	1.00	202.56	37	2310	0.67	0.100	0.05	19.28	0.05	0.001
35.77	1.70	190.67	1.00	190.67	36	2276	0.67	0.104	0.05	19.28	0.06	0.000
35.85	1.70	185.77	1.00	185.77	35	2244	0.67	0.107	0.06	19.28	0.06	0.001
35.90	1.69	180.84	1.00	180.84	34	2149	0.67	0.119	0.06	19.28	0.07	0.000
36.01	1.67	177.21	1.00	177.21	33	2047	0.67	0.134	0.07	19.28	0.08	0.001
36.05	1.65	173.76	1.00	173.76	32	1947	0.67	0.153	0.09	19.28	0.10	0.001
36.10	1.65	168.79	1.00	168.79	31	1890	0.67	0.165	0.10	19.28	0.11	0.001
36.20	1.65	163.03	1.00	163.03	30	1840	0.67	0.179	0.11	19.28	0.12	0.001
36.23	1.66	156.81	1.00	156.81	29	1795	0.67	0.192	0.12	19.28	0.14	0.001
36.32	1.67	152.85	1.00	152.85	28	1770	0.67	0.200	0.13	19.28	0.15	0.002
36.38	1.67	150.98	1.00	150.98	28	1751	0.67	0.207	0.14	19.28	0.16	0.001
36.47	1.67	151.39	1.00	151.39	28	1745	0.67	0.210	0.14	19.28	0.16	0.002
36.51	1.66	153.13	1.00	153.13	28	1749	0.67	0.209	0.14	19.28	0.15	0.001
36.56	1.65	155.57	1.00	155.57	29	1762	0.67	0.205	0.13	19.28	0.15	0.001
36.66	1.65	157.16	1.00	157.16	29	1778	0.67	0.200	0.13	19.28	0.14	0.002
36.71	1.65	157.77	1.00	157.77	29	1792	0.67	0.196	0.13	19.28	0.14	0.001
36.75	1.66	156.82	1.00	156.82	29	1806	0.67	0.192	0.12	19.28	0.14	0.001
36.84	1.67	155.51	1.00	155.51	29	1818	0.67	0.188	0.12	19.28	0.14	0.001
36.90	1.68	154.18	1.00	154.18	29	1830	0.67	0.185	0.12	19.28	0.14	0.001
36.97	1.69	153.36	1.00	153.36	28	1836	0.67	0.184	0.12	19.28	0.13	0.001
37.03	1.69	153.41	1.00	153.41	29	1842	0.67	0.183	0.12	19.28	0.13	0.001
37.11	1.69	153.91	1.00	153.91	29	1846	0.67	0.182	0.12	19.28	0.13	0.001
37.16	1.68	154.52	1.00	154.52	29	1847	0.67	0.182	0.12	19.28	0.13	0.001
37.21	1.68	154.81	1.00	154.81	29	1842	0.67	0.184	0.12	19.28	0.13	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
37.30	1.68	154.72	1.00	154.72	29	1834	0.67	0.186	0.12	19.28	0.14	0.001
37.35	1.67	154.64	1.00	154.64	29	1817	0.67	0.192	0.12	19.28	0.14	0.001
37.48	1.67	154.33	1.00	154.33	28	1803	0.67	0.197	0.13	19.28	0.14	0.002
37.50	1.66	154.21	1.00	154.21	28	1790	0.67	0.201	0.13	19.28	0.15	0.000
37.53	1.66	153.82	1.00	153.82	28	1783	0.67	0.203	0.13	19.28	0.15	0.001
37.62	1.66	152.89	1.00	152.89	28	1777	0.67	0.206	0.14	19.28	0.15	0.002
37.67	1.66	151.46	1.00	151.46	28	1766	0.67	0.210	0.14	19.28	0.16	0.001
37.77	1.67	149.95	1.00	149.95	28	1759	0.67	0.213	0.14	19.28	0.16	0.002
37.80	1.67	147.29	1.00	147.29	27	1749	0.67	0.217	0.15	19.28	0.17	0.001
37.91	1.68	144.58	1.00	144.58	27	1746	0.67	0.219	0.15	19.28	0.17	0.002
37.95	1.70	141.22	1.00	141.22	26	1742	0.67	0.221	0.16	19.28	0.18	0.001
38.04	1.71	138.70	1.00	138.90	26	1738	0.67	0.223	0.16	19.28	0.18	0.002
38.09	1.72	136.36	1.02	139.06	26	1732	0.67	0.225	0.16	19.28	0.18	0.001
38.13	1.72	135.49	1.02	138.19	26	1722	0.67	0.229	0.17	19.28	0.19	0.001
38.23	1.69	137.62	1.00	137.62	26	1694	0.67	0.242	0.18	19.28	0.20	0.002
38.32	1.67	140.99	1.00	140.99	26	1668	0.67	0.254	0.19	19.28	0.21	0.002
38.36	1.64	144.71	1.00	144.71	26	1650	0.67	0.264	0.19	19.28	0.21	0.001
38.41	1.63	146.96	1.00	146.96	27	1658	0.67	0.260	0.18	19.28	0.20	0.001
38.46	1.63	148.96	1.00	148.96	27	1671	0.67	0.254	0.18	19.28	0.20	0.001
38.55	1.62	150.26	1.00	150.26	27	1681	0.67	0.250	0.17	19.28	0.19	0.002
38.59	1.63	148.25	1.00	148.25	27	1678	0.67	0.251	0.17	19.28	0.20	0.001
38.68	1.64	146.41	1.00	146.41	27	1671	0.67	0.255	0.18	19.28	0.20	0.002
38.74	1.64	145.05	1.00	145.05	27	1661	0.67	0.260	0.19	19.28	0.21	0.001
38.78	1.63	147.00	1.00	147.00	27	1660	0.67	0.261	0.18	19.28	0.21	0.001
38.87	1.62	148.92	1.00	148.92	27	1662	0.67	0.261	0.18	19.28	0.20	0.002
38.92	1.61	150.98	1.00	150.98	27	1674	0.66	0.256	0.18	19.28	0.20	0.001
38.98	1.62	152.60	1.00	152.60	28	1701	0.66	0.243	0.16	19.28	0.18	0.001
39.06	1.63	154.87	1.00	154.87	28	1751	0.66	0.223	0.15	19.28	0.17	0.002
39.16	1.64	157.14	1.00	157.14	29	1799	0.66	0.207	0.13	19.28	0.15	0.002
39.20	1.64	159.32	1.00	159.32	29	1838	0.66	0.194	0.12	19.28	0.14	0.001
39.24	1.65	160.89	1.00	160.89	30	1877	0.66	0.184	0.12	19.28	0.13	0.001
39.34	1.67	160.01	1.00	160.01	30	1918	0.66	0.174	0.11	19.28	0.12	0.001
39.38	1.68	158.32	1.00	158.32	29	1952	0.66	0.166	0.10	19.28	0.12	0.001
39.47	1.70	154.70	1.00	154.70	29	1967	0.66	0.163	0.10	19.28	0.12	0.001
39.56	1.71	152.70	1.02	155.35	29	1976	0.66	0.161	0.10	19.28	0.11	0.001
39.57	1.71	151.83	1.01	153.99	29	1960	0.66	0.165	0.11	19.28	0.12	0.000
39.65	1.71	150.91	1.01	152.05	28	1942	0.66	0.169	0.11	19.28	0.12	0.001
39.70	1.71	150.69	1.00	150.69	28	1927	0.66	0.173	0.11	19.28	0.13	0.001
39.77	1.70	152.05	1.00	152.05	28	1930	0.66	0.172	0.11	19.28	0.13	0.001
39.84	1.68	158.33	1.00	158.33	29	1941	0.66	0.170	0.11	19.28	0.12	0.001
39.94	1.65	165.45	1.00	165.45	30	1949	0.66	0.168	0.10	19.28	0.11	0.001
39.98	1.62	173.66	1.00	173.66	32	1974	0.66	0.163	0.09	19.28	0.11	0.001
40.08	1.61	179.77	1.00	179.77	33	2005	0.66	0.157	0.09	19.28	0.10	0.001
40.12	1.59	185.91	1.00	185.91	33	2042	0.66	0.150	0.08	19.28	0.09	0.001
40.20	1.59	189.08	1.00	189.08	34	2070	0.66	0.145	0.08	19.28	0.09	0.001
40.26	1.59	190.36	1.00	190.36	34	2090	0.66	0.142	0.07	19.28	0.08	0.001
40.30	1.60	189.65	1.00	189.65	34	2104	0.66	0.140	0.07	19.28	0.08	0.000
40.39	1.61	188.25	1.00	188.25	34	2114	0.66	0.138	0.07	19.28	0.08	0.001

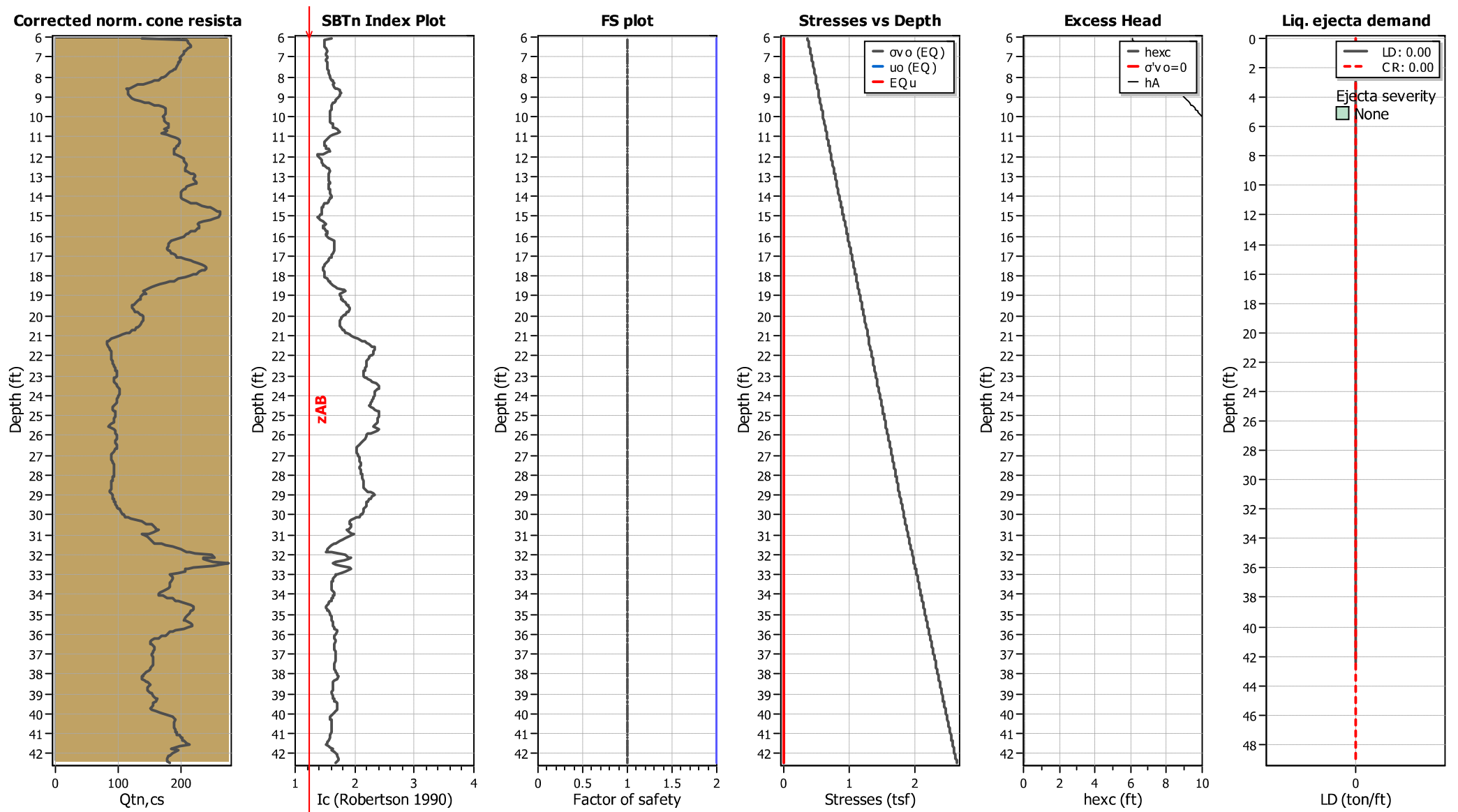
:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
40.44	1.62	187.06	1.00	187.06	34	2121	0.66	0.137	0.07	19.28	0.08	0.001
40.49	1.62	187.26	1.00	187.26	34	2124	0.66	0.137	0.07	19.28	0.08	0.000
40.58	1.61	187.95	1.00	187.95	34	2125	0.66	0.137	0.07	19.28	0.08	0.001
40.63	1.61	188.64	1.00	188.64	34	2127	0.66	0.137	0.07	19.28	0.08	0.001
40.69	1.61	188.92	1.00	188.92	34	2128	0.66	0.137	0.07	19.28	0.08	0.001
40.77	1.61	189.47	1.00	189.47	34	2133	0.66	0.136	0.07	19.28	0.08	0.001
40.84	1.60	190.13	1.00	190.13	34	2136	0.66	0.136	0.07	19.28	0.08	0.001
40.90	1.60	190.90	1.00	190.90	34	2139	0.66	0.136	0.07	19.28	0.08	0.001
40.95	1.60	191.87	1.00	191.87	35	2142	0.66	0.135	0.07	19.28	0.08	0.000
41.04	1.60	192.99	1.00	192.99	35	2150	0.66	0.134	0.07	19.28	0.08	0.001
41.08	1.59	196.61	1.00	196.61	35	2172	0.66	0.131	0.07	19.28	0.07	0.000
41.20	1.58	199.56	1.00	199.56	36	2194	0.66	0.129	0.06	19.28	0.07	0.001
41.23	1.58	202.11	1.00	202.11	36	2199	0.66	0.128	0.06	19.28	0.07	0.000
41.32	1.57	202.14	1.00	202.14	36	2183	0.66	0.130	0.06	19.28	0.07	0.001
41.36	1.56	202.81	1.00	202.81	36	2158	0.66	0.134	0.07	19.28	0.07	0.000
41.42	1.54	205.14	1.00	205.14	36	2148	0.66	0.136	0.07	19.28	0.07	0.000
41.50	1.53	208.95	1.00	208.95	37	2154	0.66	0.135	0.06	19.28	0.07	0.001
41.55	1.52	212.60	1.00	212.60	37	2171	0.66	0.133	0.06	19.28	0.07	0.000
41.63	1.57	196.33	1.00	196.33	35	2143	0.66	0.137	0.07	19.28	0.08	0.001
41.67	1.59	191.25	1.00	191.25	34	2143	0.66	0.137	0.07	19.28	0.08	0.000
41.76	1.62	184.71	1.00	184.71	34	2153	0.65	0.136	0.07	19.28	0.08	0.001
41.81	1.62	194.62	1.00	194.62	35	2245	0.65	0.123	0.06	19.28	0.07	0.000
41.90	1.65	190.33	1.00	190.33	35	2302	0.65	0.117	0.06	19.28	0.07	0.001
41.95	1.68	185.27	1.00	185.27	34	2349	0.65	0.111	0.06	19.28	0.07	0.000
42.00	1.69	182.59	1.00	182.59	34	2359	0.65	0.110	0.06	19.28	0.07	0.000
42.10	1.70	180.80	1.00	180.80	34	2365	0.65	0.110	0.06	19.28	0.07	0.001
42.14	1.71	176.00	1.01	178.30	33	2350	0.65	0.112	0.06	19.28	0.07	0.000
42.23	1.72	171.35	1.04	177.61	33	2331	0.65	0.114	0.06	19.28	0.07	0.001
42.27	1.73	167.91	1.05	176.53	33	2312	0.65	0.116	0.06	19.28	0.07	0.000
42.33	1.72	170.99	1.03	176.03	33	2317	0.65	0.116	0.06	19.28	0.07	0.001
42.41	1.70	176.32	1.00	176.32	33	2302	0.65	0.118	0.06	19.28	0.07	0.001
42.46	1.67	180.72	1.00	180.72	33	2287	0.65	0.119	0.06	19.28	0.07	0.000

Total estimated settlement: 1.09**Abbreviations**

Q _{tn} :	Equivalent clean sand normalized cone resistance
K _c :	Fines correction factor
Q _{tn,cs} :	Post-liquefaction volumetric strain
G _{max} :	Small strain shear modulus
CSR:	Soil cyclic stress ratio
γ:	Cyclic shear strain
e _{vol(15)} :	Volumetric strain after 15 cycles
N _c :	Equivalent number of cycles
e _v :	Volumetric strain
Settle.:	Calculated settlement

Ejecta Severity Estimation



LIQUEFACTION ANALYSIS REPORT

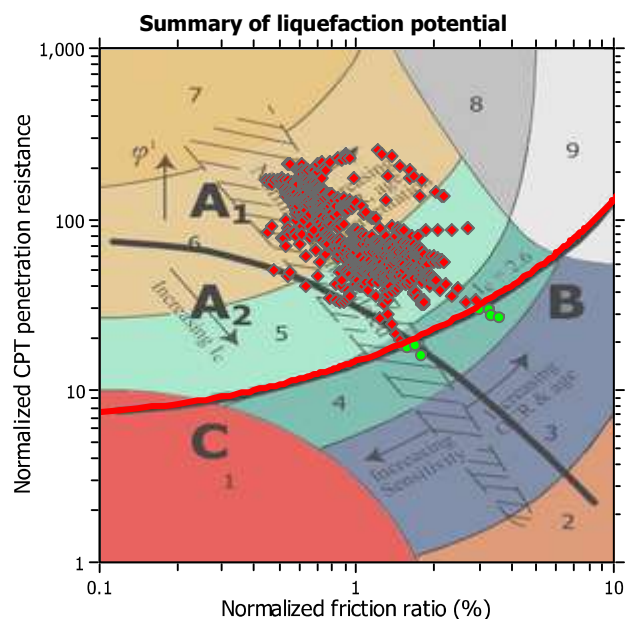
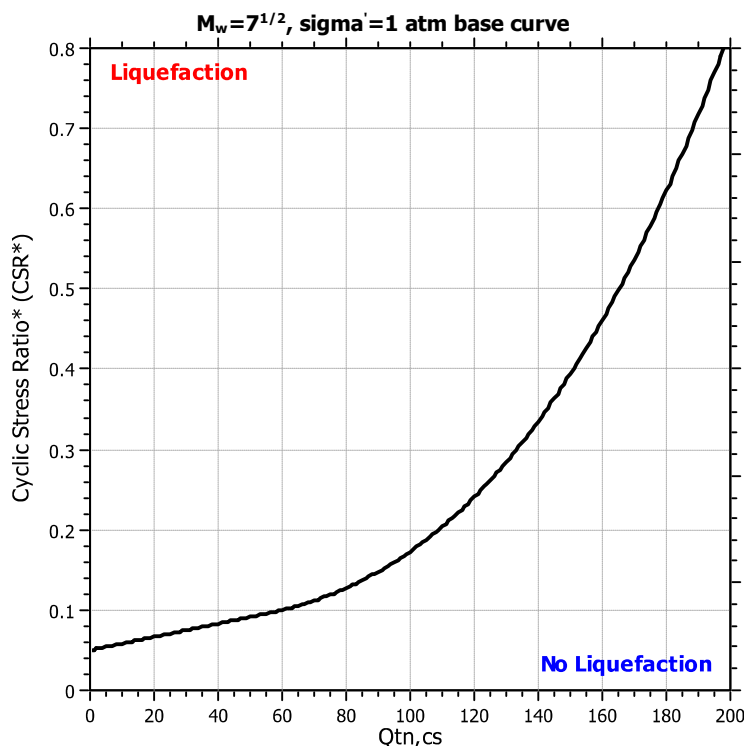
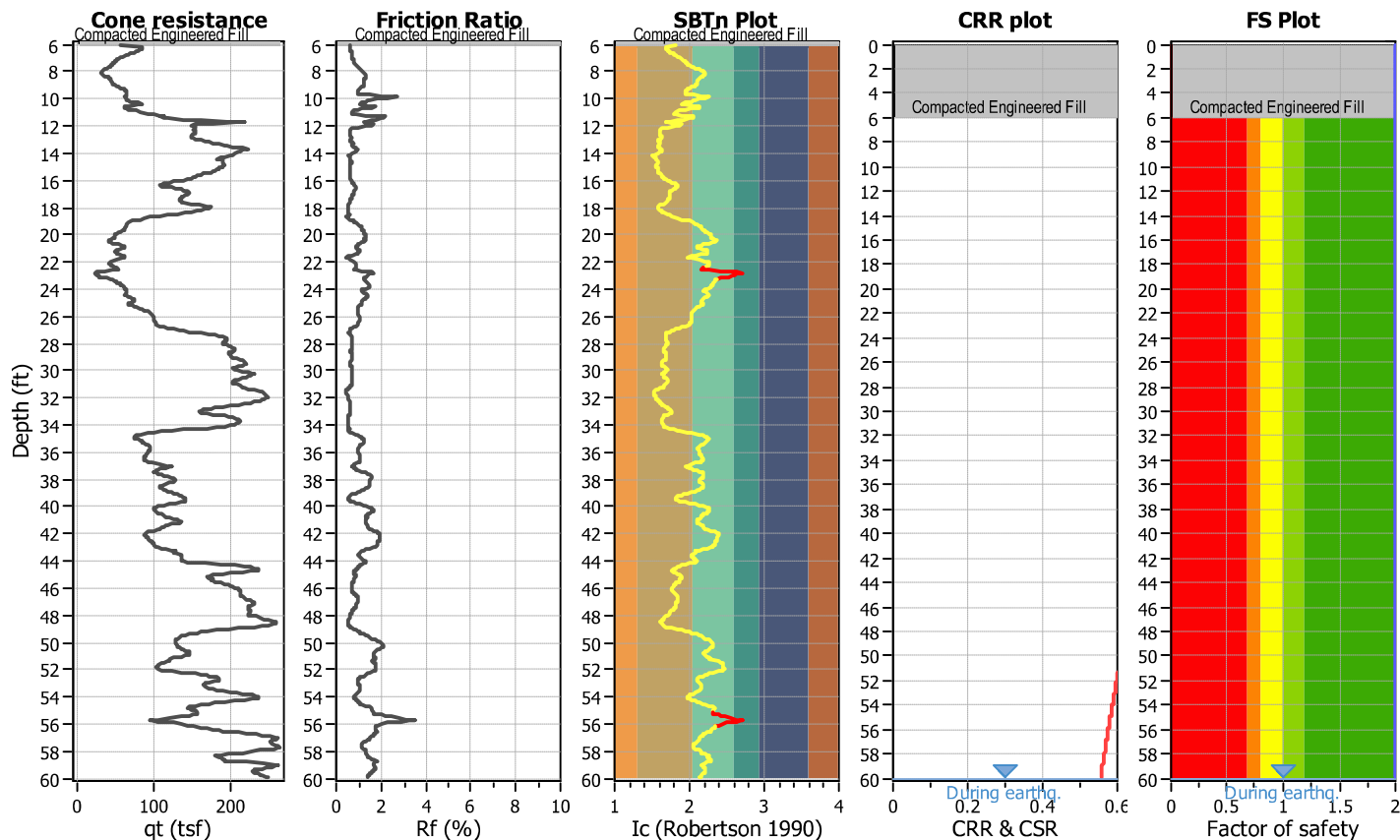
Project title : 4212-CR

Location : Northeast of the 210 Freeway and West San

CPT file : CPT-3

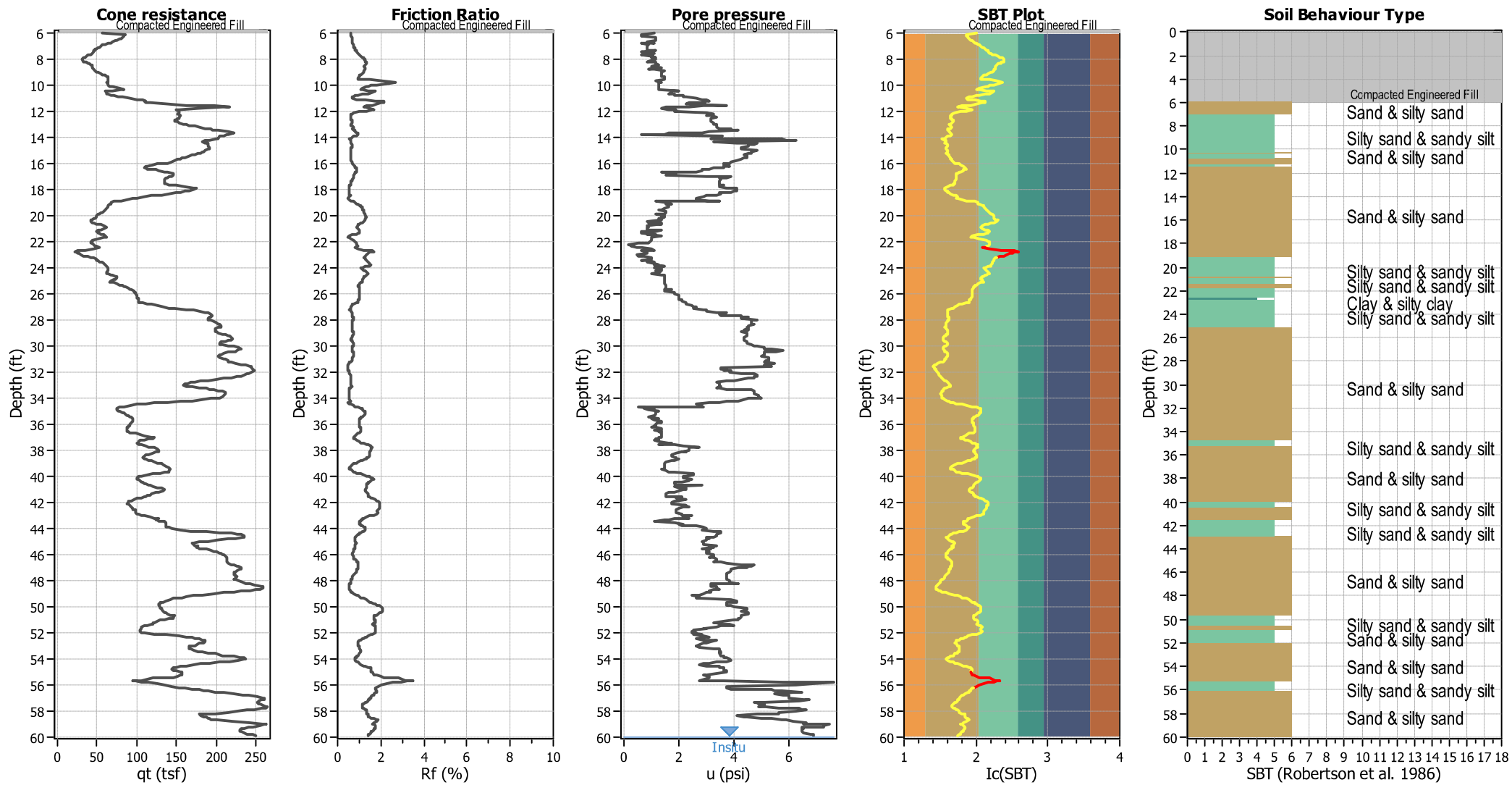
Input parameters and analysis data

Analysis method:	NCEER (1998)	G.W.T. (in-situ):	60.00 ft	Use fill:	No	Clay like behavior	
Fines correction method:	NCEER (1998)	G.W.T. (earthq.):	60.00 ft	Fill height:	N/A	applied:	Sands only
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth applied:	No
Earthquake magnitude M_w :	7.91	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	Limit depth:	N/A
Peak ground acceleration:	0.88	Unit weight calculation:	Based on SBT	K_g applied:	Yes	MSF method:	Method based



Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading
 Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground geometry
 Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening
 Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

CPT basic interpretation plots

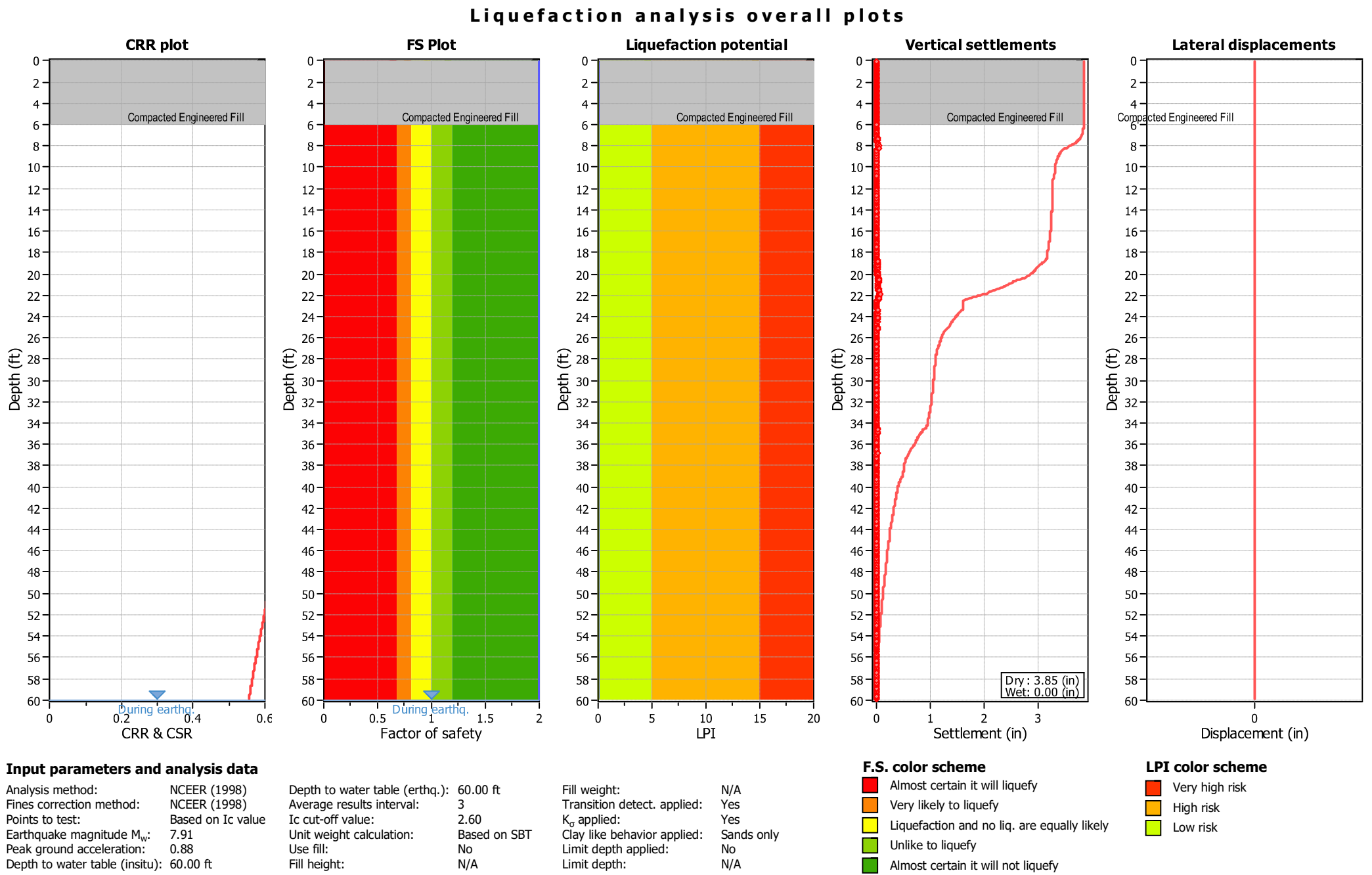


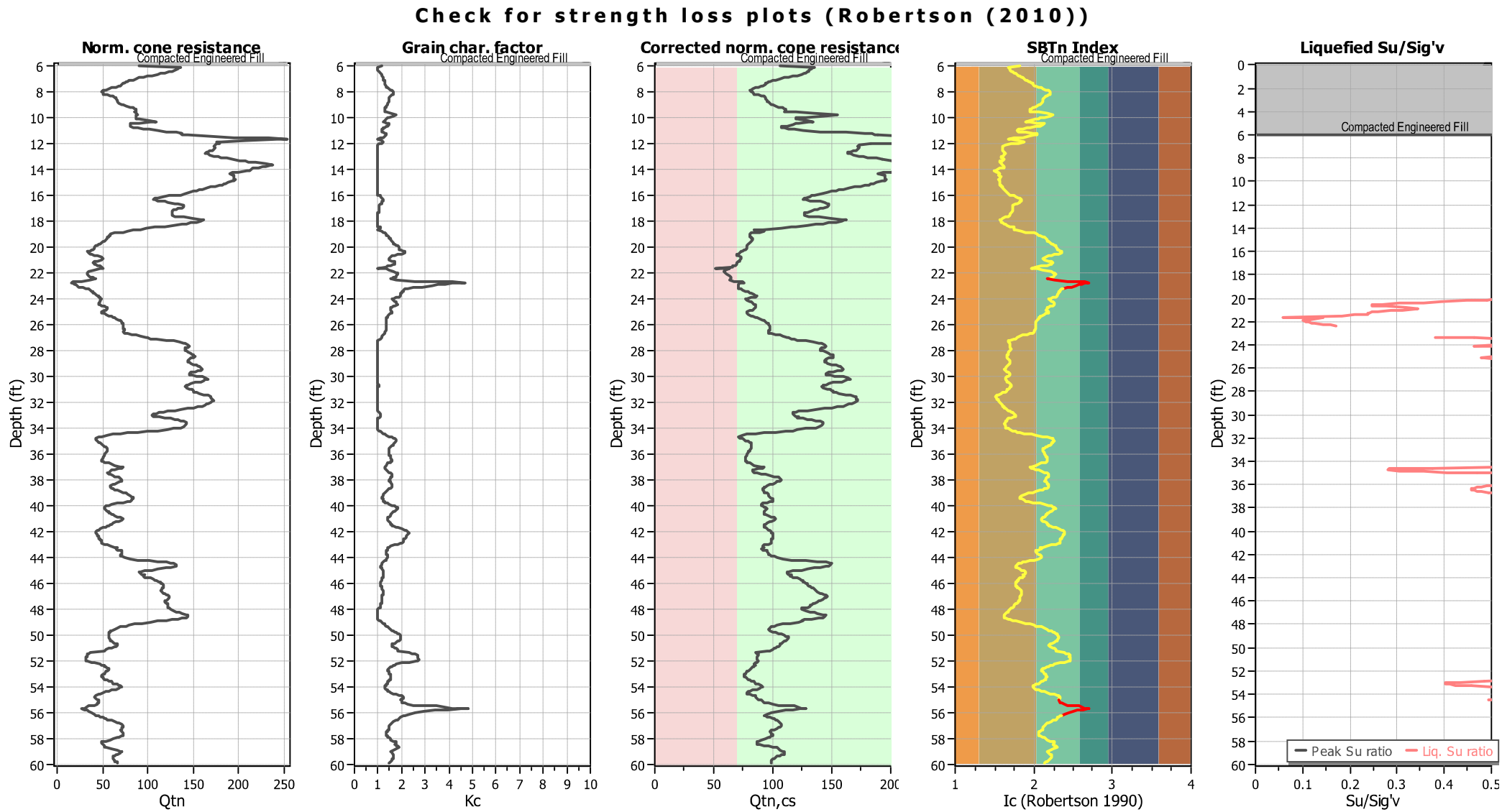
Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _o applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

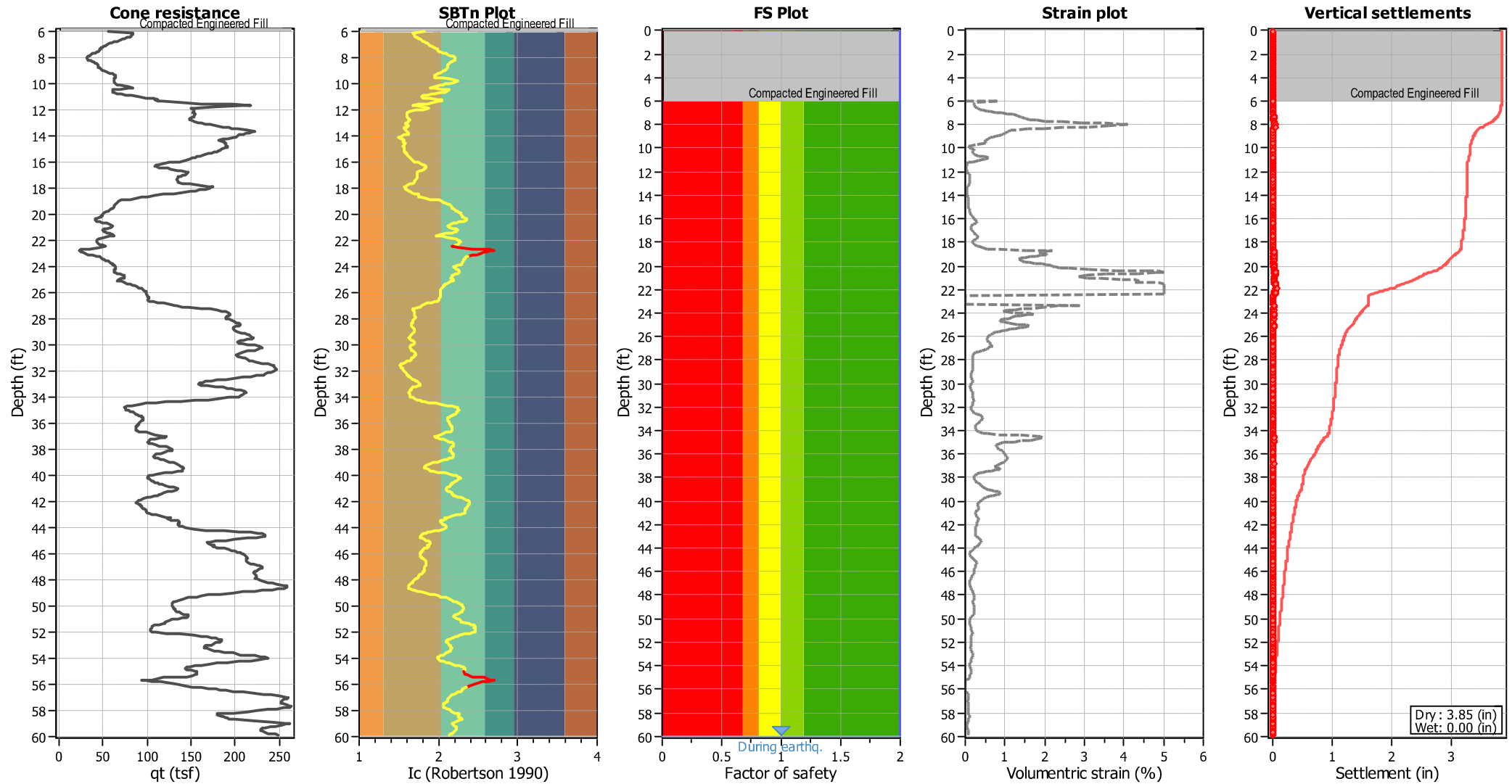




Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _g applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

Estimation of post-earthquake settlements



Abbreviations

q_t : Total cone resistance (cone resistance q_c corrected for pore water effects)
 I_c : Soil Behaviour Type Index
 FS: Calculated Factor of Safety against liquefaction
 Volumetric strain: Post-liquefaction volumetric strain

:: Post-earthquake settlement of dry sands ::												
Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
0.07	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.18	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.23	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.28	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.33	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.41	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.46	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.56	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.64	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.69	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.74	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.83	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.88	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.92	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.99	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.06	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.15	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.20	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.29	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.34	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.39	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.52	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.54	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.61	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.71	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.80	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.84	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.99	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.07	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.12	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.20	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.26	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.32	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.37	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.46	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.51	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.59	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.73	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.83	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.92	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.96	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.06	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.10	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.15	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
3.24	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.37	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.46	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.52	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.56	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.62	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.70	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.84	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.88	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.94	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.02	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.07	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.16	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.21	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.35	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.43	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.48	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.53	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.62	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.67	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.76	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.81	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.88	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.95	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.99	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.07	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.13	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.22	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.26	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.36	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.40	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.45	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.54	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.59	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.65	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.71	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.86	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.91	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.99	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
6.05	1.82	90.48	1.17	105.78	21	505	0.65	0.736	0.71	19.28	0.80	0.005
6.17	1.67	135.68	1.00	135.68	25	633	0.65	0.245	0.19	19.28	0.21	0.003
6.19	1.68	134.58	1.00	134.58	25	635	0.65	0.242	0.19	19.28	0.21	0.000
6.28	1.69	133.46	1.00	133.46	25	636	0.65	0.250	0.19	19.28	0.22	0.002
6.33	1.69	132.35	1.00	132.35	25	636	0.65	0.254	0.20	19.28	0.22	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
6.37	1.70	130.46	1.00	130.46	24	634	0.65	0.261	0.21	19.28	0.23	0.001
6.46	1.72	127.33	1.02	129.89	24	631	0.65	0.276	0.22	19.28	0.24	0.003
6.51	1.73	123.10	1.05	129.77	24	623	0.65	0.299	0.23	19.28	0.26	0.001
6.60	1.75	118.80	1.08	128.59	24	615	0.65	0.329	0.26	19.28	0.29	0.003
6.65	1.76	114.66	1.11	127.00	24	605	0.65	0.361	0.29	19.28	0.32	0.002
6.70	1.78	110.14	1.13	124.32	24	592	0.65	0.409	0.33	19.28	0.37	0.002
6.79	1.80	105.07	1.15	120.84	23	579	0.65	0.477	0.40	19.28	0.44	0.005
6.83	1.82	99.97	1.17	117.06	23	563	0.65	0.559	0.48	19.28	0.54	0.003
6.89	1.84	95.40	1.19	113.65	22	552	0.65	0.640	0.56	19.28	0.63	0.004
6.97	1.86	90.59	1.21	109.69	22	540	0.65	0.745	0.68	19.28	0.76	0.007
7.04	1.88	86.57	1.23	106.10	21	529	0.65	0.857	0.80	19.28	0.90	0.008
7.10	1.90	83.12	1.24	102.93	21	520	0.65	0.971	0.94	19.28	1.05	0.008
7.16	1.92	79.94	1.25	100.05	20	512	0.65	1.083	1.07	19.28	1.20	0.008
7.25	1.94	77.65	1.26	97.94	20	509	0.65	1.173	1.18	19.28	1.32	0.014
7.29	1.95	75.82	1.27	96.42	20	506	0.65	1.231	1.25	19.28	1.40	0.007
7.38	1.96	74.67	1.28	95.51	20	507	0.65	1.258	1.29	19.28	1.44	0.016
7.43	1.98	73.15	1.29	94.41	19	507	0.65	1.288	1.33	19.28	1.49	0.009
7.52	1.99	71.71	1.30	93.32	19	507	0.65	1.327	1.38	19.28	1.54	0.016
7.57	2.01	69.96	1.32	92.02	19	505	0.65	1.386	1.45	19.28	1.63	0.010
7.62	2.03	67.59	1.33	90.22	19	500	0.65	1.498	1.59	19.28	1.78	0.011
7.69	2.06	64.69	1.36	88.09	19	494	0.65	1.667	1.80	19.28	2.02	0.017
7.75	2.09	60.95	1.40	85.46	18	484	0.65	1.966	2.17	19.28	2.43	0.020
7.85	2.12	57.48	1.45	83.21	18	474	0.65	2.336	2.63	19.28	2.94	0.032
7.89	2.16	53.81	1.52	81.60	18	463	0.65	2.790	3.15	19.28	3.53	0.019
7.98	2.19	51.15	1.58	81.05	18	457	0.64	3.117	3.49	19.28	3.91	0.042
8.03	2.21	49.15	1.65	81.21	18	455	0.64	3.305	3.65	19.28	4.08	0.024
8.07	2.21	50.11	1.65	82.51	19	463	0.64	2.983	3.23	19.28	3.62	0.020
8.21	2.21	51.40	1.63	83.75	19	477	0.64	2.575	2.75	19.28	3.08	0.052
8.26	2.20	53.50	1.61	85.91	19	493	0.64	2.092	2.18	19.28	2.44	0.014
8.30	2.19	55.59	1.58	87.90	20	509	0.64	1.741	1.77	19.28	1.98	0.010
8.35	2.17	58.23	1.55	90.35	20	527	0.64	1.410	1.40	19.28	1.56	0.009
8.41	2.15	61.09	1.51	92.43	20	545	0.64	1.178	1.15	19.28	1.28	0.009
8.49	2.14	63.12	1.48	93.56	21	557	0.64	1.064	1.03	19.28	1.15	0.011
8.53	2.13	64.57	1.46	94.25	21	564	0.64	1.003	0.97	19.28	1.08	0.006
8.61	2.12	65.30	1.44	94.29	21	568	0.64	0.989	0.96	19.28	1.07	0.010
8.68	2.11	65.89	1.44	94.93	21	575	0.64	0.945	0.91	19.28	1.02	0.008
8.77	2.11	66.17	1.44	95.25	21	580	0.64	0.919	0.88	19.28	0.99	0.011
8.81	2.12	66.17	1.45	95.76	21	585	0.64	0.890	0.84	19.28	0.95	0.005
8.87	2.11	67.25	1.43	96.42	21	592	0.64	0.850	0.80	19.28	0.90	0.006
8.96	2.10	68.41	1.42	97.06	21	599	0.64	0.813	0.77	19.28	0.86	0.008
9.00	2.08	70.82	1.39	98.71	21	611	0.64	0.742	0.69	19.28	0.77	0.004
9.07	2.07	72.97	1.37	100.22	21	622	0.64	0.688	0.63	19.28	0.71	0.006
9.14	2.04	76.67	1.34	102.90	22	637	0.64	0.618	0.56	19.28	0.63	0.005
9.23	2.01	80.20	1.31	105.45	22	650	0.64	0.571	0.51	19.28	0.57	0.006
9.28	1.98	83.82	1.29	108.27	22	661	0.64	0.532	0.46	19.28	0.52	0.003
9.36	1.96	85.66	1.28	109.66	23	668	0.64	0.517	0.45	19.28	0.50	0.005
9.39	1.95	87.08	1.27	110.72	23	671	0.64	0.508	0.44	19.28	0.49	0.002
9.46	1.95	87.09	1.27	110.59	23	672	0.64	0.514	0.44	19.28	0.50	0.004

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
9.55	1.95	86.49	1.27	109.94	22	673	0.64	0.524	0.46	19.28	0.51	0.005
9.60	1.95	85.95	1.27	109.44	22	673	0.64	0.531	0.46	19.28	0.52	0.003
9.65	2.06	87.03	1.37	119.29	25	769	0.64	0.287	0.21	19.28	0.24	0.001
9.76	2.16	87.77	1.53	134.19	30	870	0.64	0.178	0.11	19.28	0.12	0.002
9.78	2.25	88.59	1.74	154.36	35	963	0.64	0.124	0.06	19.28	0.07	0.000
9.87	2.25	87.86	1.76	154.44	36	966	0.64	0.125	0.06	19.28	0.07	0.001
9.93	2.20	87.70	1.62	142.21	32	918	0.64	0.151	0.09	19.28	0.10	0.001
10.01	2.13	87.00	1.48	128.34	28	850	0.64	0.206	0.14	19.28	0.15	0.002
10.05	2.08	86.33	1.39	120.03	26	797	0.64	0.272	0.20	19.28	0.22	0.001
10.13	2.06	87.42	1.37	119.34	25	793	0.64	0.283	0.21	19.28	0.24	0.002
10.20	2.03	90.79	1.33	121.10	25	800	0.64	0.275	0.21	19.28	0.23	0.002
10.24	1.95	102.10	1.27	129.34	26	823	0.64	0.248	0.18	19.28	0.20	0.001
10.33	1.90	108.50	1.24	134.39	27	838	0.64	0.234	0.16	19.28	0.18	0.002
10.38	1.95	102.86	1.27	130.44	27	837	0.64	0.238	0.17	19.28	0.19	0.001
10.47	2.05	89.96	1.36	122.00	26	826	0.64	0.257	0.19	19.28	0.21	0.002
10.52	2.14	79.99	1.49	119.07	26	814	0.64	0.275	0.20	19.28	0.22	0.001
10.61	2.12	80.22	1.46	116.76	26	805	0.64	0.296	0.22	19.28	0.25	0.003
10.66	2.08	80.05	1.40	111.79	24	771	0.64	0.362	0.29	19.28	0.32	0.002
10.70	2.02	80.79	1.33	107.09	22	727	0.64	0.482	0.42	19.28	0.47	0.002
10.79	1.96	83.86	1.27	106.90	22	706	0.64	0.568	0.51	19.28	0.57	0.006
10.86	1.91	88.52	1.24	109.85	22	707	0.64	0.575	0.51	19.28	0.57	0.005
10.90	1.86	94.39	1.21	114.00	23	716	0.64	0.542	0.47	19.28	0.53	0.003
10.97	1.81	101.20	1.17	117.94	23	731	0.64	0.499	0.42	19.28	0.48	0.004
11.06	1.78	111.20	1.13	125.75	24	780	0.64	0.374	0.30	19.28	0.33	0.003
11.16	1.81	120.58	1.16	140.32	27	878	0.64	0.231	0.16	19.28	0.18	0.002
11.20	1.90	130.77	1.24	161.78	32	1056	0.64	0.120	0.07	19.28	0.07	0.000
11.26	1.98	138.13	1.29	178.25	37	1223	0.64	0.078	0.04	19.28	0.04	0.000
11.30	2.05	135.67	1.36	184.02	39	1307	0.64	0.066	0.03	19.28	0.03	0.000
11.35	2.04	138.50	1.35	186.78	40	1327	0.64	0.064	0.03	19.28	0.03	0.000
11.42	2.01	143.95	1.32	189.58	40	1335	0.64	0.064	0.03	19.28	0.03	0.000
11.49	1.93	167.55	1.26	210.57	43	1423	0.64	0.056	0.02	19.28	0.03	0.000
11.58	1.85	194.35	1.20	233.75	46	1518	0.64	0.049	0.02	19.28	0.02	0.000
11.63	1.75	232.24	1.08	250.78	48	1613	0.64	0.043	0.02	19.28	0.02	0.000
11.68	1.69	253.31	1.00	253.31	47	1644	0.64	0.042	0.02	19.28	0.02	0.000
11.75	1.71	242.07	1.00	242.15	45	1614	0.64	0.044	0.02	19.28	0.02	0.000
11.82	1.80	207.05	1.15	239.10	46	1545	0.64	0.049	0.02	19.28	0.02	0.000
11.91	1.88	180.77	1.23	221.50	44	1487	0.64	0.054	0.02	19.28	0.02	0.000
11.95	1.88	175.84	1.22	215.09	43	1445	0.64	0.057	0.02	19.28	0.03	0.000
12.04	1.81	178.67	1.17	208.20	40	1363	0.64	0.067	0.03	19.28	0.03	0.000
12.09	1.72	176.46	1.03	182.33	34	1218	0.64	0.091	0.05	19.28	0.05	0.000
12.14	1.64	173.64	1.00	173.64	32	1094	0.64	0.126	0.07	19.28	0.08	0.000
12.23	1.61	171.79	1.00	171.79	31	1041	0.64	0.151	0.09	19.28	0.10	0.001
12.28	1.60	172.98	1.00	172.98	31	1044	0.64	0.151	0.09	19.28	0.10	0.001
12.36	1.60	173.62	1.00	173.62	31	1047	0.64	0.152	0.09	19.28	0.10	0.001
12.41	1.60	173.55	1.00	173.55	31	1043	0.64	0.154	0.09	19.28	0.10	0.001
12.48	1.60	172.13	1.00	172.13	31	1041	0.64	0.158	0.09	19.28	0.10	0.001
12.55	1.60	170.53	1.00	170.53	31	1036	0.64	0.162	0.10	19.28	0.11	0.001
12.60	1.61	168.31	1.00	168.31	30	1036	0.64	0.163	0.10	19.28	0.11	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
12.69	1.62	165.66	1.00	165.66	30	1035	0.64	0.166	0.10	19.28	0.11	0.001
12.74	1.63	163.97	1.00	163.97	30	1041	0.64	0.164	0.10	19.28	0.11	0.001
12.80	1.63	163.37	1.00	163.37	30	1046	0.64	0.163	0.10	19.28	0.11	0.001
12.87	1.63	165.47	1.00	165.47	30	1056	0.64	0.160	0.10	19.28	0.11	0.001
12.96	1.62	168.56	1.00	168.56	31	1062	0.64	0.159	0.10	19.28	0.11	0.001
13.01	1.60	172.95	1.00	172.95	31	1067	0.64	0.158	0.09	19.28	0.10	0.001
13.06	1.58	177.17	1.00	177.17	32	1076	0.64	0.154	0.09	19.28	0.10	0.001
13.15	1.59	180.87	1.00	180.87	33	1111	0.64	0.141	0.08	19.28	0.09	0.001
13.20	1.60	187.42	1.00	187.42	34	1174	0.64	0.119	0.06	19.28	0.07	0.000
13.29	1.61	192.68	1.00	192.68	35	1230	0.64	0.105	0.05	19.28	0.06	0.001
13.33	1.60	200.94	1.00	200.94	36	1255	0.64	0.099	0.05	19.28	0.05	0.000
13.43	1.57	207.49	1.00	207.49	37	1261	0.64	0.099	0.05	19.28	0.05	0.001
13.46	1.57	210.62	1.00	210.62	38	1278	0.64	0.096	0.05	19.28	0.05	0.000
13.52	1.59	217.82	1.00	217.82	39	1355	0.64	0.083	0.04	19.28	0.04	0.000
13.61	1.59	229.78	1.00	229.78	41	1443	0.64	0.071	0.03	19.28	0.03	0.000
13.66	1.60	237.62	1.00	237.62	43	1519	0.64	0.063	0.03	19.28	0.03	0.000
13.75	1.61	236.13	1.00	236.13	43	1527	0.64	0.063	0.03	19.28	0.03	0.000
13.79	1.63	229.34	1.00	229.34	42	1518	0.64	0.065	0.03	19.28	0.03	0.000
13.87	1.62	226.21	1.00	226.21	41	1494	0.64	0.068	0.03	19.28	0.03	0.000
13.93	1.62	221.49	1.00	221.49	40	1450	0.64	0.073	0.03	19.28	0.04	0.000
13.98	1.57	215.55	1.00	215.55	39	1340	0.64	0.090	0.04	19.28	0.05	0.000
14.11	1.52	211.98	1.00	211.98	37	1240	0.64	0.115	0.05	19.28	0.06	0.001
14.12	1.49	210.57	1.00	210.57	37	1182	0.64	0.133	0.06	19.28	0.07	0.000
14.20	1.50	208.96	1.00	208.96	36	1193	0.64	0.130	0.06	19.28	0.07	0.001
14.26	1.53	200.26	1.00	200.26	35	1191	0.64	0.132	0.07	19.28	0.07	0.001
14.32	1.55	193.77	1.00	193.77	34	1183	0.64	0.136	0.07	19.28	0.08	0.001
14.38	1.56	189.55	1.00	189.55	34	1175	0.64	0.140	0.07	19.28	0.08	0.001
14.44	1.54	193.30	1.00	193.30	34	1175	0.64	0.141	0.07	19.28	0.08	0.001
14.52	1.55	195.41	1.00	195.41	35	1203	0.64	0.133	0.07	19.28	0.08	0.001
14.61	1.57	194.81	1.00	194.81	35	1230	0.64	0.125	0.06	19.28	0.07	0.001
14.65	1.58	194.63	1.00	194.63	35	1257	0.64	0.118	0.06	19.28	0.07	0.000
14.70	1.58	195.21	1.00	195.21	35	1250	0.64	0.121	0.06	19.28	0.07	0.000
14.79	1.57	195.27	1.00	195.27	35	1242	0.64	0.125	0.06	19.28	0.07	0.001
14.83	1.56	196.21	1.00	196.21	35	1234	0.64	0.128	0.07	19.28	0.07	0.000
14.89	1.56	194.86	1.00	194.86	35	1231	0.64	0.130	0.07	19.28	0.08	0.001
14.96	1.56	192.69	1.00	192.69	34	1228	0.64	0.132	0.07	19.28	0.08	0.001
15.06	1.57	188.20	1.00	188.20	34	1218	0.64	0.137	0.07	19.28	0.08	0.001
15.11	1.57	185.79	1.00	185.79	33	1204	0.63	0.143	0.08	19.28	0.09	0.000
15.16	1.57	183.47	1.00	183.47	33	1189	0.63	0.150	0.08	19.28	0.09	0.001
15.26	1.57	181.34	1.00	181.34	32	1178	0.63	0.156	0.09	19.28	0.10	0.001
15.30	1.58	178.15	1.00	178.15	32	1168	0.63	0.162	0.09	19.28	0.10	0.001
15.39	1.59	174.23	1.00	174.23	31	1159	0.63	0.168	0.10	19.28	0.11	0.001
15.43	1.60	169.67	1.00	169.67	31	1147	0.63	0.175	0.11	19.28	0.12	0.001
15.50	1.61	164.99	1.00	164.99	30	1133	0.63	0.184	0.11	19.28	0.13	0.001
15.58	1.62	160.04	1.00	160.04	29	1118	0.63	0.195	0.12	19.28	0.14	0.001
15.64	1.63	156.35	1.00	156.35	28	1106	0.63	0.205	0.13	19.28	0.15	0.001
15.69	1.64	153.27	1.00	153.27	28	1097	0.63	0.212	0.14	19.28	0.16	0.001
15.76	1.65	150.27	1.00	150.27	28	1089	0.63	0.220	0.15	19.28	0.17	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
15.85	1.66	146.34	1.00	146.34	27	1082	0.63	0.228	0.16	19.28	0.18	0.002
15.90	1.68	141.29	1.00	141.29	26	1070	0.63	0.240	0.17	19.28	0.19	0.001
15.99	1.70	135.52	1.00	135.52	25	1056	0.63	0.255	0.19	19.28	0.22	0.002
16.03	1.72	129.80	1.03	133.36	25	1040	0.63	0.272	0.21	19.28	0.23	0.001
16.09	1.74	123.48	1.08	132.87	25	1023	0.63	0.292	0.22	19.28	0.25	0.002
16.16	1.77	117.09	1.12	131.28	25	1009	0.63	0.312	0.24	19.28	0.27	0.002
16.22	1.81	110.14	1.16	128.25	25	994	0.63	0.334	0.26	19.28	0.29	0.002
16.32	1.82	108.14	1.18	127.12	25	993	0.63	0.340	0.26	19.28	0.29	0.003
16.36	1.84	105.61	1.19	125.62	25	990	0.63	0.347	0.27	19.28	0.30	0.001
16.40	1.84	107.88	1.19	128.61	25	1016	0.63	0.313	0.24	19.28	0.27	0.001
16.50	1.85	110.05	1.20	132.16	26	1054	0.63	0.274	0.20	19.28	0.22	0.002
16.54	1.82	119.26	1.18	140.46	27	1107	0.63	0.229	0.16	19.28	0.18	0.001
16.64	1.82	122.48	1.17	143.36	28	1130	0.63	0.215	0.14	19.28	0.16	0.002
16.67	1.78	129.71	1.13	146.72	28	1147	0.63	0.205	0.14	19.28	0.15	0.001
16.73	1.77	132.68	1.11	147.41	28	1154	0.63	0.202	0.13	19.28	0.15	0.001
16.82	1.74	138.82	1.06	147.57	28	1167	0.63	0.196	0.13	19.28	0.15	0.001
16.87	1.73	140.07	1.05	147.31	28	1170	0.63	0.196	0.13	19.28	0.15	0.001
16.96	1.73	139.38	1.05	145.83	27	1164	0.63	0.202	0.14	19.28	0.15	0.002
17.01	1.73	137.44	1.04	142.91	27	1145	0.63	0.215	0.15	19.28	0.17	0.001
17.09	1.73	133.59	1.05	140.10	26	1122	0.63	0.234	0.17	19.28	0.19	0.002
17.15	1.73	130.89	1.06	138.22	26	1106	0.63	0.248	0.18	19.28	0.20	0.001
17.19	1.73	128.76	1.05	135.84	26	1089	0.63	0.265	0.20	19.28	0.22	0.001
17.29	1.73	127.34	1.04	132.99	25	1073	0.63	0.284	0.22	19.28	0.24	0.003
17.33	1.72	126.52	1.02	128.97	24	1051	0.63	0.310	0.25	19.28	0.28	0.002
17.43	1.71	126.41	1.00	126.66	24	1043	0.63	0.324	0.26	19.28	0.30	0.003
17.47	1.70	126.28	1.00	126.28	24	1035	0.63	0.335	0.27	19.28	0.31	0.002
17.55	1.70	126.24	1.00	126.24	24	1035	0.63	0.339	0.28	19.28	0.31	0.003
17.61	1.70	127.22	1.00	127.22	24	1039	0.63	0.336	0.27	19.28	0.31	0.002
17.66	1.68	130.23	1.00	130.23	24	1049	0.63	0.325	0.26	19.28	0.29	0.002
17.72	1.65	138.59	1.00	138.59	25	1070	0.63	0.303	0.23	19.28	0.25	0.002
17.80	1.61	149.08	1.00	149.08	27	1099	0.63	0.276	0.19	19.28	0.22	0.002
17.89	1.58	158.33	1.00	158.33	28	1125	0.63	0.255	0.17	19.28	0.19	0.002
17.93	1.57	162.00	1.00	162.00	29	1140	0.63	0.244	0.16	19.28	0.18	0.001
17.98	1.57	161.27	1.00	161.27	29	1139	0.63	0.246	0.16	19.28	0.18	0.001
18.07	1.58	158.08	1.00	158.08	28	1129	0.63	0.257	0.17	19.28	0.19	0.002
18.12	1.59	153.27	1.00	153.27	28	1113	0.63	0.273	0.19	19.28	0.21	0.001
18.19	1.61	147.78	1.00	147.78	27	1096	0.64	0.292	0.21	19.28	0.23	0.002
18.25	1.62	142.67	1.00	142.67	26	1078	0.64	0.313	0.23	19.28	0.26	0.002
18.31	1.64	137.01	1.00	137.01	25	1058	0.64	0.341	0.26	19.28	0.29	0.002
18.39	1.66	128.94	1.00	128.94	24	1033	0.64	0.380	0.31	19.28	0.35	0.003
18.45	1.70	119.14	1.00	119.14	22	1006	0.64	0.429	0.38	19.28	0.42	0.003
18.53	1.74	107.98	1.07	115.59	22	961	0.64	0.530	0.48	19.28	0.53	0.005
18.58	1.75	98.45	1.09	107.21	20	890	0.64	0.767	0.75	19.28	0.84	0.005
18.67	1.75	91.15	1.00	91.15	17	825	0.64	1.147	1.36	19.28	1.53	0.016
18.70	1.77	84.53	1.00	84.53	16	786	0.64	1.509	1.95	19.28	2.18	0.009
18.80	1.83	78.25	1.19	92.87	18	790	0.64	1.484	1.67	19.28	1.86	0.023
18.90	1.90	72.18	1.23	89.13	18	792	0.64	1.478	1.69	19.28	1.90	0.022
18.92	1.96	66.00	1.28	84.42	17	790	0.64	1.510	1.79	19.28	2.01	0.004

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
18.96	2.00	62.61	1.31	82.04	17	791	0.64	1.511	1.82	19.28	2.04	0.012
19.03	2.05	59.16	1.35	80.00	17	792	0.64	1.509	1.84	19.28	2.06	0.017
19.11	2.06	58.72	1.37	80.40	17	804	0.64	1.408	1.69	19.28	1.90	0.017
19.19	2.09	57.28	1.40	80.22	17	811	0.64	1.351	1.61	19.28	1.80	0.017
19.23	2.11	56.24	1.44	80.81	18	823	0.64	1.252	1.46	19.28	1.64	0.008
19.31	2.13	55.56	1.47	81.55	18	836	0.64	1.170	1.34	19.28	1.50	0.015
19.37	2.14	55.15	1.49	82.27	18	845	0.64	1.108	1.25	19.28	1.40	0.010
19.42	2.15	54.82	1.51	82.55	18	850	0.64	1.085	1.21	19.28	1.36	0.008
19.50	2.16	53.85	1.53	82.35	18	850	0.64	1.097	1.22	19.28	1.37	0.013
19.60	2.18	52.51	1.56	81.86	18	846	0.64	1.135	1.26	19.28	1.42	0.016
19.65	2.19	51.01	1.59	81.26	18	839	0.64	1.194	1.33	19.28	1.49	0.008
19.69	2.21	49.44	1.63	80.46	18	830	0.64	1.279	1.43	19.28	1.60	0.009
19.78	2.22	47.74	1.67	79.59	18	820	0.64	1.385	1.56	19.28	1.75	0.019
19.83	2.24	45.71	1.72	78.80	18	807	0.64	1.523	1.72	19.28	1.92	0.011
19.92	2.26	43.99	1.78	78.39	18	798	0.64	1.638	1.84	19.28	2.06	0.023
19.97	2.28	42.55	1.84	78.38	18	791	0.64	1.728	1.93	19.28	2.16	0.012
20.02	2.29	41.93	1.87	78.44	18	789	0.64	1.764	1.96	19.28	2.19	0.012
20.10	2.29	41.17	1.90	78.35	18	786	0.64	1.823	2.02	19.28	2.26	0.024
20.15	2.30	40.67	1.92	77.90	18	782	0.64	1.902	2.11	19.28	2.37	0.015
20.22	2.30	40.15	1.91	76.80	18	773	0.64	2.053	2.32	19.28	2.60	0.020
20.28	2.31	38.44	1.96	75.29	18	753	0.65	2.414	2.78	19.28	3.11	0.024
20.38	2.33	36.13	2.05	74.08	18	732	0.65	2.924	3.39	19.28	3.80	0.045
20.43	2.36	33.95	2.16	73.37	18	711	0.65	3.507	4.06	19.28	4.55	0.023
20.48	2.35	34.19	2.11	72.18	17	707	0.65	3.664	4.35	19.28	4.87	0.032
20.57	2.32	35.64	1.98	70.68	17	711	0.65	3.606	4.46	19.28	5.00	0.053
20.61	2.27	38.24	1.82	69.68	16	721	0.65	3.312	4.27	19.28	4.78	0.025
20.69	2.23	41.04	1.70	69.63	16	735	0.65	2.951	3.88	19.28	4.35	0.041
20.74	2.18	45.04	1.58	71.07	16	761	0.65	2.412	3.17	19.28	3.55	0.020
20.84	2.15	48.42	1.50	72.65	16	782	0.65	2.078	2.71	19.28	3.03	0.039
20.89	2.12	50.93	1.45	73.67	16	791	0.65	1.952	2.54	19.28	2.84	0.015
20.98	2.12	51.02	1.44	73.65	16	793	0.65	1.951	2.54	19.28	2.85	0.032
21.03	2.14	48.68	1.49	72.46	16	784	0.65	2.098	2.75	19.28	3.08	0.016
21.10	2.19	44.64	1.60	71.38	16	771	0.65	2.327	3.03	19.28	3.39	0.030
21.17	2.23	41.44	1.70	70.25	16	753	0.65	2.713	3.53	19.28	3.96	0.034
21.21	2.24	40.35	1.73	69.62	16	745	0.65	2.932	3.84	19.28	4.30	0.021
21.27	2.23	40.81	1.70	69.27	16	745	0.65	2.947	3.90	19.28	4.37	0.033
21.34	2.23	41.05	1.68	69.12	16	746	0.65	2.945	3.92	19.28	4.39	0.035
21.39	2.21	42.09	1.64	69.15	16	751	0.65	2.845	3.81	19.28	4.27	0.029
21.46	2.17	44.13	1.53	67.73	15	742	0.65	3.084	4.34	19.28	4.86	0.038
21.54	2.09	46.98	1.40	65.95	14	716	0.65	3.909	5.89	19.28	5.00	0.044
21.59	2.02	49.53	1.32	65.45	14	686	0.65	5.216	8.21	19.28	5.00	0.032
21.66	1.98	51.15	1.00	51.15	11	673	0.65	5.985	12.89	19.28	5.00	0.040
21.72	2.03	46.95	1.34	62.84	13	669	0.65	6.350	10.40	19.28	5.00	0.038
21.86	2.12	41.22	1.45	59.71	13	659	0.65	7.116	11.90	19.28	5.00	0.082
21.92	2.22	35.03	1.68	58.71	13	645	0.65	8.379	13.58	19.28	5.00	0.040
21.99	2.25	33.70	1.76	59.30	14	646	0.65	8.385	13.24	19.28	5.00	0.040
22.06	2.27	33.20	1.81	59.97	14	650	0.65	8.089	12.51	19.28	5.00	0.041
22.14	2.27	33.28	1.82	60.69	14	658	0.65	7.526	11.44	19.28	5.00	0.047

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
22.19	2.27	33.81	1.82	61.38	14	667	0.65	6.869	10.31	19.28	5.00	0.033
22.24	2.26	35.07	1.79	62.86	15	687	0.65	5.669	8.30	19.28	5.00	0.032
22.33	2.25	36.67	1.75	64.16	15	707	0.65	4.722	6.79	19.28	5.00	0.050
22.38	2.22	38.88	1.68	65.17	15	726	0.65	3.998	5.72	19.28	5.00	0.029
22.46	2.19	40.83	1.60	65.14	15	732	0.65	3.803	5.53	19.28	5.00	0.050
22.52	2.17	42.00	1.54	64.87	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.61	2.26	35.81	1.78	63.67	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.66	2.43	26.38	2.55	67.23	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.71	2.64	18.07	4.15	74.97	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.79	2.70	16.13	4.70	75.82	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.84	2.64	17.52	4.13	72.33	0	0	0.65	0.000	0.00	0.00	0.00	0.000
22.94	2.61	18.43	3.85	70.92	0	0	0.66	0.000	0.00	0.00	0.00	0.000
22.98	2.58	19.82	3.55	70.39	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.07	2.54	21.57	3.26	70.37	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.12	2.48	24.41	2.87	70.16	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.17	2.43	28.08	2.51	70.59	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.26	2.37	31.84	2.24	71.28	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.30	2.35	34.65	2.12	73.38	0	0	0.66	0.000	0.00	0.00	0.00	0.000
23.38	2.33	36.52	2.05	74.90	18	811	0.66	2.266	2.59	19.28	2.90	0.027
23.44	2.33	37.68	2.05	77.21	18	838	0.66	1.891	2.09	19.28	2.34	0.017
23.53	2.32	38.99	2.02	78.88	19	862	0.66	1.628	1.76	19.28	1.97	0.021
23.58	2.32	40.14	2.01	80.63	19	884	0.66	1.420	1.50	19.28	1.67	0.010
23.63	2.31	41.82	1.96	82.17	19	909	0.66	1.232	1.28	19.28	1.43	0.009
23.72	2.30	43.10	1.94	83.73	20	932	0.66	1.092	1.11	19.28	1.24	0.013
23.77	2.30	44.49	1.93	85.69	20	958	0.66	0.956	0.95	19.28	1.06	0.006
23.86	2.30	45.20	1.93	87.07	20	976	0.66	0.880	0.85	19.28	0.96	0.010
23.90	2.28	46.12	1.86	85.97	20	974	0.66	0.891	0.89	19.28	0.99	0.005
23.98	2.25	47.07	1.74	81.90	19	947	0.66	1.036	1.11	19.28	1.25	0.012
24.04	2.21	48.00	1.63	78.28	18	917	0.66	1.228	1.42	19.28	1.59	0.011
24.09	2.19	48.52	1.59	77.22	17	908	0.66	1.299	1.54	19.28	1.72	0.010
24.18	2.21	48.16	1.63	78.43	18	922	0.66	1.208	1.40	19.28	1.56	0.017
24.22	2.23	47.47	1.69	80.03	18	937	0.66	1.117	1.25	19.28	1.40	0.008
24.31	2.25	46.74	1.74	81.40	19	949	0.66	1.056	1.14	19.28	1.28	0.013
24.37	2.26	46.25	1.79	82.97	19	962	0.66	0.991	1.04	19.28	1.16	0.009
24.46	2.27	46.18	1.82	83.95	19	973	0.66	0.946	0.98	19.28	1.09	0.013
24.50	2.26	47.14	1.80	84.79	20	987	0.66	0.888	0.91	19.28	1.02	0.004
24.56	2.24	49.18	1.73	85.27	20	1002	0.66	0.826	0.85	19.28	0.95	0.006
24.61	2.21	51.89	1.65	85.71	19	1018	0.66	0.771	0.80	19.28	0.89	0.005
24.71	2.19	53.80	1.60	85.92	19	1028	0.66	0.745	0.78	19.28	0.87	0.011
24.74	2.17	54.98	1.55	85.46	19	1025	0.66	0.757	0.80	19.28	0.90	0.003
24.81	2.16	55.26	1.53	84.49	19	1015	0.66	0.796	0.86	19.28	0.96	0.007
24.88	2.15	54.54	1.51	82.52	18	993	0.66	0.891	0.99	19.28	1.11	0.010
24.97	2.17	51.91	1.54	80.14	18	967	0.66	1.026	1.18	19.28	1.32	0.014
25.02	2.18	49.61	1.58	78.27	18	945	0.66	1.160	1.36	19.28	1.52	0.009
25.11	2.19	48.61	1.60	77.60	17	938	0.66	1.215	1.43	19.28	1.60	0.017
25.16	2.18	49.77	1.56	77.66	17	941	0.66	1.198	1.42	19.28	1.59	0.009
25.20	2.15	51.98	1.51	78.41	17	951	0.66	1.139	1.35	19.28	1.51	0.008
25.29	2.13	54.55	1.46	79.68	17	966	0.66	1.065	1.26	19.28	1.41	0.016

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
25.34	2.10	57.50	1.42	81.68	18	984	0.66	0.972	1.13	19.28	1.26	0.007
25.42	2.08	59.77	1.40	83.50	18	1002	0.66	0.896	1.02	19.28	1.14	0.011
25.48	2.07	61.76	1.38	85.29	18	1019	0.66	0.829	0.93	19.28	1.04	0.006
25.52	2.06	63.56	1.37	87.11	19	1038	0.66	0.764	0.83	19.28	0.93	0.006
25.61	2.06	65.25	1.36	88.82	19	1056	0.66	0.709	0.76	19.28	0.85	0.009
25.66	2.05	67.37	1.35	90.96	19	1077	0.66	0.653	0.68	19.28	0.76	0.004
25.76	2.04	69.05	1.34	92.57	20	1093	0.66	0.616	0.63	19.28	0.71	0.008
25.80	2.03	70.71	1.33	94.28	20	1109	0.66	0.580	0.59	19.28	0.66	0.003
25.89	2.03	71.50	1.33	95.19	20	1120	0.66	0.558	0.56	19.28	0.62	0.007
25.94	2.03	72.30	1.33	96.20	20	1133	0.66	0.534	0.53	19.28	0.59	0.003
26.03	2.03	72.53	1.33	96.59	20	1141	0.66	0.522	0.51	19.28	0.57	0.006
26.08	2.03	72.65	1.33	96.87	20	1147	0.66	0.513	0.50	19.28	0.56	0.003
26.13	2.03	72.74	1.33	97.02	20	1150	0.66	0.508	0.50	19.28	0.55	0.004
26.21	2.03	73.00	1.33	97.27	20	1155	0.66	0.503	0.49	19.28	0.55	0.005
26.33	2.03	73.30	1.33	97.48	20	1158	0.67	0.502	0.49	19.28	0.55	0.007
26.35	2.02	73.74	1.33	97.71	20	1158	0.67	0.504	0.49	19.28	0.55	0.002
26.40	2.02	74.01	1.32	97.74	20	1155	0.67	0.510	0.50	19.28	0.56	0.003
26.45	2.01	73.96	1.32	97.32	20	1147	0.67	0.527	0.52	19.28	0.58	0.003
26.54	2.01	73.77	1.31	96.80	20	1140	0.67	0.546	0.54	19.28	0.60	0.006
26.59	2.00	73.52	1.31	96.33	20	1134	0.67	0.561	0.56	19.28	0.63	0.004
26.67	2.00	74.31	1.30	96.89	20	1136	0.67	0.560	0.56	19.28	0.62	0.006
26.72	1.95	77.88	1.27	99.13	20	1125	0.67	0.588	0.58	19.28	0.65	0.004
26.81	1.91	82.17	1.24	102.02	21	1114	0.67	0.618	0.60	19.28	0.67	0.008
26.85	1.86	87.47	1.21	105.51	21	1107	0.67	0.637	0.61	19.28	0.68	0.003
26.91	1.83	92.55	1.18	109.11	21	1122	0.67	0.603	0.56	19.28	0.63	0.004
27.00	1.79	98.41	1.14	112.26	22	1137	0.67	0.572	0.52	19.28	0.58	0.006
27.05	1.79	99.03	1.14	113.11	22	1147	0.67	0.553	0.50	19.28	0.56	0.003
27.11	1.77	103.59	1.12	115.63	22	1169	0.67	0.513	0.45	19.28	0.51	0.004
27.17	1.75	109.28	1.08	118.37	22	1198	0.67	0.465	0.40	19.28	0.45	0.003
27.25	1.69	121.07	1.00	121.07	23	1230	0.67	0.421	0.36	19.28	0.41	0.004
27.30	1.68	128.62	1.00	128.62	24	1280	0.67	0.361	0.29	19.28	0.33	0.002
27.39	1.68	134.19	1.00	134.19	25	1336	0.67	0.309	0.24	19.28	0.27	0.003
27.44	1.69	138.55	1.00	138.55	26	1405	0.67	0.260	0.19	19.28	0.21	0.001
27.53	1.69	141.65	1.00	141.65	26	1445	0.67	0.238	0.17	19.28	0.19	0.002
27.62	1.69	144.07	1.00	144.07	27	1472	0.67	0.225	0.16	19.28	0.18	0.002
27.67	1.69	145.23	1.00	145.23	27	1476	0.67	0.224	0.16	19.28	0.17	0.001
27.72	1.69	145.08	1.00	145.08	27	1475	0.67	0.225	0.16	19.28	0.18	0.001
27.76	1.69	143.36	1.00	143.36	27	1472	0.67	0.227	0.16	19.28	0.18	0.001
27.86	1.70	141.60	1.00	141.60	26	1470	0.67	0.230	0.16	19.28	0.18	0.002
27.90	1.71	140.13	1.00	140.13	26	1466	0.67	0.232	0.17	19.28	0.19	0.001
27.99	1.70	140.22	1.00	140.22	26	1466	0.67	0.234	0.17	19.28	0.19	0.002
28.04	1.70	141.07	1.00	141.07	26	1467	0.67	0.234	0.17	19.28	0.19	0.001
28.09	1.69	142.92	1.00	142.92	27	1472	0.67	0.232	0.16	19.28	0.18	0.001
28.18	1.68	144.70	1.00	144.70	27	1478	0.67	0.230	0.16	19.28	0.18	0.002
28.23	1.67	147.22	1.00	147.22	27	1484	0.67	0.228	0.16	19.28	0.18	0.001
28.32	1.67	148.87	1.00	148.87	27	1491	0.67	0.226	0.15	19.28	0.17	0.002
28.36	1.66	150.69	1.00	150.69	28	1500	0.67	0.222	0.15	19.28	0.17	0.001
28.45	1.66	151.14	1.00	151.14	28	1506	0.67	0.220	0.15	19.28	0.17	0.002

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
28.50	1.66	151.52	1.00	151.52	28	1507	0.67	0.220	0.15	19.28	0.17	0.001
28.55	1.66	150.76	1.00	150.76	28	1493	0.67	0.228	0.15	19.28	0.17	0.001
28.63	1.67	147.86	1.00	147.86	27	1482	0.67	0.234	0.16	19.28	0.18	0.002
28.68	1.68	145.10	1.00	145.10	27	1478	0.67	0.237	0.17	19.28	0.19	0.001
28.77	1.69	143.97	1.00	143.97	27	1491	0.67	0.232	0.16	19.28	0.18	0.002
28.83	1.69	144.56	1.00	144.56	27	1503	0.67	0.227	0.16	19.28	0.18	0.001
28.91	1.69	144.50	1.00	144.50	27	1513	0.67	0.223	0.16	19.28	0.17	0.002
28.96	1.70	145.05	1.00	145.05	27	1529	0.67	0.216	0.15	19.28	0.17	0.001
29.05	1.69	146.89	1.00	146.89	27	1546	0.67	0.210	0.14	19.28	0.16	0.002
29.10	1.68	150.40	1.00	150.40	28	1562	0.67	0.204	0.14	19.28	0.15	0.001
29.15	1.67	153.38	1.00	153.38	28	1571	0.67	0.201	0.13	19.28	0.15	0.001
29.20	1.66	155.64	1.00	155.64	29	1570	0.67	0.202	0.13	19.28	0.15	0.001
29.28	1.65	157.07	1.00	157.07	29	1562	0.67	0.206	0.13	19.28	0.15	0.001
29.38	1.64	158.42	1.00	158.42	29	1552	0.67	0.211	0.14	19.28	0.15	0.002
29.46	1.63	159.30	1.00	159.30	29	1548	0.67	0.214	0.14	19.28	0.15	0.001
29.47	1.63	159.67	1.00	159.67	29	1547	0.67	0.215	0.14	19.28	0.15	0.000
29.55	1.63	158.31	1.00	158.31	29	1544	0.67	0.217	0.14	19.28	0.16	0.001
29.61	1.64	155.43	1.00	155.43	28	1539	0.67	0.220	0.14	19.28	0.16	0.001
29.70	1.66	151.29	1.00	151.29	28	1537	0.67	0.222	0.15	19.28	0.17	0.002
29.75	1.68	147.52	1.00	147.52	27	1538	0.67	0.222	0.15	19.28	0.17	0.001
29.79	1.69	145.41	1.00	145.41	27	1547	0.67	0.218	0.15	19.28	0.17	0.001
29.88	1.70	145.23	1.00	145.23	27	1561	0.67	0.213	0.15	19.28	0.17	0.002
29.93	1.69	148.50	1.00	148.50	28	1583	0.67	0.205	0.14	19.28	0.16	0.001
30.02	1.68	152.83	1.00	152.83	28	1601	0.67	0.199	0.13	19.28	0.15	0.002
30.06	1.66	159.34	1.00	159.34	29	1621	0.67	0.192	0.12	19.28	0.14	0.001
30.16	1.65	163.01	1.00	163.01	30	1633	0.67	0.189	0.12	19.28	0.13	0.002
30.20	1.64	165.58	1.00	165.58	30	1645	0.67	0.185	0.11	19.28	0.13	0.001
30.25	1.65	163.84	1.00	163.84	30	1644	0.67	0.186	0.11	19.28	0.13	0.001
30.39	1.65	161.29	1.00	161.29	30	1638	0.67	0.189	0.12	19.28	0.13	0.002
30.43	1.66	158.44	1.00	158.44	29	1628	0.67	0.193	0.12	19.28	0.14	0.001
30.45	1.67	155.85	1.00	155.85	29	1620	0.67	0.196	0.13	19.28	0.14	0.000
30.53	1.68	151.40	1.00	151.40	28	1610	0.67	0.201	0.13	19.28	0.15	0.001
30.62	1.70	147.08	1.00	147.08	27	1601	0.67	0.205	0.14	19.28	0.16	0.002
30.66	1.71	143.69	1.01	144.57	27	1591	0.67	0.210	0.15	19.28	0.16	0.001
30.71	1.71	143.18	1.00	143.29	27	1581	0.67	0.214	0.15	19.28	0.17	0.001
30.80	1.71	141.10	1.01	142.29	27	1569	0.67	0.221	0.16	19.28	0.18	0.002
30.85	1.70	141.91	1.00	141.91	26	1558	0.67	0.226	0.16	19.28	0.18	0.001
30.97	1.70	142.37	1.00	142.37	27	1554	0.67	0.230	0.16	19.28	0.18	0.003
31.00	1.68	146.29	1.00	146.29	27	1555	0.67	0.230	0.16	19.28	0.18	0.001
31.08	1.67	148.59	1.00	148.59	27	1559	0.67	0.229	0.16	19.28	0.18	0.002
31.13	1.66	150.09	1.00	150.09	28	1561	0.67	0.228	0.15	19.28	0.17	0.001
31.17	1.66	150.65	1.00	150.65	28	1558	0.67	0.230	0.16	19.28	0.17	0.001
31.27	1.64	151.19	1.00	151.19	28	1542	0.67	0.239	0.16	19.28	0.18	0.002
31.30	1.61	153.47	1.00	153.47	28	1502	0.67	0.261	0.18	19.28	0.20	0.001
31.40	1.58	156.70	1.00	156.70	28	1470	0.67	0.282	0.19	19.28	0.21	0.002
31.45	1.54	161.75	1.00	161.75	29	1454	0.67	0.294	0.19	19.28	0.21	0.001
31.50	1.53	166.20	1.00	166.20	29	1463	0.67	0.288	0.18	19.28	0.20	0.001
31.59	1.52	169.15	1.00	169.15	30	1472	0.67	0.284	0.18	19.28	0.20	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
31.64	1.52	170.15	1.00	170.15	30	1488	0.67	0.274	0.17	19.28	0.19	0.001
31.72	1.53	170.15	1.00	170.15	30	1510	0.67	0.262	0.16	19.28	0.18	0.002
31.77	1.54	170.88	1.00	170.88	30	1533	0.67	0.250	0.15	19.28	0.17	0.001
31.86	1.54	171.62	1.00	171.62	30	1547	0.67	0.244	0.15	19.28	0.17	0.002
31.91	1.54	172.38	1.00	172.38	31	1558	0.67	0.238	0.14	19.28	0.16	0.001
31.96	1.55	171.46	1.00	171.46	30	1571	0.67	0.233	0.14	19.28	0.16	0.001
32.05	1.56	169.68	1.00	169.68	30	1583	0.67	0.228	0.14	19.28	0.16	0.002
32.10	1.58	166.90	1.00	166.90	30	1590	0.67	0.226	0.14	19.28	0.16	0.001
32.19	1.59	164.74	1.00	164.74	30	1592	0.67	0.226	0.14	19.28	0.16	0.002
32.23	1.60	162.49	1.00	162.49	29	1588	0.67	0.228	0.14	19.28	0.16	0.001
32.32	1.61	160.13	1.00	160.13	29	1580	0.67	0.233	0.15	19.28	0.17	0.002
32.37	1.62	155.20	1.00	155.20	28	1553	0.67	0.247	0.16	19.28	0.18	0.001
32.47	1.63	149.90	1.00	149.90	27	1522	0.67	0.265	0.18	19.28	0.20	0.002
32.51	1.64	143.74	1.00	143.74	26	1488	0.67	0.286	0.21	19.28	0.23	0.001
32.56	1.66	136.43	1.00	136.43	25	1457	0.67	0.309	0.23	19.28	0.26	0.002
32.65	1.68	129.26	1.00	129.26	24	1425	0.67	0.335	0.27	19.28	0.30	0.003
32.69	1.70	121.73	1.00	121.73	23	1382	0.67	0.376	0.32	19.28	0.36	0.002
32.79	1.72	116.68	1.02	118.91	22	1351	0.67	0.411	0.36	19.28	0.40	0.005
32.84	1.73	112.17	1.05	117.33	22	1325	0.67	0.444	0.39	19.28	0.44	0.003
32.88	1.74	109.11	1.07	116.85	22	1316	0.67	0.458	0.41	19.28	0.45	0.002
32.97	1.76	106.75	1.10	116.93	22	1317	0.67	0.457	0.40	19.28	0.45	0.005
33.01	1.77	105.25	1.12	118.05	23	1334	0.67	0.436	0.38	19.28	0.42	0.002
33.11	1.77	106.71	1.12	119.61	23	1354	0.67	0.414	0.35	19.28	0.39	0.005
33.16	1.75	112.43	1.08	121.62	23	1375	0.67	0.392	0.33	19.28	0.37	0.002
33.24	1.71	119.10	1.01	120.54	23	1383	0.67	0.385	0.33	19.28	0.37	0.004
33.30	1.68	125.71	1.00	125.71	23	1395	0.67	0.374	0.31	19.28	0.35	0.002
33.34	1.67	129.32	1.00	129.32	24	1410	0.67	0.360	0.29	19.28	0.33	0.002
33.42	1.66	132.95	1.00	132.95	24	1432	0.67	0.341	0.27	19.28	0.30	0.003
33.48	1.65	136.38	1.00	136.38	25	1456	0.67	0.322	0.25	19.28	0.28	0.002
33.53	1.64	139.96	1.00	139.96	26	1473	0.67	0.311	0.23	19.28	0.26	0.002
33.62	1.63	142.29	1.00	142.29	26	1483	0.67	0.305	0.22	19.28	0.25	0.003
33.66	1.63	143.22	1.00	143.22	26	1485	0.67	0.304	0.22	19.28	0.25	0.001
33.74	1.63	142.70	1.00	142.70	26	1485	0.67	0.304	0.22	19.28	0.25	0.002
33.80	1.63	141.51	1.00	141.51	26	1482	0.67	0.307	0.23	19.28	0.25	0.002
33.89	1.64	139.17	1.00	139.17	25	1477	0.67	0.313	0.23	19.28	0.26	0.003
33.99	1.65	136.72	1.00	136.72	25	1465	0.67	0.323	0.25	19.28	0.28	0.003
34.02	1.66	133.31	1.00	133.31	25	1446	0.67	0.338	0.27	19.28	0.30	0.001
34.08	1.67	128.60	1.00	128.60	24	1422	0.67	0.360	0.29	19.28	0.33	0.002
34.13	1.69	121.14	1.00	121.14	23	1389	0.67	0.394	0.34	19.28	0.38	0.002
34.21	1.73	111.33	1.05	117.25	22	1352	0.67	0.437	0.39	19.28	0.43	0.004
34.26	1.77	100.37	1.11	111.44	21	1284	0.67	0.539	0.50	19.28	0.56	0.003
34.34	1.79	89.34	1.14	102.19	20	1188	0.67	0.753	0.77	19.28	0.86	0.009
34.40	1.83	78.17	1.18	92.46	18	1098	0.67	1.085	1.23	19.28	1.37	0.009
34.49	1.88	69.68	1.22	85.19	17	1055	0.67	1.329	1.62	19.28	1.82	0.021
34.53	1.97	61.31	1.28	78.58	16	1057	0.67	1.320	1.71	19.28	1.91	0.009
34.63	2.03	55.83	1.34	74.65	16	1062	0.67	1.292	1.72	19.28	1.93	0.023
34.68	2.12	49.24	1.45	71.28	16	1063	0.67	1.287	1.74	19.28	1.95	0.010
34.72	2.18	45.77	1.56	71.22	16	1075	0.67	1.220	1.61	19.28	1.80	0.009

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
34.82	2.22	43.27	1.67	72.13	16	1088	0.67	1.155	1.47	19.28	1.64	0.019
34.84	2.24	43.13	1.72	74.09	17	1114	0.67	1.032	1.26	19.28	1.41	0.005
34.94	2.25	43.30	1.75	75.63	17	1137	0.67	0.943	1.11	19.28	1.25	0.015
35.00	2.25	43.78	1.77	77.36	18	1162	0.67	0.855	0.98	19.28	1.10	0.007
35.04	2.25	45.04	1.76	79.06	18	1190	0.67	0.769	0.86	19.28	0.96	0.005
35.14	2.24	46.56	1.73	80.52	18	1217	0.67	0.700	0.77	19.28	0.86	0.010
35.19	2.23	48.24	1.69	81.75	19	1240	0.67	0.647	0.70	19.28	0.79	0.005
35.27	2.22	49.40	1.66	82.04	19	1249	0.67	0.629	0.68	19.28	0.77	0.008
35.32	2.21	50.22	1.63	81.88	19	1250	0.67	0.629	0.69	19.28	0.77	0.005
35.37	2.19	51.40	1.59	81.54	18	1247	0.67	0.636	0.71	19.28	0.79	0.005
35.46	2.17	52.69	1.54	81.37	18	1245	0.67	0.642	0.72	19.28	0.81	0.009
35.51	2.15	54.34	1.50	81.47	18	1244	0.67	0.646	0.73	19.28	0.82	0.005
35.56	2.13	55.30	1.47	81.41	18	1240	0.67	0.656	0.75	19.28	0.84	0.006
35.64	2.12	55.89	1.45	81.10	18	1232	0.67	0.676	0.78	19.28	0.88	0.008
35.73	2.12	55.85	1.44	80.59	18	1224	0.67	0.697	0.82	19.28	0.91	0.010
35.77	2.12	55.52	1.44	80.13	17	1218	0.67	0.714	0.84	19.28	0.94	0.004
35.86	2.12	54.79	1.45	79.64	17	1216	0.67	0.723	0.86	19.28	0.96	0.011
35.92	2.13	53.96	1.47	79.12	17	1212	0.67	0.735	0.87	19.28	0.98	0.007
35.97	2.13	53.37	1.47	78.67	17	1208	0.67	0.748	0.89	19.28	1.00	0.006
36.05	2.14	52.77	1.48	78.22	17	1204	0.67	0.760	0.91	19.28	1.02	0.009
36.11	2.14	52.23	1.49	77.89	17	1201	0.67	0.769	0.92	19.28	1.03	0.008
36.17	2.15	51.60	1.50	77.62	17	1200	0.67	0.773	0.93	19.28	1.04	0.007
36.24	2.16	50.97	1.52	77.40	17	1200	0.67	0.776	0.93	19.28	1.05	0.009
36.29	2.16	50.39	1.53	77.22	17	1199	0.67	0.780	0.94	19.28	1.05	0.007
36.38	2.17	49.92	1.54	77.08	17	1200	0.67	0.781	0.94	19.28	1.05	0.011
36.43	2.17	49.57	1.56	77.08	17	1201	0.67	0.777	0.93	19.28	1.04	0.006
36.52	2.18	49.43	1.56	77.19	17	1205	0.67	0.769	0.92	19.28	1.03	0.011
36.56	2.18	49.59	1.56	77.41	17	1210	0.67	0.758	0.90	19.28	1.01	0.005
36.62	2.17	50.43	1.54	77.62	17	1213	0.67	0.750	0.89	19.28	1.00	0.006
36.69	2.15	52.06	1.50	78.00	17	1217	0.67	0.742	0.89	19.28	0.99	0.009
36.75	2.09	56.90	1.41	80.01	17	1225	0.67	0.722	0.86	19.28	0.97	0.006
36.88	2.02	63.55	1.33	84.44	18	1241	0.67	0.685	0.79	19.28	0.89	0.014
36.98	1.97	70.20	1.28	89.93	18	1259	0.67	0.647	0.71	19.28	0.80	0.010
37.03	1.95	72.92	1.27	92.45	19	1272	0.67	0.621	0.67	19.28	0.75	0.004
37.09	1.96	71.39	1.28	91.18	19	1272	0.67	0.622	0.67	19.28	0.75	0.006
37.16	1.99	67.58	1.30	87.89	18	1263	0.67	0.641	0.72	19.28	0.80	0.007
37.21	2.03	63.33	1.33	84.41	18	1252	0.67	0.668	0.77	19.28	0.86	0.005
37.29	2.06	60.13	1.37	82.49	18	1256	0.67	0.660	0.77	19.28	0.86	0.008
37.35	2.10	58.22	1.42	82.77	18	1287	0.67	0.598	0.68	19.28	0.76	0.005
37.44	2.13	57.39	1.47	84.51	19	1331	0.67	0.523	0.57	19.28	0.64	0.007
37.49	2.17	56.14	1.54	86.63	19	1375	0.67	0.460	0.48	19.28	0.54	0.003
37.54	2.19	57.00	1.58	90.06	20	1433	0.67	0.396	0.39	19.28	0.44	0.003
37.63	2.20	58.30	1.61	93.68	21	1493	0.67	0.342	0.32	19.28	0.36	0.004
37.68	2.19	61.38	1.60	98.13	22	1565	0.67	0.292	0.26	19.28	0.29	0.002
37.77	2.19	63.16	1.60	101.13	23	1616	0.66	0.264	0.23	19.28	0.25	0.002
37.82	2.19	65.15	1.59	103.68	23	1658	0.66	0.243	0.20	19.28	0.23	0.001
37.87	2.17	67.95	1.55	105.40	24	1686	0.66	0.231	0.19	19.28	0.21	0.001
37.97	2.16	70.24	1.52	106.55	24	1703	0.66	0.225	0.18	19.28	0.21	0.003

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
38.01	2.14	71.79	1.49	107.08	24	1708	0.66	0.223	0.18	19.28	0.20	0.001
38.09	2.14	71.59	1.49	106.56	23	1702	0.66	0.226	0.19	19.28	0.21	0.002
38.14	2.14	70.76	1.49	105.68	23	1690	0.66	0.231	0.19	19.28	0.22	0.001
38.19	2.15	69.02	1.51	104.20	23	1671	0.66	0.240	0.20	19.28	0.23	0.001
38.26	2.16	66.74	1.53	102.21	23	1644	0.66	0.253	0.22	19.28	0.24	0.002
38.33	2.18	63.59	1.56	99.51	22	1606	0.66	0.273	0.24	19.28	0.27	0.002
38.42	2.19	61.03	1.59	97.18	22	1571	0.66	0.294	0.26	19.28	0.30	0.003
38.47	2.20	59.28	1.60	95.03	21	1538	0.66	0.317	0.29	19.28	0.33	0.002
38.56	2.19	58.87	1.59	93.67	21	1518	0.66	0.332	0.31	19.28	0.35	0.004
38.60	2.18	59.17	1.57	92.71	21	1503	0.66	0.344	0.33	19.28	0.37	0.002
38.65	2.16	60.13	1.53	92.11	20	1492	0.66	0.353	0.34	19.28	0.38	0.002
38.74	2.14	61.83	1.48	91.54	20	1477	0.66	0.367	0.36	19.28	0.41	0.004
38.84	2.11	63.96	1.43	91.41	20	1461	0.66	0.382	0.39	19.28	0.43	0.005
38.86	2.08	66.22	1.39	91.73	20	1444	0.66	0.399	0.41	19.28	0.46	0.002
38.91	2.03	69.35	1.33	92.43	19	1410	0.66	0.437	0.45	19.28	0.51	0.003
39.04	1.98	72.09	1.29	93.27	19	1372	0.66	0.486	0.51	19.28	0.57	0.009
39.06	1.93	75.44	1.26	94.71	19	1321	0.66	0.564	0.59	19.28	0.66	0.002
39.16	1.89	77.90	1.23	95.76	19	1288	0.66	0.629	0.66	19.28	0.74	0.008
39.24	1.85	80.75	1.20	96.74	19	1255	0.66	0.702	0.75	19.28	0.83	0.009
39.34	1.82	82.55	1.18	97.29	19	1243	0.66	0.733	0.78	19.28	0.87	0.010
39.39	1.82	83.58	1.17	97.91	19	1245	0.66	0.728	0.77	19.28	0.86	0.005
39.44	1.83	83.66	1.18	98.79	19	1266	0.66	0.680	0.71	19.28	0.79	0.005
39.52	1.85	83.11	1.20	99.69	20	1300	0.66	0.609	0.62	19.28	0.70	0.007
39.62	1.87	82.06	1.22	100.05	20	1336	0.66	0.546	0.55	19.28	0.62	0.007
39.66	1.90	80.75	1.24	99.80	20	1365	0.66	0.503	0.50	19.28	0.56	0.003
39.70	1.94	77.57	1.26	97.99	20	1396	0.66	0.461	0.46	19.28	0.52	0.002
39.80	2.00	72.72	1.30	94.80	20	1427	0.66	0.426	0.43	19.28	0.49	0.006
39.85	2.07	66.38	1.38	91.53	20	1460	0.66	0.393	0.40	19.28	0.45	0.003
39.93	2.14	60.60	1.48	89.78	20	1478	0.66	0.376	0.38	19.28	0.43	0.004
39.98	2.19	56.35	1.60	90.02	20	1495	0.66	0.362	0.36	19.28	0.40	0.002
40.03	2.23	53.86	1.69	91.27	21	1513	0.66	0.348	0.33	19.28	0.37	0.002
40.12	2.25	52.67	1.76	92.63	21	1531	0.66	0.334	0.31	19.28	0.35	0.004
40.18	2.27	52.02	1.81	94.17	22	1551	0.66	0.320	0.29	19.28	0.32	0.002
40.25	2.27	51.99	1.83	95.16	22	1566	0.66	0.311	0.27	19.28	0.31	0.003
40.31	2.27	52.73	1.81	95.19	22	1572	0.66	0.307	0.27	19.28	0.31	0.002
40.39	2.25	53.85	1.75	94.32	22	1567	0.66	0.311	0.28	19.28	0.31	0.003
40.45	2.23	55.22	1.69	93.06	21	1554	0.66	0.320	0.30	19.28	0.33	0.002
40.49	2.21	56.57	1.63	92.43	21	1548	0.66	0.324	0.31	19.28	0.34	0.002
40.58	2.19	57.79	1.60	92.32	21	1549	0.66	0.324	0.31	19.28	0.35	0.004
40.63	2.18	59.11	1.56	92.49	21	1552	0.66	0.322	0.31	19.28	0.35	0.002
40.70	2.17	60.41	1.54	93.15	21	1564	0.66	0.315	0.30	19.28	0.34	0.003
40.75	2.16	61.85	1.53	94.42	21	1584	0.66	0.302	0.29	19.28	0.32	0.002
40.84	2.15	64.03	1.50	96.34	21	1615	0.66	0.284	0.26	19.28	0.29	0.003
40.90	2.13	67.18	1.47	98.72	22	1648	0.66	0.266	0.24	19.28	0.27	0.002
40.97	2.11	70.44	1.43	100.92	22	1673	0.65	0.255	0.23	19.28	0.26	0.002
41.03	2.10	72.46	1.41	102.36	22	1688	0.65	0.248	0.22	19.28	0.25	0.002
41.08	2.09	72.57	1.41	102.32	22	1687	0.65	0.249	0.22	19.28	0.25	0.001
41.16	2.10	71.45	1.42	101.35	22	1678	0.65	0.253	0.23	19.28	0.25	0.003

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
41.21	2.12	68.24	1.45	99.08	22	1657	0.65	0.264	0.24	19.28	0.27	0.002
41.30	2.14	65.15	1.49	97.13	21	1638	0.65	0.274	0.25	19.28	0.28	0.003
41.34	2.17	62.06	1.54	95.31	21	1616	0.65	0.286	0.27	19.28	0.30	0.001
41.44	2.17	60.57	1.55	94.15	21	1601	0.65	0.295	0.28	19.28	0.31	0.004
41.49	2.18	59.13	1.57	93.10	21	1586	0.65	0.305	0.29	19.28	0.32	0.002
41.54	2.20	57.74	1.60	92.60	21	1579	0.65	0.310	0.29	19.28	0.33	0.002
41.63	2.22	55.96	1.66	92.67	21	1581	0.65	0.309	0.29	19.28	0.33	0.003
41.68	2.25	53.33	1.75	93.28	21	1585	0.65	0.307	0.28	19.28	0.32	0.002
41.74	2.28	50.65	1.86	94.20	22	1586	0.65	0.306	0.27	19.28	0.31	0.002
41.81	2.32	47.81	1.99	95.23	23	1581	0.65	0.310	0.27	19.28	0.30	0.002
41.87	2.34	45.68	2.11	96.26	23	1577	0.65	0.313	0.26	19.28	0.29	0.002
41.96	2.37	44.00	2.21	97.22	24	1575	0.65	0.315	0.26	19.28	0.29	0.003
42.00	2.38	43.05	2.28	98.20	24	1577	0.65	0.314	0.25	19.28	0.28	0.001
42.09	2.39	42.69	2.32	98.84	24	1583	0.65	0.310	0.25	19.28	0.28	0.003
42.15	2.39	43.02	2.31	99.41	24	1594	0.65	0.303	0.24	19.28	0.27	0.002
42.23	2.38	43.78	2.28	99.70	24	1608	0.65	0.295	0.23	19.28	0.26	0.003
42.28	2.37	44.68	2.24	99.87	24	1621	0.65	0.288	0.23	19.28	0.26	0.002
42.33	2.37	45.08	2.22	100.11	24	1630	0.65	0.283	0.22	19.28	0.25	0.001
42.41	2.37	45.28	2.21	100.19	24	1635	0.65	0.281	0.22	19.28	0.25	0.003
42.47	2.36	45.80	2.18	100.01	24	1640	0.65	0.278	0.22	19.28	0.25	0.002
42.55	2.35	46.50	2.14	99.56	24	1644	0.65	0.276	0.22	19.28	0.25	0.003
42.59	2.34	47.61	2.07	98.76	24	1646	0.65	0.276	0.23	19.28	0.25	0.001
42.66	2.33	48.07	2.05	98.69	24	1651	0.65	0.273	0.22	19.28	0.25	0.002
42.74	2.33	48.33	2.04	98.81	24	1657	0.65	0.270	0.22	19.28	0.25	0.002
42.79	2.33	48.32	2.06	99.32	24	1665	0.65	0.267	0.22	19.28	0.24	0.001
42.88	2.33	48.70	2.04	99.11	24	1668	0.65	0.265	0.22	19.28	0.24	0.003
42.92	2.31	49.82	1.97	98.23	23	1667	0.65	0.266	0.22	19.28	0.25	0.001
43.01	2.28	51.94	1.84	95.77	22	1650	0.65	0.275	0.24	19.28	0.27	0.003
43.06	2.23	55.03	1.69	92.79	21	1620	0.65	0.292	0.27	19.28	0.31	0.002
43.11	2.17	58.81	1.55	91.00	20	1589	0.65	0.311	0.31	19.28	0.34	0.002
43.20	2.12	63.00	1.45	91.28	20	1574	0.65	0.321	0.32	19.28	0.36	0.004
43.29	2.08	66.26	1.40	92.51	20	1570	0.65	0.325	0.33	19.28	0.37	0.004
43.33	2.09	64.46	1.41	90.84	20	1551	0.64	0.338	0.35	19.28	0.39	0.002
43.37	2.08	65.32	1.39	91.01	20	1544	0.64	0.343	0.35	19.28	0.39	0.002
43.45	2.07	66.39	1.38	91.45	20	1542	0.64	0.346	0.35	19.28	0.40	0.004
43.51	2.03	71.39	1.33	95.03	20	1554	0.64	0.337	0.34	19.28	0.38	0.003
43.60	2.03	71.86	1.33	95.54	20	1562	0.64	0.331	0.33	19.28	0.37	0.004
43.65	2.03	71.56	1.34	95.81	20	1580	0.64	0.319	0.31	19.28	0.35	0.002
43.74	2.05	71.03	1.35	96.05	20	1602	0.64	0.305	0.30	19.28	0.33	0.004
43.78	2.06	70.63	1.37	96.49	21	1624	0.64	0.292	0.28	19.28	0.32	0.002
43.84	2.08	70.41	1.39	97.53	21	1661	0.64	0.273	0.26	19.28	0.29	0.002
43.92	2.09	70.31	1.41	98.95	21	1703	0.64	0.252	0.23	19.28	0.26	0.003
43.97	2.10	71.73	1.42	101.58	22	1755	0.64	0.231	0.21	19.28	0.23	0.001
44.06	2.09	73.84	1.41	103.85	22	1791	0.64	0.218	0.19	19.28	0.21	0.003
44.11	2.06	79.96	1.36	108.97	23	1840	0.64	0.202	0.17	19.28	0.19	0.001
44.20	2.01	88.12	1.32	116.03	24	1891	0.64	0.187	0.15	19.28	0.17	0.002
44.25	1.95	100.81	1.27	127.83	26	1961	0.64	0.170	0.12	19.28	0.14	0.001
44.34	1.90	110.92	1.24	137.29	28	2013	0.64	0.159	0.11	19.28	0.12	0.001

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
44.38	1.86	119.78	1.21	144.88	29	2045	0.64	0.153	0.10	19.28	0.11	0.001
44.43	1.83	124.93	1.19	148.21	29	2043	0.64	0.154	0.10	19.28	0.11	0.001
44.52	1.81	128.62	1.16	149.55	29	2028	0.64	0.157	0.10	19.28	0.11	0.001
44.56	1.78	131.52	1.13	149.21	29	1999	0.64	0.163	0.11	19.28	0.12	0.001
44.66	1.77	131.82	1.12	147.34	28	1967	0.64	0.170	0.11	19.28	0.13	0.001
44.70	1.76	131.34	1.10	144.81	28	1929	0.64	0.179	0.12	19.28	0.14	0.001
44.75	1.77	125.83	1.12	140.46	27	1877	0.64	0.193	0.14	19.28	0.15	0.001
44.84	1.78	119.80	1.13	135.83	26	1826	0.64	0.208	0.15	19.28	0.17	0.002
44.89	1.81	110.57	1.17	129.18	25	1767	0.64	0.229	0.17	19.28	0.19	0.001
44.98	1.84	103.75	1.19	123.89	24	1733	0.64	0.242	0.19	19.28	0.21	0.002
45.02	1.88	96.00	1.22	117.47	23	1703	0.64	0.256	0.21	19.28	0.24	0.001
45.12	1.90	92.26	1.24	114.02	23	1685	0.64	0.265	0.23	19.28	0.25	0.003
45.16	1.90	90.35	1.24	111.92	22	1662	0.64	0.276	0.24	19.28	0.27	0.001
45.22	1.89	91.58	1.23	112.53	22	1648	0.64	0.284	0.25	19.28	0.28	0.002
45.31	1.87	93.59	1.22	113.93	23	1644	0.64	0.286	0.25	19.28	0.28	0.003
45.35	1.86	95.89	1.21	115.93	23	1656	0.64	0.279	0.24	19.28	0.27	0.001
45.44	1.87	94.27	1.22	114.77	23	1659	0.63	0.278	0.24	19.28	0.27	0.003
45.48	1.87	95.30	1.21	115.68	23	1665	0.63	0.275	0.23	19.28	0.26	0.001
45.56	1.86	96.72	1.21	116.75	23	1669	0.63	0.274	0.23	19.28	0.26	0.002
45.62	1.82	102.47	1.18	120.78	24	1680	0.63	0.268	0.22	19.28	0.25	0.002
45.71	1.81	105.39	1.16	122.61	24	1689	0.63	0.264	0.21	19.28	0.24	0.003
45.76	1.80	107.69	1.15	123.97	24	1698	0.63	0.260	0.21	19.28	0.23	0.001
45.81	1.79	109.97	1.14	125.27	24	1707	0.63	0.256	0.20	19.28	0.23	0.001
45.90	1.78	111.92	1.13	126.21	24	1715	0.63	0.252	0.20	19.28	0.22	0.002
45.94	1.77	113.76	1.12	127.10	24	1724	0.63	0.249	0.20	19.28	0.22	0.001
46.03	1.77	114.74	1.11	127.72	24	1733	0.63	0.245	0.19	19.28	0.22	0.002
46.08	1.77	115.78	1.11	128.71	25	1747	0.63	0.239	0.19	19.28	0.21	0.001
46.17	1.77	116.46	1.11	129.66	25	1762	0.63	0.233	0.18	19.28	0.20	0.002
46.21	1.77	116.79	1.12	130.70	25	1779	0.63	0.227	0.17	19.28	0.19	0.001
46.27	1.78	116.99	1.13	131.97	25	1802	0.63	0.219	0.16	19.28	0.18	0.001
46.36	1.79	116.82	1.14	133.20	26	1829	0.63	0.210	0.16	19.28	0.17	0.002
46.40	1.81	115.93	1.16	134.69	26	1870	0.63	0.198	0.14	19.28	0.16	0.001
46.49	1.82	115.18	1.18	135.64	26	1906	0.63	0.188	0.13	19.28	0.15	0.002
46.55	1.84	114.26	1.19	136.19	27	1939	0.63	0.179	0.13	19.28	0.14	0.001
46.59	1.84	114.55	1.20	136.98	27	1959	0.63	0.175	0.12	19.28	0.14	0.001
46.66	1.85	115.03	1.20	138.05	27	1985	0.63	0.169	0.12	19.28	0.13	0.001
46.77	1.85	116.73	1.20	139.98	28	2014	0.63	0.163	0.11	19.28	0.12	0.002
46.81	1.85	118.47	1.20	141.92	28	2040	0.63	0.158	0.11	19.28	0.12	0.001
46.86	1.84	120.63	1.19	143.88	28	2058	0.63	0.154	0.10	19.28	0.11	0.001
46.95	1.84	122.16	1.19	145.15	28	2070	0.63	0.152	0.10	19.28	0.11	0.001
46.99	1.83	123.72	1.18	146.21	29	2074	0.63	0.152	0.10	19.28	0.11	0.000
47.05	1.82	124.13	1.18	146.09	28	2066	0.63	0.153	0.10	19.28	0.11	0.001
47.13	1.82	124.18	1.17	145.43	28	2050	0.63	0.156	0.10	19.28	0.12	0.001
47.19	1.81	123.35	1.17	144.07	28	2028	0.63	0.161	0.11	19.28	0.12	0.001
47.24	1.82	121.51	1.17	142.15	28	2005	0.63	0.166	0.11	19.28	0.13	0.001
47.32	1.82	119.79	1.17	140.37	27	1984	0.62	0.170	0.12	19.28	0.13	0.001
47.39	1.81	118.96	1.17	138.88	27	1959	0.62	0.176	0.12	19.28	0.14	0.001
47.46	1.80	119.33	1.15	137.37	27	1921	0.62	0.185	0.13	19.28	0.15	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
47.56	1.78	120.04	1.13	135.52	26	1880	0.62	0.196	0.14	19.28	0.16	0.002
47.60	1.76	121.19	1.10	133.70	25	1845	0.62	0.207	0.15	19.28	0.17	0.001
47.65	1.75	122.24	1.09	132.64	25	1829	0.62	0.212	0.16	19.28	0.18	0.001
47.74	1.74	122.59	1.08	131.88	25	1820	0.62	0.215	0.16	19.28	0.18	0.002
47.79	1.74	122.15	1.08	131.41	25	1815	0.62	0.217	0.17	19.28	0.19	0.001
47.87	1.74	121.55	1.07	129.74	25	1794	0.62	0.224	0.18	19.28	0.20	0.002
47.92	1.73	121.57	1.04	126.98	24	1761	0.62	0.237	0.19	19.28	0.21	0.001
47.97	1.71	122.78	1.01	124.17	23	1735	0.62	0.247	0.21	19.28	0.23	0.001
48.06	1.70	124.88	1.00	124.88	23	1732	0.62	0.249	0.21	19.28	0.23	0.003
48.11	1.69	128.43	1.00	128.43	24	1744	0.62	0.244	0.20	19.28	0.22	0.001
48.20	1.67	131.70	1.00	131.70	24	1750	0.62	0.241	0.19	19.28	0.21	0.002
48.25	1.67	131.99	1.00	131.99	24	1751	0.62	0.241	0.19	19.28	0.21	0.001
48.30	1.66	134.83	1.00	134.83	25	1755	0.62	0.240	0.19	19.28	0.21	0.001
48.39	1.64	138.20	1.00	138.20	25	1757	0.62	0.239	0.18	19.28	0.20	0.002
48.45	1.62	143.28	1.00	143.28	26	1771	0.62	0.233	0.17	19.28	0.19	0.001
48.52	1.62	144.59	1.00	144.59	26	1780	0.62	0.230	0.17	19.28	0.19	0.002
48.58	1.62	144.45	1.00	144.45	26	1788	0.62	0.227	0.16	19.28	0.18	0.001
48.64	1.63	142.69	1.00	142.69	26	1789	0.62	0.227	0.17	19.28	0.19	0.001
48.72	1.65	138.46	1.00	138.46	25	1783	0.62	0.229	0.17	19.28	0.19	0.002
48.81	1.67	133.47	1.00	133.47	25	1777	0.62	0.232	0.18	19.28	0.20	0.002
48.84	1.70	126.44	1.00	126.44	24	1762	0.62	0.237	0.19	19.28	0.22	0.001
48.92	1.72	120.28	1.04	124.52	23	1749	0.62	0.243	0.20	19.28	0.22	0.002
48.95	1.78	111.50	1.12	125.23	24	1763	0.62	0.237	0.19	19.28	0.21	0.001
49.04	1.83	102.90	1.19	122.08	24	1783	0.62	0.230	0.19	19.28	0.21	0.002
49.09	1.91	92.83	1.24	115.14	23	1803	0.62	0.223	0.19	19.28	0.21	0.001
49.18	1.96	85.03	1.28	108.48	22	1798	0.61	0.224	0.20	19.28	0.22	0.002
49.22	2.00	79.71	1.31	104.18	22	1800	0.61	0.224	0.20	19.28	0.23	0.001
49.31	2.03	76.00	1.34	101.55	21	1808	0.61	0.221	0.20	19.28	0.23	0.002
49.36	2.06	73.00	1.37	99.82	21	1819	0.61	0.217	0.20	19.28	0.23	0.001
49.41	2.10	69.52	1.41	98.29	21	1834	0.61	0.212	0.20	19.28	0.22	0.001
49.50	2.13	66.50	1.47	97.44	21	1849	0.61	0.208	0.19	19.28	0.22	0.002
49.55	2.16	63.44	1.53	96.95	22	1860	0.61	0.204	0.19	19.28	0.21	0.001
49.64	2.19	60.89	1.60	97.12	22	1874	0.61	0.200	0.18	19.28	0.20	0.002
49.72	2.22	59.11	1.66	98.00	22	1894	0.61	0.195	0.17	19.28	0.19	0.002
49.78	2.24	58.01	1.72	99.52	23	1922	0.61	0.187	0.16	19.28	0.18	0.001
49.82	2.26	57.34	1.78	101.87	24	1962	0.61	0.178	0.15	19.28	0.16	0.001
49.91	2.28	56.80	1.84	104.52	24	2006	0.61	0.168	0.13	19.28	0.15	0.002
49.97	2.29	56.57	1.90	107.29	25	2050	0.61	0.159	0.12	19.28	0.14	0.001
50.02	2.30	56.70	1.94	109.82	26	2092	0.61	0.151	0.11	19.28	0.12	0.001
50.14	2.31	56.85	1.96	111.53	26	2123	0.61	0.146	0.11	19.28	0.12	0.002
50.16	2.31	57.03	1.98	112.83	27	2144	0.61	0.143	0.10	19.28	0.11	0.000
50.25	2.31	57.11	1.98	112.89	27	2149	0.61	0.142	0.10	19.28	0.11	0.001
50.29	2.31	57.34	1.97	112.85	27	2151	0.61	0.142	0.10	19.28	0.11	0.000
50.33	2.30	58.09	1.93	112.32	26	2150	0.61	0.142	0.10	19.28	0.11	0.001
50.42	2.29	59.06	1.89	111.48	26	2147	0.61	0.143	0.10	19.28	0.12	0.001
50.47	2.27	60.33	1.83	110.31	26	2136	0.61	0.144	0.11	19.28	0.12	0.001
50.56	2.25	61.78	1.76	108.98	25	2123	0.61	0.147	0.11	19.28	0.12	0.001
50.65	2.23	63.55	1.70	107.84	25	2110	0.61	0.149	0.12	19.28	0.13	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
50.70	2.21	65.21	1.64	107.15	24	2100	0.61	0.150	0.12	19.28	0.13	0.001
50.74	2.20	66.19	1.61	106.69	24	2092	0.61	0.152	0.12	19.28	0.14	0.001
50.79	2.20	66.07	1.60	105.94	24	2078	0.61	0.154	0.13	19.28	0.14	0.001
50.89	2.20	65.26	1.61	105.07	24	2064	0.60	0.157	0.13	19.28	0.14	0.002
50.93	2.22	62.72	1.65	103.77	24	2041	0.60	0.162	0.13	19.28	0.15	0.001
51.02	2.24	60.07	1.71	102.68	23	2019	0.60	0.166	0.14	19.28	0.15	0.002
51.06	2.26	56.78	1.79	101.67	24	1992	0.60	0.172	0.14	19.28	0.16	0.001
51.12	2.27	54.43	1.83	99.72	23	1949	0.60	0.182	0.15	19.28	0.17	0.001
51.21	2.28	52.61	1.86	97.69	23	1908	0.60	0.192	0.16	19.28	0.18	0.002
51.25	2.27	52.08	1.83	95.42	22	1868	0.60	0.203	0.18	19.28	0.20	0.001
51.35	2.38	37.18	2.29	85.30	21	2149	0.60	0.143	0.14	19.28	0.15	0.002
51.39	2.39	36.70	2.32	85.09	21	2136	0.60	0.145	0.14	19.28	0.15	0.001
51.48	2.42	35.03	2.46	86.32	21	2115	0.60	0.148	0.14	19.28	0.15	0.002
51.51	2.44	33.72	2.59	87.30	22	2095	0.60	0.152	0.14	19.28	0.15	0.000
51.59	2.45	32.65	2.68	87.61	22	2072	0.60	0.156	0.14	19.28	0.15	0.001
51.68	2.45	32.62	2.66	86.79	22	2064	0.60	0.158	0.14	19.28	0.16	0.002
51.71	2.45	32.17	2.69	86.39	22	2047	0.60	0.161	0.14	19.28	0.16	0.001
51.81	2.45	32.03	2.68	85.99	22	2042	0.60	0.162	0.15	19.28	0.16	0.002
51.86	2.46	31.71	2.72	86.10	22	2037	0.60	0.163	0.15	19.28	0.16	0.001
51.91	2.46	31.83	2.72	86.65	22	2049	0.60	0.160	0.14	19.28	0.16	0.001
52.00	2.46	32.45	2.69	87.20	22	2078	0.60	0.155	0.14	19.28	0.15	0.002
52.05	2.44	33.61	2.61	87.69	22	2119	0.60	0.148	0.13	19.28	0.15	0.001
52.10	2.39	37.42	2.30	86.09	21	2199	0.60	0.136	0.13	19.28	0.14	0.001
52.19	2.33	41.80	2.03	84.92	20	2285	0.60	0.125	0.12	19.28	0.14	0.002
52.24	2.26	47.05	1.80	84.47	20	2375	0.60	0.115	0.12	19.28	0.13	0.001
52.33	2.24	49.61	1.71	84.77	19	2420	0.60	0.110	0.11	19.28	0.13	0.001
52.38	2.21	51.63	1.64	84.74	19	2444	0.60	0.108	0.11	19.28	0.13	0.001
52.47	2.19	52.84	1.59	84.08	19	2443	0.60	0.108	0.12	19.28	0.13	0.001
52.51	2.16	54.45	1.53	83.26	18	2432	0.60	0.109	0.12	19.28	0.13	0.001
52.56	2.14	55.69	1.48	82.29	18	2408	0.60	0.112	0.13	19.28	0.14	0.001
52.63	2.11	56.57	1.44	81.44	18	2380	0.59	0.114	0.13	19.28	0.15	0.001
52.70	2.10	56.58	1.42	80.41	17	2348	0.59	0.118	0.14	19.28	0.16	0.001
52.79	2.10	55.97	1.42	79.35	17	2320	0.59	0.121	0.15	19.28	0.16	0.002
52.83	2.11	54.19	1.44	77.77	17	2281	0.59	0.125	0.15	19.28	0.17	0.001
52.97	2.13	52.35	1.46	76.43	17	2253	0.59	0.129	0.16	19.28	0.18	0.003
53.02	2.14	50.63	1.49	75.55	17	2231	0.59	0.132	0.16	19.28	0.18	0.001
53.05	2.15	50.11	1.51	75.49	17	2230	0.59	0.132	0.16	19.28	0.18	0.001
53.15	2.16	49.76	1.52	75.49	17	2234	0.59	0.132	0.16	19.28	0.18	0.002
53.16	2.16	49.70	1.52	75.79	17	2243	0.59	0.130	0.16	19.28	0.18	0.000
53.25	2.16	49.76	1.53	76.12	17	2256	0.59	0.129	0.16	19.28	0.18	0.002
53.30	2.16	50.47	1.53	77.05	17	2286	0.59	0.125	0.15	19.28	0.17	0.001
53.39	2.16	51.44	1.52	78.17	17	2324	0.59	0.121	0.14	19.28	0.16	0.002
53.43	2.15	52.85	1.50	79.51	18	2366	0.59	0.116	0.14	19.28	0.15	0.001
53.48	2.14	54.66	1.48	80.86	18	2409	0.59	0.112	0.13	19.28	0.14	0.001
53.57	2.12	56.55	1.45	82.05	18	2445	0.59	0.109	0.12	19.28	0.14	0.002
53.62	2.09	59.57	1.41	83.79	18	2485	0.59	0.105	0.12	19.28	0.13	0.001
53.75	2.07	61.88	1.38	85.15	18	2513	0.59	0.103	0.11	19.28	0.13	0.002
53.76	2.04	64.72	1.35	87.09	18	2541	0.59	0.100	0.11	19.28	0.12	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
53.83	2.02	66.55	1.33	88.39	19	2559	0.59	0.099	0.11	19.28	0.12	0.001
53.90	2.01	68.28	1.31	89.64	19	2572	0.59	0.098	0.11	19.28	0.12	0.001
53.94	1.99	69.91	1.30	90.76	19	2577	0.59	0.098	0.11	19.28	0.12	0.001
54.03	1.98	70.58	1.29	91.14	19	2577	0.59	0.098	0.11	19.28	0.12	0.001
54.08	1.98	70.17	1.29	90.67	19	2568	0.59	0.099	0.11	19.28	0.12	0.001
54.16	2.00	67.80	1.31	88.69	18	2549	0.59	0.100	0.11	19.28	0.12	0.001
54.22	2.03	64.41	1.34	86.05	18	2521	0.59	0.102	0.12	19.28	0.13	0.001
54.28	2.07	60.50	1.38	83.25	18	2482	0.59	0.106	0.12	19.28	0.14	0.001
54.35	2.10	56.68	1.43	80.83	18	2439	0.59	0.109	0.13	19.28	0.14	0.001
54.40	2.14	53.17	1.48	78.93	17	2394	0.58	0.114	0.13	19.28	0.15	0.001
54.49	2.17	50.23	1.55	77.99	17	2365	0.58	0.117	0.14	19.28	0.15	0.002
54.54	2.21	47.58	1.65	78.56	18	2361	0.58	0.117	0.13	19.28	0.15	0.001
54.63	2.26	45.28	1.78	80.41	19	2376	0.58	0.116	0.13	19.28	0.14	0.002
54.68	2.30	43.49	1.91	83.01	19	2398	0.58	0.113	0.12	19.28	0.13	0.001
54.73	2.32	42.22	2.01	84.89	20	2409	0.58	0.112	0.11	19.28	0.12	0.001
54.82	2.33	41.67	2.06	85.89	21	2419	0.58	0.111	0.11	19.28	0.12	0.001
54.86	2.33	42.19	2.05	86.51	21	2443	0.58	0.109	0.11	19.28	0.12	0.001
54.95	2.32	43.46	2.00	86.98	21	2484	0.58	0.106	0.10	19.28	0.11	0.001
55.00	2.31	44.78	1.97	88.21	21	2536	0.58	0.101	0.10	19.28	0.11	0.001
55.08	2.32	45.42	1.99	90.48	21	2594	0.58	0.097	0.09	19.28	0.10	0.001
55.23	2.33	45.47	2.04	92.65	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.24	2.34	45.34	2.09	94.77	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.28	2.35	44.90	2.14	96.22	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.33	2.38	43.81	2.28	99.98	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.42	2.42	41.79	2.51	104.81	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.47	2.47	39.27	2.81	110.41	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.52	2.56	34.13	3.46	118.07	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.66	2.64	29.87	4.15	124.00	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.70	2.71	26.59	4.81	127.98	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.73	2.68	27.57	4.48	123.66	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.79	2.62	29.62	3.97	117.65	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.85	2.56	32.49	3.40	110.39	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.93	2.50	35.37	2.95	104.33	0	0	0.58	0.000	0.00	0.00	0.00	0.000
55.98	2.44	38.34	2.58	98.74	0	0	0.58	0.000	0.00	0.00	0.00	0.000
56.12	2.39	40.84	2.33	95.00	0	0	0.58	0.000	0.00	0.00	0.00	0.000
56.21	2.35	43.18	2.15	92.79	0	0	0.57	0.000	0.00	0.00	0.00	0.000
56.25	2.33	45.62	2.05	93.41	0	0	0.57	0.000	0.00	0.00	0.00	0.000
56.35	2.31	48.15	1.98	95.11	23	2803	0.57	0.084	0.07	19.28	0.08	0.001
56.40	2.30	50.34	1.93	97.28	23	2892	0.57	0.080	0.07	19.28	0.08	0.000
56.44	2.30	52.03	1.91	99.45	23	2970	0.57	0.076	0.06	19.28	0.07	0.000
56.52	2.29	53.66	1.89	101.21	24	3041	0.57	0.073	0.06	19.28	0.07	0.001
56.57	2.27	56.01	1.83	102.68	24	3118	0.57	0.070	0.06	19.28	0.06	0.000
56.63	2.25	59.45	1.74	103.57	24	3197	0.57	0.067	0.05	19.28	0.06	0.000
56.73	2.22	62.62	1.67	104.47	24	3266	0.57	0.065	0.05	19.28	0.06	0.001
56.76	2.20	65.73	1.61	105.62	24	3329	0.57	0.063	0.05	19.28	0.06	0.000
56.84	2.18	68.04	1.56	106.45	24	3373	0.57	0.062	0.05	19.28	0.06	0.001
56.90	2.16	70.89	1.51	107.40	24	3416	0.57	0.060	0.05	19.28	0.05	0.000
56.99	2.14	72.87	1.48	107.80	24	3435	0.57	0.060	0.05	19.28	0.05	0.001

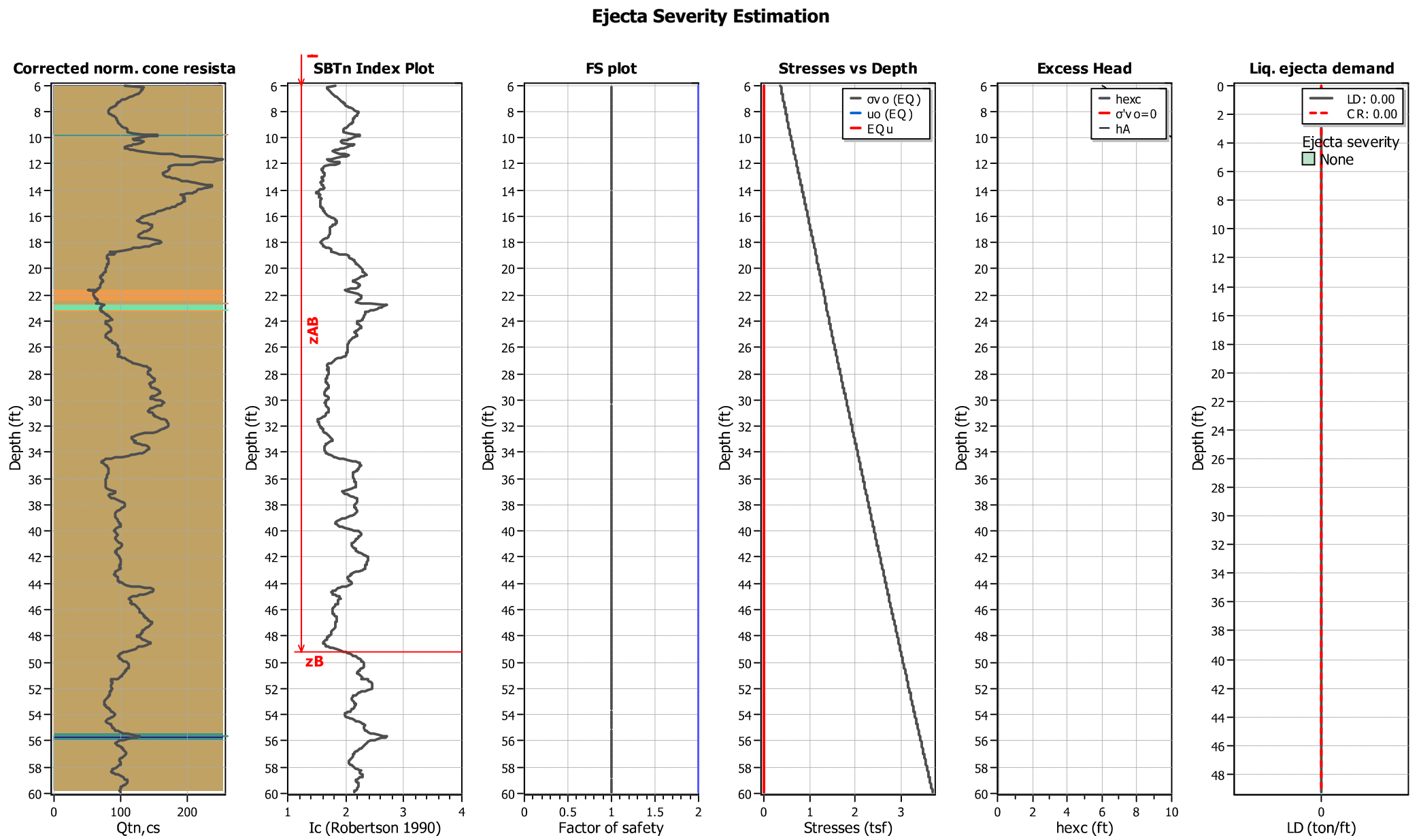
:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
57.03	2.13	73.56	1.46	107.38	23	3422	0.57	0.060	0.05	19.28	0.06	0.000
57.13	2.12	72.92	1.46	106.25	23	3391	0.57	0.061	0.05	19.28	0.06	0.001
57.18	2.12	72.00	1.46	104.98	23	3354	0.57	0.062	0.05	19.28	0.06	0.000
57.22	2.12	71.06	1.45	103.31	23	3302	0.57	0.064	0.06	19.28	0.06	0.000
57.31	2.11	70.71	1.44	101.50	22	3245	0.57	0.066	0.06	19.28	0.07	0.001
57.36	2.09	70.40	1.41	99.24	21	3163	0.57	0.069	0.06	19.28	0.07	0.000
57.44	2.08	70.72	1.39	98.02	21	3113	0.57	0.070	0.07	19.28	0.07	0.001
57.50	2.06	71.09	1.37	97.51	21	3087	0.57	0.071	0.07	19.28	0.08	0.000
57.55	2.06	72.10	1.36	98.36	21	3109	0.57	0.071	0.07	19.28	0.07	0.001
57.63	2.06	73.01	1.36	99.33	21	3140	0.57	0.069	0.07	19.28	0.07	0.001
57.69	2.06	73.43	1.36	99.98	21	3165	0.57	0.069	0.06	19.28	0.07	0.000
57.75	2.06	72.80	1.37	99.79	21	3173	0.57	0.068	0.06	19.28	0.07	0.001
57.82	2.08	71.08	1.39	99.11	21	3176	0.57	0.068	0.06	19.28	0.07	0.001
57.88	2.11	68.54	1.43	98.11	21	3168	0.57	0.069	0.06	19.28	0.07	0.001
57.95	2.13	65.74	1.47	96.47	21	3128	0.57	0.070	0.07	19.28	0.07	0.001
58.01	2.15	62.10	1.51	93.68	21	3043	0.57	0.073	0.07	19.28	0.08	0.001
58.14	2.17	58.85	1.54	90.72	20	2950	0.56	0.078	0.08	19.28	0.09	0.001
58.19	2.20	54.45	1.62	88.34	20	2855	0.56	0.082	0.08	19.28	0.09	0.001
58.25	2.24	50.90	1.72	87.49	20	2794	0.56	0.085	0.09	19.28	0.10	0.001
58.27	2.26	49.00	1.77	86.89	20	2752	0.56	0.088	0.09	19.28	0.10	0.000
58.38	2.25	49.17	1.76	86.79	20	2758	0.56	0.088	0.09	19.28	0.10	0.001
58.43	2.24	50.37	1.73	86.97	20	2783	0.56	0.086	0.09	19.28	0.10	0.001
58.47	2.25	50.88	1.76	89.52	21	2852	0.56	0.082	0.08	19.28	0.09	0.000
58.56	2.26	51.49	1.79	92.41	21	2931	0.56	0.079	0.07	19.28	0.08	0.001
58.61	2.28	52.12	1.86	96.91	23	3042	0.56	0.074	0.06	19.28	0.07	0.000
58.70	2.30	52.49	1.91	100.12	24	3121	0.56	0.071	0.06	19.28	0.07	0.001
58.73	2.29	54.36	1.90	103.21	24	3225	0.56	0.067	0.05	19.28	0.06	0.000
58.81	2.25	59.92	1.75	104.59	24	3360	0.56	0.063	0.05	19.28	0.06	0.001
58.93	2.20	65.78	1.62	106.65	24	3494	0.56	0.059	0.05	19.28	0.05	0.001
58.97	2.17	70.37	1.54	108.70	24	3588	0.56	0.056	0.04	19.28	0.05	0.000
59.01	2.17	71.16	1.54	109.57	24	3621	0.56	0.056	0.04	19.28	0.05	0.000
59.06	2.18	70.41	1.56	110.15	25	3637	0.56	0.055	0.04	19.28	0.05	0.000
59.16	2.19	68.93	1.60	110.25	25	3636	0.56	0.055	0.04	19.28	0.05	0.001
59.20	2.21	66.57	1.64	109.31	25	3590	0.56	0.056	0.04	19.28	0.05	0.000
59.29	2.22	64.72	1.66	107.53	24	3529	0.56	0.058	0.05	19.28	0.05	0.001
59.34	2.22	63.17	1.67	105.52	24	3462	0.56	0.060	0.05	19.28	0.05	0.000
59.39	2.22	62.25	1.67	103.70	24	3407	0.56	0.061	0.05	19.28	0.06	0.000
59.48	2.22	61.62	1.66	102.16	23	3366	0.56	0.063	0.05	19.28	0.06	0.001
59.53	2.21	61.44	1.64	100.89	23	3334	0.56	0.064	0.05	19.28	0.06	0.000
59.59	2.20	61.97	1.61	99.96	23	3318	0.56	0.064	0.06	19.28	0.06	0.000
59.69	2.18	62.82	1.58	99.08	22	3307	0.56	0.064	0.06	19.28	0.06	0.001
59.76	2.17	63.71	1.55	98.55	22	3300	0.56	0.065	0.06	19.28	0.06	0.000
59.80	2.16	64.65	1.52	98.53	22	3306	0.56	0.065	0.06	19.28	0.06	0.000
59.85	2.15	65.72	1.50	98.82	22	3320	0.56	0.064	0.06	19.28	0.06	0.000
59.91	2.14	66.58	1.49	99.10	22	3333	0.56	0.064	0.06	19.28	0.06	0.000

:: Post-earthquake settlement of dry sands :: (continued)												
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
Total estimated settlement: 3.85												

Abbreviations

- Q_{tn}: Equivalent clean sand normalized cone resistance
- K_c: Fines correction factor
- Q_{tn,cs}: Post-liquefaction volumetric strain
- G_{max}: Small strain shear modulus
- CSR: Soil cyclic stress ratio
- γ: Cyclic shear strain
- e_{vol(15)}: Volumetric strain after 15 cycles
- N_c: Equivalent number of cycles
- e_v: Volumetric strain
- Settle.: Calculated settlement



LIQUEFACTION ANALYSIS REPORT

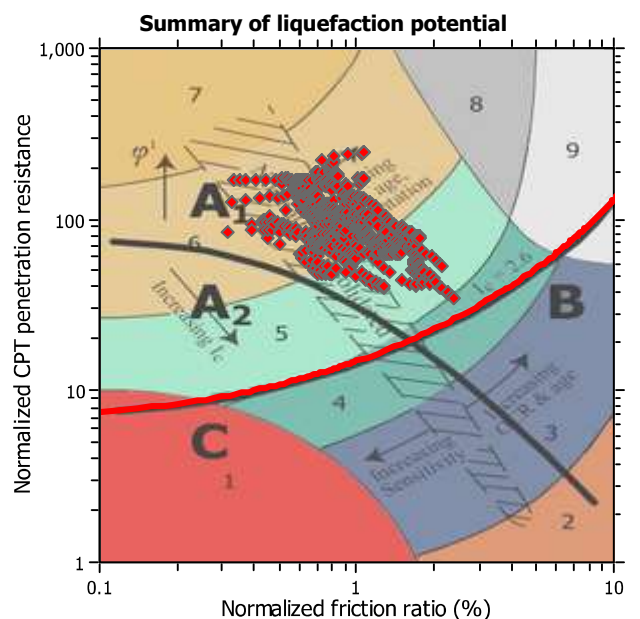
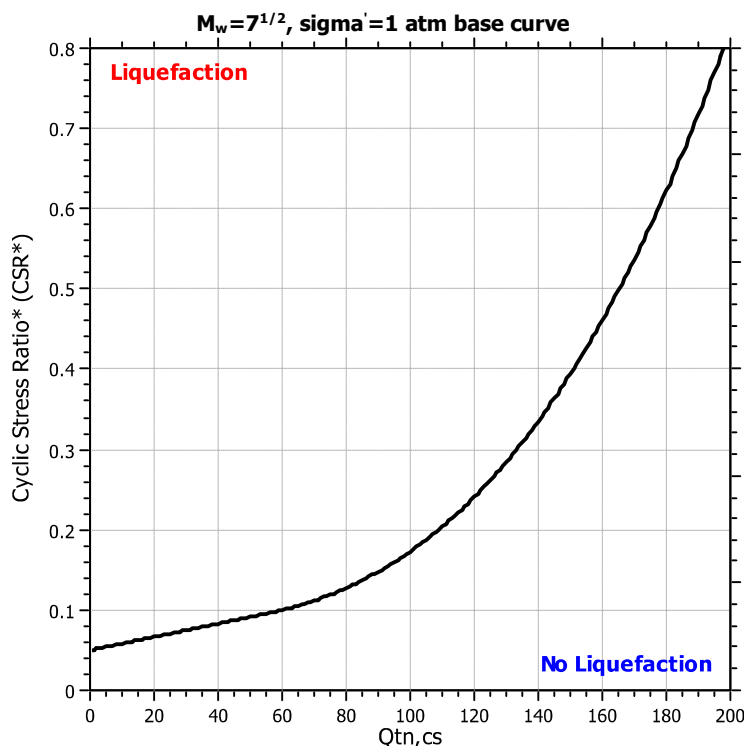
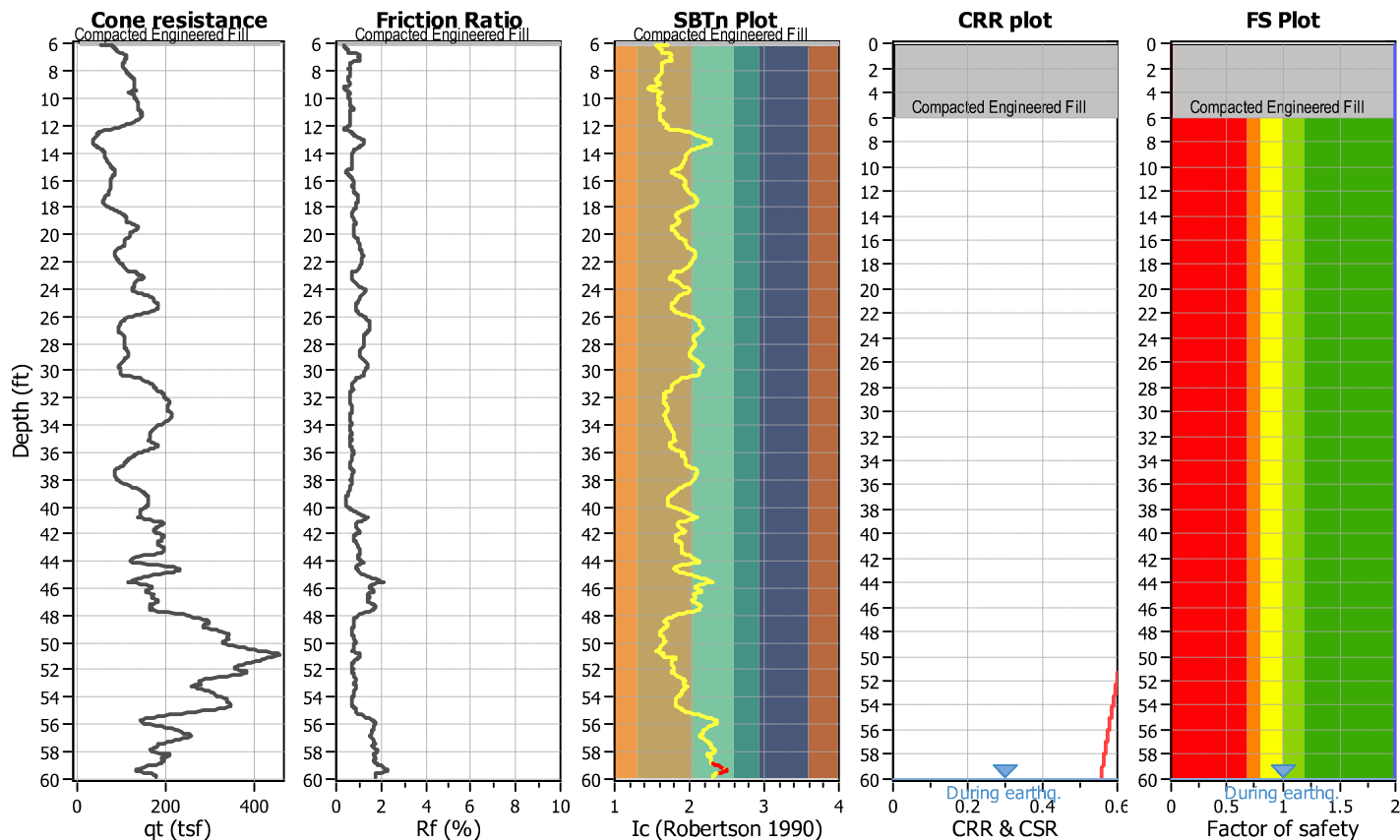
Project title : 4212-CR

Location : Northeast of the 210 Freeway and West San

CPT file : CPT-4

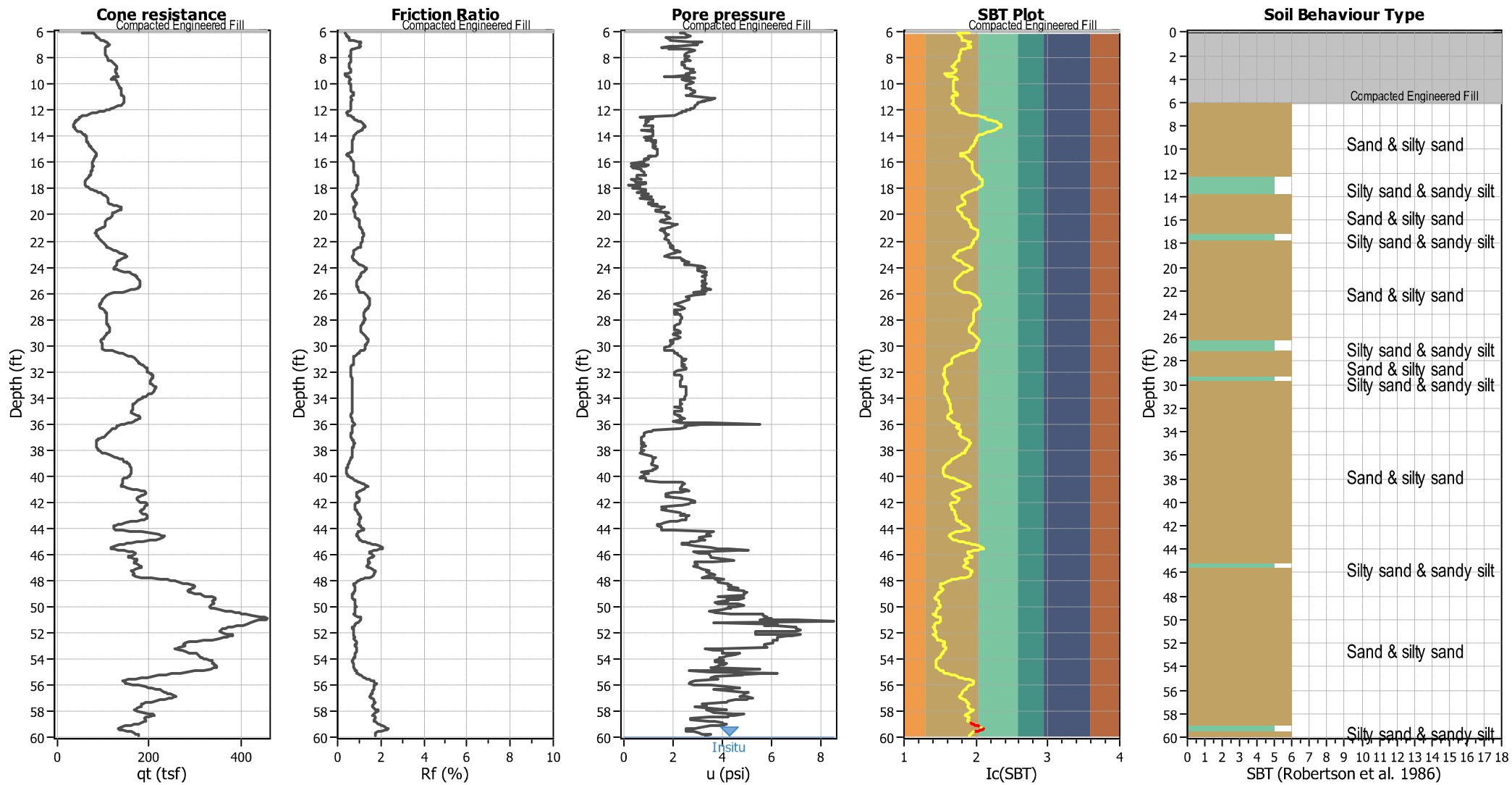
Input parameters and analysis data

Analysis method:	NCEER (1998)	G.W.T. (in-situ):	60.00 ft	Use fill:	No	Clay like behavior	
Fines correction method:	NCEER (1998)	G.W.T. (earthq.):	60.00 ft	Fill height:	N/A	applied:	Sands only
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth applied:	No
Earthquake magnitude M_w :	7.91	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	Limit depth:	N/A
Peak ground acceleration:	0.88	Unit weight calculation:	Based on SBT	K_g applied:	Yes	MSF method:	Method based



Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading
 Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground geometry
 Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening
 Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

CPT basic interpretation plots

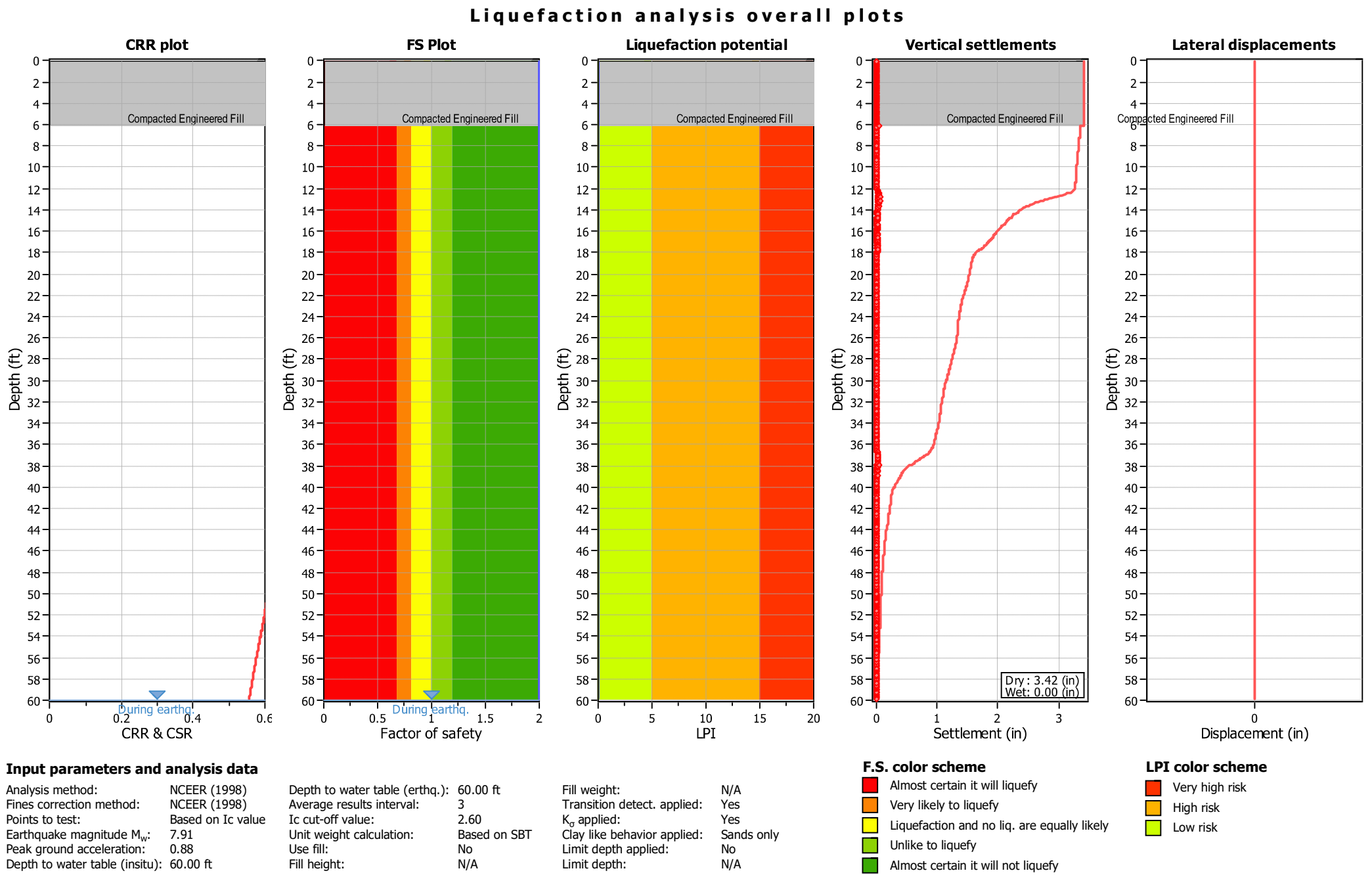


Input parameters and analysis data

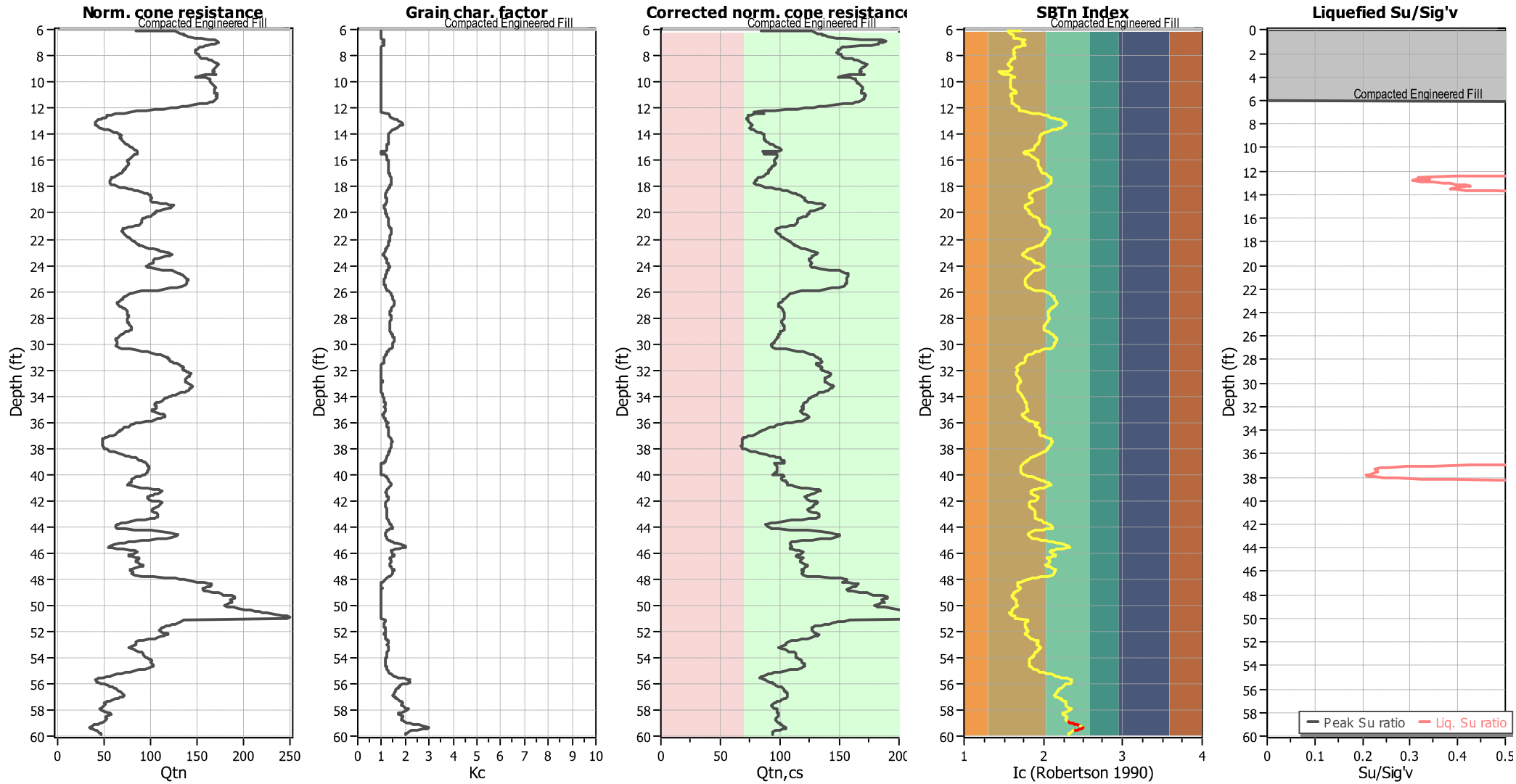
Analysis method:	NCEER (1998)	Depth to water table (erthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K ₀ applied:	Yes
Earthquake magnitude M _w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained



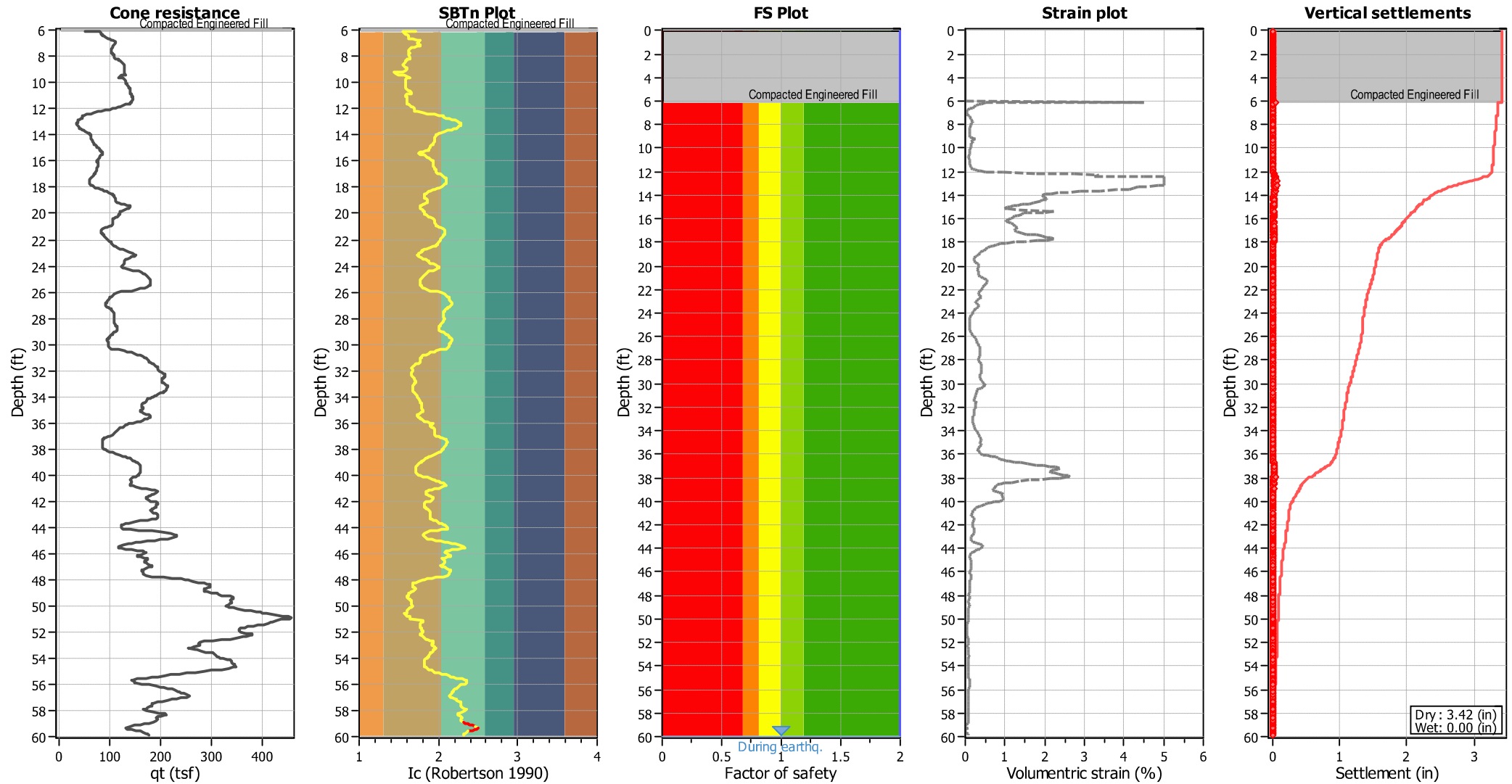
Check for strength loss plots (Robertson (2010))



Input parameters and analysis data

Analysis method:	NCEER (1998)	Depth to water table (earthq.):	60.00 ft	Fill weight:	N/A
Fines correction method:	NCEER (1998)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on I_c value	I_c cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.91	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.88	Use fill:	No	Limit depth applied:	No
Depth to water table (insitu):	60.00 ft	Fill height:	N/A	Limit depth:	N/A

Estimation of post-earthquake settlements



Abbreviations

q_c: Total cone resistance (cone resistance q_c corrected for pore water effects)
 I_c: Soil Behaviour Type Index
 FS: Calculated Factor of Safety against liquefaction
 Volumetric strain: Post-liquefaction volumetric strain

:: Post-earthquake settlement of dry sands ::												
Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
0.09	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.14	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.23	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.28	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.37	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.41	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.50	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.55	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.60	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.69	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.74	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.80	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.87	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
0.92	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.00	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.06	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.12	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.20	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.29	-1.00	-1.00	1.00	-1.00	0	0	0.66	0.000	0.00	0.00	0.00	0.000
1.33	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.43	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.47	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.52	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.60	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.66	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.75	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.79	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.85	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.93	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
1.98	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.04	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.12	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.17	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.26	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.30	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.38	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.44	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.53	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.58	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.67	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.72	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.78	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
2.91	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.01	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.06	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.10	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.19	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	Ic	Q _{tn}	Kc	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
3.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.31	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.36	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.45	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.50	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.58	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.63	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.69	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.77	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
3.91	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.01	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.10	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.14	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.20	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.28	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.42	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.46	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.56	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.60	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.75	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.79	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.82	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.88	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
4.93	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.02	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.06	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.15	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.25	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.29	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.34	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.39	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.48	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.51	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.61	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.66	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.71	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.80	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.85	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.92	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
5.98	-1.00	-1.00	1.00	-1.00	0	0	0.65	0.000	0.00	0.00	0.00	0.000
6.08	1.71	83.62	1.00	83.62	16	406	0.65	2.996	4.03	19.28	4.51	0.051
6.12	1.55	126.96	1.00	126.96	23	509	0.65	0.720	0.62	19.28	0.70	0.004
6.17	1.57	129.71	1.00	129.71	23	535	0.65	0.558	0.47	19.28	0.52	0.003
6.25	1.59	132.20	1.00	132.20	24	562	0.65	0.448	0.36	19.28	0.41	0.004
6.30	1.60	135.67	1.00	135.67	25	590	0.65	0.358	0.28	19.28	0.31	0.002
6.40	1.62	134.92	1.00	134.92	25	605	0.65	0.328	0.26	19.28	0.29	0.003
6.44	1.63	137.80	1.00	137.80	25	623	0.65	0.288	0.22	19.28	0.25	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
6.50	1.63	140.55	1.00	140.55	26	640	0.65	0.262	0.19	19.28	0.22	0.002
6.60	1.62	147.41	1.00	147.41	27	672	0.65	0.217	0.15	19.28	0.17	0.002
6.64	1.68	154.70	1.00	154.70	29	747	0.65	0.143	0.09	19.28	0.10	0.001
6.71	1.73	161.82	1.04	168.67	32	826	0.65	0.102	0.06	19.28	0.07	0.001
6.78	1.76	168.17	1.11	185.94	35	895	0.65	0.080	0.04	19.28	0.05	0.000
6.84	1.76	172.14	1.10	188.65	36	915	0.65	0.076	0.04	19.28	0.04	0.000
6.97	1.75	172.55	1.08	186.51	35	917	0.65	0.078	0.04	19.28	0.04	0.001
7.01	1.74	172.48	1.06	183.25	35	910	0.65	0.081	0.04	19.28	0.05	0.000
7.02	1.75	168.51	1.09	182.94	35	903	0.65	0.084	0.04	19.28	0.05	0.000
7.11	1.76	164.62	1.11	182.09	35	899	0.65	0.087	0.04	19.28	0.05	0.001
7.20	1.76	160.18	1.11	177.26	34	881	0.65	0.095	0.05	19.28	0.06	0.001
7.24	1.74	158.51	1.06	168.18	32	851	0.65	0.108	0.06	19.28	0.07	0.000
7.29	1.69	155.60	1.00	155.60	29	800	0.65	0.135	0.09	19.28	0.10	0.001
7.35	1.65	152.81	1.00	152.81	28	761	0.65	0.167	0.11	19.28	0.12	0.001
7.42	1.63	150.65	1.00	150.65	27	737	0.65	0.194	0.13	19.28	0.15	0.001
7.52	1.63	149.52	1.00	149.52	27	733	0.65	0.204	0.14	19.28	0.16	0.002
7.56	1.63	148.80	1.00	148.80	27	732	0.65	0.208	0.14	19.28	0.16	0.001
7.61	1.63	148.58	1.00	148.58	27	733	0.65	0.210	0.15	19.28	0.16	0.001
7.70	1.63	147.91	1.00	147.91	27	735	0.65	0.213	0.15	19.28	0.17	0.002
7.74	1.63	148.73	1.00	148.73	27	735	0.65	0.215	0.15	19.28	0.17	0.001
7.84	1.62	149.17	1.00	149.17	27	738	0.65	0.218	0.15	19.28	0.17	0.002
7.88	1.63	150.80	1.00	150.80	27	757	0.65	0.199	0.14	19.28	0.15	0.001
8.02	1.63	152.69	1.00	152.69	28	778	0.64	0.185	0.12	19.28	0.14	0.002
8.16	1.64	154.61	1.00	154.61	28	798	0.64	0.174	0.11	19.28	0.13	0.002
8.16	1.62	157.77	1.00	157.77	29	798	0.64	0.173	0.11	19.28	0.13	0.000
8.21	1.61	160.57	1.00	160.57	29	803	0.64	0.172	0.11	19.28	0.12	0.001
8.30	1.60	162.10	1.00	162.10	29	807	0.64	0.173	0.11	19.28	0.12	0.001
8.35	1.59	165.17	1.00	165.17	30	812	0.64	0.171	0.11	19.28	0.12	0.001
8.45	1.59	166.50	1.00	166.50	30	817	0.64	0.171	0.11	19.28	0.12	0.001
8.49	1.57	169.38	1.00	169.38	30	821	0.64	0.169	0.10	19.28	0.12	0.001
8.57	1.56	170.81	1.00	170.81	30	822	0.64	0.172	0.10	19.28	0.12	0.001
8.62	1.55	172.16	1.00	172.16	31	819	0.64	0.177	0.11	19.28	0.12	0.001
8.67	1.54	173.25	1.00	173.25	31	817	0.64	0.181	0.11	19.28	0.12	0.001
8.75	1.56	172.33	1.00	172.33	31	832	0.64	0.172	0.10	19.28	0.12	0.001
8.85	1.58	170.58	1.00	170.58	31	853	0.64	0.160	0.10	19.28	0.11	0.001
8.86	1.61	169.41	1.00	169.41	31	876	0.64	0.145	0.09	19.28	0.10	0.000
8.95	1.61	168.34	1.00	168.34	30	880	0.64	0.146	0.09	19.28	0.10	0.001
9.04	1.59	167.61	1.00	167.61	30	855	0.64	0.166	0.10	19.28	0.11	0.001
9.13	1.54	167.49	1.00	167.49	30	808	0.64	0.212	0.13	19.28	0.15	0.002
9.16	1.47	167.94	1.00	167.94	29	746	0.64	0.298	0.19	19.28	0.21	0.001
9.22	1.45	167.96	1.00	167.96	29	723	0.64	0.350	0.23	19.28	0.25	0.002
9.27	1.46	167.45	1.00	167.45	29	735	0.64	0.329	0.21	19.28	0.24	0.001
9.36	1.49	168.58	1.00	168.58	29	773	0.64	0.269	0.17	19.28	0.19	0.002
9.40	1.51	170.30	1.00	170.30	30	803	0.64	0.231	0.14	19.28	0.16	0.001
9.45	1.57	158.79	1.00	158.79	28	810	0.64	0.225	0.15	19.28	0.17	0.001
9.55	1.61	152.33	1.00	152.33	28	823	0.64	0.217	0.15	19.28	0.17	0.002
9.59	1.64	148.56	1.00	148.56	27	836	0.64	0.205	0.14	19.28	0.16	0.001
9.65	1.60	158.97	1.00	158.97	29	852	0.64	0.193	0.13	19.28	0.14	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
9.72	1.59	162.36	1.00	162.36	29	857	0.64	0.191	0.12	19.28	0.14	0.001
9.79	1.58	163.93	1.00	163.93	29	859	0.64	0.193	0.12	19.28	0.14	0.001
9.88	1.59	163.80	1.00	163.80	29	869	0.64	0.189	0.12	19.28	0.13	0.001
9.92	1.59	164.20	1.00	164.20	30	878	0.64	0.183	0.11	19.28	0.13	0.001
10.01	1.59	164.32	1.00	164.32	30	886	0.64	0.180	0.11	19.28	0.13	0.001
10.06	1.59	165.03	1.00	165.03	30	894	0.64	0.176	0.11	19.28	0.12	0.001
10.11	1.59	166.05	1.00	166.05	30	901	0.64	0.173	0.11	19.28	0.12	0.001
10.20	1.59	166.51	1.00	166.51	30	907	0.64	0.171	0.11	19.28	0.12	0.001
10.24	1.59	168.00	1.00	168.00	30	914	0.64	0.168	0.10	19.28	0.11	0.001
10.34	1.59	168.73	1.00	168.73	30	920	0.64	0.167	0.10	19.28	0.11	0.001
10.39	1.59	169.64	1.00	169.64	30	925	0.64	0.166	0.10	19.28	0.11	0.001
10.43	1.59	169.95	1.00	169.95	31	933	0.64	0.162	0.10	19.28	0.11	0.001
10.56	1.60	169.20	1.00	169.20	31	950	0.64	0.156	0.09	19.28	0.10	0.002
10.62	1.62	168.79	1.00	168.79	31	974	0.64	0.144	0.09	19.28	0.10	0.001
10.66	1.64	168.53	1.00	168.53	31	1000	0.64	0.133	0.08	19.28	0.09	0.000
10.71	1.66	168.42	1.00	168.42	31	1017	0.64	0.127	0.08	19.28	0.08	0.000
10.80	1.66	168.10	1.00	168.10	31	1020	0.64	0.128	0.08	19.28	0.08	0.001
10.84	1.64	169.01	1.00	169.01	31	1006	0.64	0.135	0.08	19.28	0.09	0.000
10.89	1.61	170.86	1.00	170.86	31	984	0.64	0.147	0.09	19.28	0.10	0.001
10.99	1.59	171.55	1.00	171.55	31	971	0.64	0.156	0.09	19.28	0.10	0.001
11.03	1.59	171.78	1.00	171.78	31	975	0.64	0.156	0.09	19.28	0.10	0.001
11.12	1.60	170.64	1.00	170.64	31	984	0.64	0.153	0.09	19.28	0.10	0.001
11.16	1.61	170.93	1.00	170.93	31	993	0.64	0.150	0.09	19.28	0.10	0.000
11.26	1.61	170.29	1.00	170.29	31	994	0.64	0.152	0.09	19.28	0.10	0.001
11.31	1.60	170.01	1.00	170.01	31	990	0.64	0.155	0.09	19.28	0.10	0.001
11.36	1.60	168.39	1.00	168.39	30	980	0.64	0.162	0.10	19.28	0.11	0.001
11.47	1.60	166.11	1.00	166.11	30	973	0.64	0.170	0.10	19.28	0.12	0.001
11.49	1.61	163.76	1.00	163.76	30	963	0.64	0.177	0.11	19.28	0.12	0.000
11.57	1.61	160.96	1.00	160.96	29	955	0.64	0.185	0.12	19.28	0.13	0.001
11.63	1.63	153.50	1.00	153.50	28	934	0.64	0.204	0.14	19.28	0.15	0.001
11.77	1.65	145.56	1.00	145.56	27	912	0.64	0.229	0.16	19.28	0.18	0.003
11.81	1.67	138.30	1.00	138.30	26	888	0.64	0.258	0.19	19.28	0.22	0.001
11.85	1.68	133.53	1.00	133.53	25	868	0.64	0.284	0.22	19.28	0.25	0.001
11.91	1.69	128.61	1.00	128.61	24	848	0.64	0.319	0.26	19.28	0.29	0.002
11.95	1.70	121.33	1.00	121.33	23	817	0.64	0.379	0.33	19.28	0.37	0.002
12.04	1.70	114.54	1.00	114.54	21	770	0.64	0.509	0.47	19.28	0.53	0.006
12.09	1.70	104.58	1.00	104.58	19	702	0.64	0.823	0.85	19.28	0.95	0.006
12.18	1.70	95.14	1.00	95.14	18	642	0.64	1.378	1.59	19.28	1.78	0.018
12.23	1.74	86.08	1.00	86.08	16	610	0.64	1.903	2.44	19.28	2.73	0.017
12.28	1.80	78.37	1.00	78.37	15	599	0.64	2.164	3.02	19.28	3.38	0.018
12.37	1.86	72.11	1.21	86.96	17	590	0.64	2.432	2.92	19.28	3.27	0.035
12.42	1.95	61.99	1.27	78.89	16	569	0.64	3.117	4.03	19.28	4.52	0.029
12.47	2.01	57.32	1.31	75.29	16	561	0.64	3.463	4.63	19.28	5.00	0.029
12.55	2.06	53.21	1.36	72.62	15	555	0.64	3.802	5.18	19.28	5.00	0.052
12.61	2.06	53.78	1.37	73.52	16	563	0.64	3.470	4.65	19.28	5.00	0.032
12.69	2.10	51.35	1.41	72.58	16	563	0.64	3.560	4.76	19.28	5.00	0.052
12.74	2.14	48.72	1.48	72.15	16	562	0.64	3.646	4.82	19.28	5.00	0.027
12.83	2.17	46.45	1.55	72.15	16	561	0.64	3.753	4.87	19.28	5.00	0.055

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
12.88	2.21	44.15	1.65	72.92	17	561	0.64	3.818	4.79	19.28	5.00	0.028
12.97	2.24	42.48	1.74	73.72	17	561	0.64	3.890	4.75	19.28	5.00	0.053
13.02	2.27	41.09	1.81	74.52	17	560	0.64	3.990	4.75	19.28	5.00	0.029
13.06	2.28	40.50	1.85	74.96	17	560	0.64	4.017	4.72	19.28	5.00	0.026
13.15	2.29	40.19	1.87	75.34	18	563	0.64	3.975	4.63	19.28	5.00	0.056
13.19	2.29	40.36	1.88	76.00	18	568	0.64	3.773	4.34	19.28	4.86	0.024
13.28	2.28	40.85	1.87	76.27	18	575	0.64	3.571	4.10	19.28	4.59	0.048
13.34	2.26	42.23	1.80	75.94	18	582	0.64	3.333	3.89	19.28	4.35	0.030
13.43	2.24	43.63	1.73	75.31	17	587	0.64	3.188	3.80	19.28	4.25	0.046
13.47	2.22	45.11	1.66	74.94	17	592	0.64	3.046	3.69	19.28	4.14	0.022
13.52	2.19	47.06	1.60	75.23	17	602	0.64	2.782	3.40	19.28	3.81	0.021
13.61	2.16	49.69	1.53	75.94	17	614	0.64	2.477	3.04	19.28	3.40	0.036
13.66	2.11	54.02	1.44	77.64	17	631	0.64	2.120	2.60	19.28	2.91	0.019
13.75	2.07	57.57	1.38	79.46	17	643	0.64	1.916	2.33	19.28	2.61	0.028
13.80	2.03	61.27	1.34	81.87	17	654	0.64	1.744	2.08	19.28	2.33	0.012
13.85	2.01	63.75	1.31	83.73	17	663	0.64	1.637	1.92	19.28	2.15	0.013
13.93	1.99	65.85	1.30	85.40	18	670	0.64	1.552	1.80	19.28	2.01	0.020
13.98	1.97	67.20	1.29	86.46	18	675	0.64	1.512	1.74	19.28	1.94	0.012
14.07	1.97	67.68	1.28	86.73	18	676	0.64	1.520	1.75	19.28	1.95	0.022
14.12	1.96	67.97	1.28	86.81	18	676	0.64	1.542	1.77	19.28	1.99	0.010
14.21	1.96	67.73	1.28	86.49	18	675	0.64	1.573	1.82	19.28	2.04	0.023
14.24	1.96	67.79	1.28	86.51	18	676	0.64	1.577	1.82	19.28	2.04	0.007
14.34	1.96	67.87	1.28	86.55	18	678	0.64	1.573	1.82	19.28	2.04	0.025
14.43	1.95	68.19	1.27	86.81	18	681	0.64	1.560	1.80	19.28	2.01	0.021
14.44	1.95	68.89	1.27	87.45	18	684	0.64	1.525	1.75	19.28	1.96	0.003
14.52	1.94	69.66	1.27	88.17	18	690	0.64	1.480	1.68	19.28	1.88	0.017
14.58	1.93	71.34	1.26	89.83	18	700	0.64	1.379	1.54	19.28	1.73	0.014
14.71	1.93	72.78	1.25	91.23	18	710	0.64	1.301	1.43	19.28	1.60	0.025
14.73	1.92	74.36	1.25	92.76	19	718	0.64	1.225	1.33	19.28	1.49	0.003
14.77	1.91	75.36	1.24	93.74	19	724	0.64	1.179	1.26	19.28	1.42	0.006
14.85	1.90	76.54	1.24	94.90	19	732	0.64	1.124	1.19	19.28	1.33	0.013
14.90	1.90	78.22	1.24	96.60	19	743	0.64	1.046	1.09	19.28	1.22	0.009
14.99	1.89	79.58	1.23	97.92	20	752	0.64	0.993	1.02	19.28	1.14	0.012
15.04	1.88	81.35	1.22	99.59	20	762	0.64	0.935	0.95	19.28	1.06	0.006
15.12	1.88	82.74	1.22	100.95	20	772	0.63	0.886	0.88	19.28	0.99	0.010
15.18	1.85	84.25	1.20	101.14	20	763	0.63	0.950	0.96	19.28	1.07	0.008
15.24	1.81	85.22	1.16	99.16	19	737	0.63	1.163	1.22	19.28	1.36	0.010
15.33	1.77	85.68	1.00	85.68	16	706	0.63	1.499	1.91	19.28	2.13	0.023
15.37	1.76	86.19	1.00	86.19	16	702	0.63	1.564	1.98	19.28	2.22	0.009
15.45	1.77	86.27	1.00	86.27	17	717	0.63	1.404	1.77	19.28	1.98	0.021
15.51	1.80	85.05	1.15	97.59	19	729	0.63	1.293	1.39	19.28	1.56	0.010
15.55	1.82	82.95	1.18	97.59	19	736	0.63	1.231	1.31	19.28	1.46	0.008
15.64	1.85	81.00	1.20	97.01	19	743	0.63	1.193	1.26	19.28	1.41	0.015
15.69	1.86	80.13	1.21	97.06	19	752	0.63	1.126	1.18	19.28	1.32	0.007
15.78	1.88	79.49	1.22	97.04	19	760	0.63	1.076	1.12	19.28	1.26	0.014
15.82	1.90	78.12	1.23	96.47	19	767	0.63	1.031	1.08	19.28	1.20	0.007
15.92	1.92	76.50	1.25	95.43	19	771	0.63	1.016	1.06	19.28	1.19	0.013
15.96	1.93	75.30	1.26	94.68	19	775	0.63	1.000	1.05	19.28	1.18	0.007

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
16.02	1.94	74.95	1.26	94.60	19	780	0.63	0.975	1.02	19.28	1.14	0.008
16.10	1.94	75.36	1.26	95.26	19	789	0.63	0.928	0.96	19.28	1.08	0.010
16.15	1.94	76.06	1.26	96.08	20	797	0.63	0.891	0.92	19.28	1.03	0.006
16.24	1.94	76.29	1.26	96.20	20	798	0.63	0.893	0.92	19.28	1.03	0.011
16.28	1.93	75.98	1.26	95.70	19	794	0.63	0.925	0.96	19.28	1.07	0.006
16.38	1.93	75.29	1.26	94.82	19	789	0.63	0.970	1.02	19.28	1.14	0.013
16.42	1.94	74.60	1.26	94.10	19	786	0.63	0.996	1.05	19.28	1.18	0.006
16.47	1.94	73.64	1.27	93.25	19	785	0.63	1.016	1.08	19.28	1.21	0.007
16.57	1.95	72.54	1.27	92.28	19	784	0.63	1.037	1.11	19.28	1.24	0.014
16.62	1.96	71.68	1.28	91.56	19	783	0.63	1.050	1.13	19.28	1.27	0.008
16.69	1.96	71.26	1.28	91.14	19	783	0.63	1.065	1.15	19.28	1.29	0.011
16.77	1.97	70.85	1.28	90.78	19	784	0.63	1.072	1.17	19.28	1.30	0.013
16.81	1.98	70.21	1.29	90.46	19	787	0.63	1.052	1.14	19.28	1.28	0.006
16.90	1.99	69.00	1.30	89.85	19	794	0.63	1.024	1.11	19.28	1.25	0.013
16.99	2.01	67.70	1.32	89.13	19	798	0.63	1.006	1.10	19.28	1.23	0.014
17.00	2.03	66.50	1.33	88.42	19	799	0.63	1.006	1.10	19.28	1.23	0.002
17.09	2.04	65.06	1.34	87.41	18	798	0.63	1.027	1.13	19.28	1.27	0.013
17.14	2.06	63.09	1.36	86.01	18	793	0.63	1.070	1.19	19.28	1.33	0.008
17.23	2.07	61.09	1.38	84.49	18	786	0.63	1.136	1.28	19.28	1.43	0.016
17.30	2.09	59.50	1.40	83.23	18	779	0.63	1.204	1.37	19.28	1.54	0.013
17.32	2.09	58.48	1.41	82.22	18	772	0.63	1.276	1.47	19.28	1.65	0.005
17.41	2.10	57.41	1.41	81.09	18	764	0.63	1.364	1.60	19.28	1.79	0.018
17.51	2.10	56.34	1.42	79.95	17	757	0.63	1.463	1.74	19.28	1.95	0.023
17.55	2.10	55.68	1.42	79.28	17	752	0.63	1.525	1.83	19.28	2.05	0.011
17.59	2.10	55.40	1.42	78.75	17	748	0.63	1.586	1.92	19.28	2.15	0.009
17.69	2.10	55.46	1.41	78.39	17	746	0.63	1.632	1.99	19.28	2.23	0.028
17.73	2.09	56.14	1.40	78.55	17	747	0.63	1.635	2.00	19.28	2.24	0.012
17.79	2.07	58.05	1.38	79.97	17	758	0.63	1.520	1.83	19.28	2.05	0.015
17.88	2.05	60.21	1.36	81.83	17	773	0.63	1.379	1.63	19.28	1.83	0.018
17.92	2.03	63.46	1.34	84.90	18	795	0.63	1.184	1.35	19.28	1.51	0.008
18.01	2.01	66.99	1.32	88.28	18	819	0.63	1.021	1.12	19.28	1.26	0.013
18.06	1.98	71.52	1.29	92.60	19	846	0.63	0.872	0.92	19.28	1.03	0.007
18.12	1.96	75.68	1.27	96.44	20	866	0.63	0.784	0.80	19.28	0.89	0.007
18.20	1.93	79.37	1.26	99.73	20	880	0.63	0.731	0.72	19.28	0.81	0.007
18.24	1.90	82.78	1.24	102.61	21	889	0.63	0.698	0.67	19.28	0.75	0.004
18.32	1.88	86.28	1.22	105.50	21	900	0.63	0.664	0.63	19.28	0.70	0.006
18.38	1.85	90.40	1.20	108.79	21	915	0.63	0.621	0.57	19.28	0.64	0.005
18.47	1.84	93.68	1.19	111.31	22	928	0.63	0.586	0.53	19.28	0.59	0.006
18.52	1.82	96.56	1.18	113.57	22	943	0.63	0.549	0.49	19.28	0.54	0.003
18.61	1.82	97.74	1.17	114.80	22	955	0.63	0.525	0.46	19.28	0.51	0.005
18.65	1.82	99.12	1.17	116.26	23	967	0.64	0.497	0.43	19.28	0.48	0.003
18.70	1.82	100.02	1.18	117.88	23	985	0.64	0.462	0.39	19.28	0.44	0.003
18.79	1.83	100.59	1.19	119.24	23	1003	0.64	0.432	0.36	19.28	0.40	0.004
18.84	1.84	100.69	1.20	120.45	24	1021	0.64	0.401	0.33	19.28	0.37	0.002
18.93	1.86	100.20	1.20	120.74	24	1034	0.64	0.386	0.31	19.28	0.35	0.004
19.03	1.86	99.84	1.21	121.00	24	1045	0.64	0.373	0.30	19.28	0.34	0.004
19.07	1.87	100.47	1.21	121.84	24	1054	0.64	0.362	0.29	19.28	0.32	0.002
19.12	1.86	101.83	1.21	123.22	24	1065	0.64	0.349	0.28	19.28	0.31	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
19.16	1.85	105.90	1.20	126.88	25	1088	0.64	0.322	0.25	19.28	0.28	0.001
19.26	1.83	110.19	1.18	130.51	26	1112	0.64	0.298	0.22	19.28	0.25	0.003
19.30	1.80	116.60	1.16	135.00	26	1139	0.64	0.274	0.20	19.28	0.22	0.001
19.40	1.78	120.79	1.13	136.94	26	1152	0.64	0.265	0.19	19.28	0.21	0.003
19.44	1.77	124.10	1.11	137.57	26	1157	0.64	0.262	0.19	19.28	0.21	0.001
19.51	1.76	123.88	1.10	136.71	26	1152	0.64	0.268	0.20	19.28	0.22	0.002
19.59	1.77	122.30	1.11	135.69	26	1146	0.64	0.276	0.20	19.28	0.23	0.002
19.62	1.77	119.83	1.12	134.37	26	1136	0.64	0.286	0.21	19.28	0.24	0.001
19.70	1.80	114.30	1.15	131.34	25	1118	0.64	0.308	0.23	19.28	0.26	0.002
19.78	1.81	109.84	1.17	128.18	25	1100	0.64	0.331	0.25	19.28	0.28	0.003
19.82	1.83	106.17	1.18	125.25	24	1082	0.64	0.355	0.28	19.28	0.31	0.001
19.91	1.82	105.95	1.18	124.79	24	1080	0.64	0.361	0.29	19.28	0.32	0.003
19.97	1.83	105.22	1.18	124.12	24	1077	0.64	0.368	0.29	19.28	0.33	0.003
20.02	1.83	104.65	1.18	123.98	24	1080	0.64	0.365	0.29	19.28	0.32	0.002
20.11	1.84	103.51	1.19	123.53	24	1085	0.64	0.362	0.29	19.28	0.32	0.004
20.19	1.86	101.73	1.21	122.82	24	1093	0.64	0.355	0.28	19.28	0.32	0.003
20.25	1.88	99.34	1.22	121.48	24	1099	0.64	0.349	0.28	19.28	0.31	0.002
20.29	1.90	96.58	1.24	119.67	24	1104	0.64	0.345	0.28	19.28	0.31	0.002
20.38	1.92	93.74	1.25	117.45	24	1104	0.64	0.348	0.28	19.28	0.32	0.003
20.43	1.94	91.75	1.26	115.80	24	1102	0.64	0.353	0.29	19.28	0.32	0.002
20.48	1.94	90.68	1.27	114.75	23	1098	0.64	0.360	0.30	19.28	0.33	0.002
20.57	1.95	90.20	1.27	114.28	23	1098	0.64	0.363	0.30	19.28	0.34	0.003
20.61	1.95	90.00	1.27	114.33	23	1104	0.64	0.357	0.30	19.28	0.33	0.002
20.71	1.96	89.86	1.27	114.48	23	1114	0.65	0.349	0.29	19.28	0.32	0.004
20.75	1.96	89.72	1.28	114.56	23	1119	0.65	0.343	0.28	19.28	0.32	0.002
20.84	1.96	88.81	1.28	113.62	23	1116	0.65	0.350	0.29	19.28	0.33	0.004
20.90	1.97	86.88	1.28	111.61	23	1104	0.65	0.368	0.31	19.28	0.35	0.002
20.97	1.99	83.78	1.30	108.53	22	1088	0.65	0.395	0.34	19.28	0.38	0.003
21.03	2.01	80.01	1.31	104.96	22	1069	0.65	0.428	0.38	19.28	0.43	0.003
21.08	2.03	76.10	1.33	101.51	21	1051	0.65	0.462	0.43	19.28	0.48	0.003
21.17	2.05	72.72	1.36	98.76	21	1038	0.65	0.492	0.46	19.28	0.52	0.005
21.22	2.07	70.47	1.38	97.02	21	1029	0.65	0.515	0.49	19.28	0.55	0.003
21.31	2.08	69.58	1.39	96.40	21	1028	0.65	0.522	0.50	19.28	0.56	0.006
21.35	2.08	69.26	1.39	96.33	21	1030	0.65	0.519	0.50	19.28	0.56	0.003
21.40	2.08	69.42	1.39	96.79	21	1038	0.65	0.506	0.48	19.28	0.54	0.003
21.48	2.09	69.42	1.40	97.09	21	1045	0.65	0.495	0.47	19.28	0.53	0.005
21.54	2.08	70.26	1.39	97.93	21	1054	0.65	0.479	0.45	19.28	0.50	0.003
21.62	2.08	71.10	1.39	98.63	21	1062	0.65	0.468	0.44	19.28	0.49	0.005
21.66	2.07	72.31	1.38	99.52	21	1069	0.65	0.457	0.42	19.28	0.47	0.002
21.72	2.06	73.35	1.37	100.21	21	1074	0.65	0.450	0.42	19.28	0.47	0.003
21.80	2.05	74.57	1.36	101.06	21	1081	0.65	0.442	0.41	19.28	0.46	0.004
21.86	2.04	76.11	1.34	102.34	22	1091	0.65	0.429	0.39	19.28	0.44	0.003
22.00	2.03	78.00	1.33	103.97	22	1105	0.65	0.412	0.37	19.28	0.41	0.007
22.09	2.02	79.84	1.32	105.67	22	1119	0.65	0.395	0.35	19.28	0.39	0.004
22.13	2.01	81.40	1.32	107.14	22	1130	0.65	0.381	0.33	19.28	0.37	0.002
22.18	2.00	83.14	1.31	108.78	23	1142	0.65	0.367	0.32	19.28	0.35	0.002
22.29	1.99	84.86	1.30	110.30	23	1153	0.65	0.357	0.30	19.28	0.34	0.005
22.38	1.98	86.37	1.29	111.58	23	1160	0.65	0.351	0.30	19.28	0.33	0.003

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
22.41	1.98	86.94	1.29	112.00	23	1161	0.65	0.351	0.30	19.28	0.33	0.001
22.45	1.97	87.52	1.28	112.42	23	1162	0.65	0.351	0.29	19.28	0.33	0.002
22.51	1.93	90.39	1.26	113.74	23	1143	0.65	0.378	0.32	19.28	0.36	0.002
22.64	1.89	93.80	1.23	115.44	23	1125	0.65	0.408	0.34	19.28	0.39	0.006
22.68	1.84	99.01	1.19	118.01	23	1110	0.65	0.433	0.36	19.28	0.41	0.002
22.73	1.82	102.76	1.17	120.63	23	1124	0.65	0.414	0.34	19.28	0.38	0.003
22.78	1.79	108.44	1.14	124.15	24	1145	0.65	0.385	0.31	19.28	0.35	0.002
22.87	1.80	107.73	1.15	124.37	24	1153	0.65	0.377	0.30	19.28	0.34	0.004
22.91	1.79	111.11	1.14	127.15	25	1176	0.65	0.350	0.27	19.28	0.31	0.001
22.99	1.79	112.23	1.14	128.38	25	1190	0.65	0.337	0.26	19.28	0.29	0.003
23.04	1.76	119.61	1.10	131.25	25	1213	0.65	0.315	0.24	19.28	0.27	0.002
23.14	1.74	121.77	1.07	130.79	25	1214	0.66	0.316	0.24	19.28	0.27	0.003
23.18	1.73	122.62	1.06	129.52	24	1207	0.66	0.324	0.25	19.28	0.29	0.002
23.27	1.74	119.78	1.07	128.28	24	1195	0.66	0.340	0.27	19.28	0.30	0.003
23.32	1.75	116.48	1.09	127.22	24	1183	0.66	0.355	0.28	19.28	0.32	0.002
23.36	1.78	112.08	1.13	126.71	24	1182	0.66	0.358	0.28	19.28	0.32	0.002
23.45	1.81	108.38	1.16	125.88	24	1187	0.66	0.355	0.28	19.28	0.31	0.004
23.50	1.84	104.91	1.19	124.88	24	1197	0.66	0.345	0.27	19.28	0.30	0.002
23.60	1.86	103.09	1.21	124.30	25	1210	0.66	0.334	0.26	19.28	0.29	0.003
23.64	1.87	102.47	1.22	124.70	25	1228	0.66	0.317	0.25	19.28	0.27	0.001
23.69	1.89	102.46	1.23	125.78	25	1255	0.66	0.294	0.22	19.28	0.25	0.002
23.78	1.90	102.33	1.24	126.79	25	1285	0.66	0.271	0.20	19.28	0.23	0.002
23.82	1.93	100.84	1.26	126.83	26	1317	0.66	0.249	0.18	19.28	0.21	0.001
23.91	1.97	98.20	1.28	125.82	26	1348	0.66	0.232	0.17	19.28	0.19	0.002
24.00	1.99	96.64	1.30	125.77	26	1378	0.66	0.216	0.16	19.28	0.18	0.002
24.08	2.01	95.68	1.31	125.77	26	1397	0.66	0.208	0.15	19.28	0.17	0.002
24.11	2.01	96.48	1.31	126.68	26	1406	0.66	0.204	0.15	19.28	0.16	0.001
24.15	1.99	99.32	1.30	129.16	27	1420	0.66	0.199	0.14	19.28	0.16	0.001
24.24	1.96	105.46	1.28	134.71	28	1449	0.66	0.188	0.13	19.28	0.14	0.002
24.33	1.93	112.79	1.25	141.48	29	1482	0.66	0.176	0.11	19.28	0.13	0.001
24.39	1.90	118.04	1.24	146.30	29	1506	0.66	0.168	0.11	19.28	0.12	0.001
24.42	1.88	122.65	1.22	150.23	30	1521	0.66	0.164	0.10	19.28	0.11	0.000
24.52	1.87	125.98	1.21	152.79	30	1531	0.66	0.162	0.10	19.28	0.11	0.001
24.56	1.85	129.61	1.20	155.45	31	1540	0.66	0.159	0.10	19.28	0.11	0.001
24.61	1.84	131.67	1.19	156.84	31	1546	0.66	0.158	0.09	19.28	0.11	0.001
24.68	1.83	133.15	1.18	157.71	31	1550	0.66	0.158	0.09	19.28	0.11	0.001
24.74	1.82	134.14	1.17	157.59	31	1542	0.66	0.161	0.10	19.28	0.11	0.001
24.88	1.81	135.46	1.16	156.99	30	1530	0.66	0.167	0.10	19.28	0.11	0.002
24.98	1.79	137.35	1.14	156.21	30	1515	0.66	0.173	0.11	19.28	0.12	0.001
25.02	1.77	139.11	1.12	155.90	30	1510	0.66	0.175	0.11	19.28	0.12	0.001
25.07	1.77	139.76	1.12	156.07	30	1513	0.66	0.175	0.11	19.28	0.12	0.001
25.16	1.77	139.60	1.12	156.25	30	1518	0.66	0.174	0.11	19.28	0.12	0.001
25.21	1.77	139.48	1.12	156.46	30	1522	0.66	0.173	0.11	19.28	0.12	0.001
25.29	1.77	139.42	1.12	156.18	30	1522	0.66	0.174	0.11	19.28	0.12	0.001
25.35	1.77	139.26	1.12	155.67	30	1518	0.66	0.176	0.11	19.28	0.12	0.001
25.40	1.77	138.99	1.12	155.32	30	1517	0.66	0.177	0.11	19.28	0.12	0.001
25.48	1.78	137.93	1.12	154.91	30	1517	0.66	0.179	0.11	19.28	0.12	0.001
25.53	1.78	136.58	1.13	154.77	30	1519	0.66	0.178	0.11	19.28	0.12	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
25.62	1.79	133.91	1.14	153.07	29	1509	0.66	0.183	0.11	19.28	0.13	0.001
25.67	1.81	128.49	1.16	149.11	29	1481	0.66	0.195	0.13	19.28	0.14	0.001
25.76	1.83	122.24	1.18	144.19	28	1449	0.66	0.210	0.14	19.28	0.16	0.002
25.81	1.87	113.72	1.21	137.86	27	1424	0.66	0.223	0.15	19.28	0.17	0.001
25.90	1.90	107.28	1.24	132.77	27	1415	0.66	0.230	0.16	19.28	0.18	0.002
25.94	1.94	100.03	1.27	126.57	26	1399	0.66	0.239	0.18	19.28	0.20	0.001
25.99	2.00	90.96	1.30	118.66	25	1373	0.66	0.257	0.20	19.28	0.22	0.001
26.06	2.04	84.09	1.34	113.02	24	1349	0.66	0.274	0.22	19.28	0.25	0.002
26.19	2.07	78.88	1.38	109.17	23	1332	0.67	0.290	0.24	19.28	0.27	0.004
26.20	2.08	77.95	1.39	108.52	23	1328	0.67	0.293	0.24	19.28	0.27	0.000
26.30	2.10	75.46	1.42	107.11	23	1324	0.67	0.298	0.25	19.28	0.28	0.003
26.34	2.12	73.28	1.45	106.08	23	1320	0.67	0.303	0.25	19.28	0.28	0.002
26.38	2.13	71.93	1.47	105.50	23	1318	0.67	0.306	0.26	19.28	0.29	0.001
26.47	2.14	70.79	1.48	104.75	23	1313	0.67	0.312	0.26	19.28	0.29	0.003
26.56	2.14	69.97	1.49	104.01	23	1307	0.67	0.319	0.27	19.28	0.30	0.003
26.62	2.14	69.71	1.48	103.48	23	1302	0.67	0.324	0.28	19.28	0.31	0.002
26.66	2.14	69.05	1.49	102.90	23	1297	0.67	0.331	0.28	19.28	0.32	0.002
26.71	2.15	67.45	1.51	101.87	23	1287	0.67	0.341	0.30	19.28	0.33	0.002
26.80	2.16	65.75	1.53	100.77	22	1277	0.67	0.354	0.31	19.28	0.35	0.004
26.85	2.17	64.36	1.55	99.66	22	1265	0.67	0.369	0.33	19.28	0.36	0.002
26.98	2.17	64.40	1.54	99.08	22	1261	0.67	0.376	0.33	19.28	0.37	0.006
27.07	2.16	65.13	1.52	98.86	22	1261	0.67	0.379	0.34	19.28	0.38	0.004
27.13	2.14	66.85	1.49	99.36	22	1267	0.67	0.374	0.34	19.28	0.38	0.002
27.17	2.12	68.82	1.45	100.11	22	1273	0.67	0.368	0.33	19.28	0.37	0.002
27.28	2.11	70.40	1.43	100.76	22	1280	0.67	0.364	0.33	19.28	0.37	0.005
27.31	2.09	72.00	1.41	101.49	22	1283	0.67	0.361	0.32	19.28	0.36	0.001
27.38	2.08	73.17	1.39	101.98	22	1285	0.67	0.361	0.32	19.28	0.36	0.003
27.49	2.07	74.29	1.38	102.50	22	1289	0.67	0.360	0.32	19.28	0.36	0.005
27.54	2.06	75.05	1.37	102.93	22	1291	0.67	0.359	0.32	19.28	0.36	0.002
27.58	2.06	75.36	1.37	103.11	22	1293	0.67	0.358	0.32	19.28	0.36	0.002
27.63	2.06	75.48	1.37	103.22	22	1295	0.67	0.357	0.32	19.28	0.36	0.002
27.71	2.06	75.54	1.37	103.43	22	1301	0.67	0.352	0.31	19.28	0.35	0.003
27.81	2.06	75.63	1.37	103.73	22	1309	0.67	0.346	0.31	19.28	0.34	0.004
27.86	2.07	75.74	1.37	104.04	22	1316	0.67	0.341	0.30	19.28	0.34	0.002
27.89	2.07	75.71	1.38	104.14	22	1319	0.67	0.339	0.30	19.28	0.33	0.001
28.00	2.07	75.34	1.38	103.86	22	1321	0.67	0.340	0.30	19.28	0.34	0.004
28.04	2.07	74.92	1.38	103.35	22	1316	0.67	0.345	0.31	19.28	0.34	0.002
28.14	2.07	74.52	1.38	102.71	22	1310	0.67	0.353	0.32	19.28	0.35	0.004
28.18	2.07	74.34	1.37	102.16	22	1302	0.67	0.363	0.33	19.28	0.37	0.002
28.23	2.06	74.37	1.37	101.83	22	1296	0.67	0.370	0.34	19.28	0.38	0.002
28.30	2.06	74.58	1.36	101.56	22	1291	0.67	0.378	0.34	19.28	0.39	0.003
28.37	2.05	74.89	1.35	101.41	22	1285	0.67	0.386	0.35	19.28	0.40	0.003
28.41	2.04	75.66	1.34	101.66	21	1282	0.67	0.391	0.36	19.28	0.40	0.002
28.50	2.03	76.65	1.33	102.14	21	1281	0.67	0.394	0.36	19.28	0.40	0.004
28.55	2.02	77.99	1.32	103.03	22	1282	0.67	0.394	0.36	19.28	0.40	0.002
28.64	2.01	78.73	1.32	103.54	22	1285	0.67	0.393	0.36	19.28	0.40	0.004
28.68	2.01	78.98	1.31	103.74	22	1287	0.67	0.392	0.36	19.28	0.40	0.002
28.78	2.01	78.77	1.31	103.55	22	1288	0.67	0.393	0.36	19.28	0.40	0.005

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
28.83	2.01	78.79	1.31	103.59	22	1290	0.67	0.392	0.36	19.28	0.40	0.002
28.92	2.01	78.82	1.32	103.66	22	1294	0.67	0.390	0.35	19.28	0.40	0.004
28.97	2.02	78.28	1.32	103.34	22	1297	0.67	0.388	0.35	19.28	0.39	0.002
29.01	2.04	75.91	1.34	101.69	21	1296	0.67	0.390	0.36	19.28	0.40	0.002
29.07	2.06	73.59	1.36	100.42	21	1300	0.67	0.387	0.36	19.28	0.40	0.003
29.17	2.08	71.62	1.39	99.59	21	1306	0.67	0.382	0.35	19.28	0.39	0.004
29.21	2.09	70.63	1.41	99.64	22	1317	0.67	0.372	0.34	19.28	0.38	0.002
29.30	2.11	69.06	1.44	99.16	22	1321	0.67	0.369	0.34	19.28	0.38	0.004
29.39	2.13	67.43	1.46	98.67	22	1324	0.67	0.369	0.34	19.28	0.38	0.004
29.44	2.14	66.19	1.48	98.25	22	1323	0.67	0.371	0.34	19.28	0.38	0.002
29.49	2.15	64.77	1.51	97.90	22	1322	0.67	0.372	0.34	19.28	0.38	0.002
29.56	2.17	63.40	1.54	97.51	22	1321	0.67	0.376	0.34	19.28	0.38	0.003
29.63	2.17	62.52	1.55	97.10	22	1317	0.67	0.381	0.35	19.28	0.39	0.003
29.67	2.17	62.33	1.55	96.43	21	1310	0.67	0.391	0.36	19.28	0.40	0.002
29.76	2.16	62.64	1.53	95.79	21	1303	0.67	0.401	0.37	19.28	0.42	0.005
29.82	2.15	63.18	1.51	95.24	21	1296	0.67	0.411	0.39	19.28	0.43	0.003
29.91	2.14	63.55	1.49	94.88	21	1292	0.67	0.418	0.40	19.28	0.44	0.004
29.95	2.14	63.56	1.49	94.48	21	1287	0.67	0.425	0.41	19.28	0.45	0.002
29.99	2.14	63.37	1.48	93.78	21	1278	0.67	0.438	0.42	19.28	0.47	0.002
30.09	2.13	63.21	1.47	93.18	20	1271	0.67	0.450	0.44	19.28	0.49	0.005
30.13	2.13	63.10	1.47	92.93	20	1269	0.67	0.454	0.44	19.28	0.50	0.003
30.22	2.12	65.03	1.45	94.28	21	1286	0.67	0.434	0.42	19.28	0.47	0.005
30.32	2.10	67.90	1.42	96.30	21	1307	0.67	0.409	0.39	19.28	0.44	0.005
30.36	2.06	73.22	1.37	100.07	21	1334	0.67	0.380	0.35	19.28	0.39	0.002
30.41	2.02	78.72	1.33	104.38	22	1357	0.67	0.356	0.32	19.28	0.36	0.002
30.47	1.98	85.17	1.29	109.83	23	1381	0.67	0.336	0.29	19.28	0.32	0.002
30.55	1.94	90.18	1.27	114.20	23	1397	0.67	0.323	0.27	19.28	0.30	0.003
30.58	1.90	96.28	1.24	119.28	24	1406	0.67	0.317	0.26	19.28	0.29	0.001
30.73	1.87	100.84	1.22	122.67	24	1410	0.67	0.316	0.25	19.28	0.28	0.005
30.77	1.84	105.30	1.19	125.59	25	1411	0.67	0.316	0.25	19.28	0.28	0.001
30.78	1.82	107.73	1.18	126.90	25	1411	0.67	0.316	0.25	19.28	0.27	0.000
30.87	1.81	110.52	1.16	128.27	25	1414	0.67	0.315	0.24	19.28	0.27	0.003
30.92	1.79	113.94	1.14	129.80	25	1421	0.67	0.311	0.24	19.28	0.27	0.002
31.01	1.78	116.26	1.13	130.91	25	1431	0.67	0.305	0.23	19.28	0.26	0.003
31.05	1.77	118.02	1.12	131.91	25	1441	0.67	0.298	0.23	19.28	0.25	0.001
31.10	1.77	119.10	1.11	132.73	25	1450	0.67	0.292	0.22	19.28	0.25	0.001
31.19	1.77	119.93	1.11	133.54	26	1461	0.67	0.286	0.21	19.28	0.24	0.002
31.24	1.77	120.82	1.11	134.57	26	1474	0.67	0.278	0.21	19.28	0.23	0.001
31.33	1.77	121.39	1.11	135.20	26	1483	0.67	0.273	0.20	19.28	0.23	0.003
31.38	1.76	123.04	1.10	135.71	26	1489	0.67	0.271	0.20	19.28	0.22	0.001
31.47	1.74	124.72	1.07	134.01	25	1474	0.67	0.282	0.21	19.28	0.24	0.003
31.52	1.72	127.79	1.02	130.41	24	1451	0.67	0.298	0.23	19.28	0.26	0.002
31.60	1.69	130.46	1.00	130.46	24	1431	0.67	0.314	0.25	19.28	0.28	0.003
31.66	1.68	132.94	1.00	132.94	25	1427	0.67	0.318	0.25	19.28	0.28	0.002
31.70	1.67	134.64	1.00	134.64	25	1432	0.67	0.315	0.24	19.28	0.27	0.001
31.79	1.67	135.15	1.00	135.15	25	1436	0.67	0.314	0.24	19.28	0.27	0.003
31.84	1.67	135.33	1.00	135.33	25	1440	0.67	0.311	0.24	19.28	0.27	0.002
31.90	1.67	134.64	1.00	134.64	25	1445	0.67	0.308	0.24	19.28	0.27	0.002

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
31.98	1.67	135.09	1.00	135.09	25	1453	0.67	0.304	0.23	19.28	0.26	0.003
32.03	1.67	136.61	1.00	136.61	25	1464	0.67	0.296	0.22	19.28	0.25	0.001
32.11	1.67	138.80	1.00	138.80	26	1479	0.67	0.287	0.21	19.28	0.24	0.002
32.17	1.66	140.63	1.00	140.63	26	1495	0.67	0.278	0.20	19.28	0.23	0.002
32.24	1.66	142.02	1.00	142.02	26	1509	0.67	0.270	0.20	19.28	0.22	0.002
32.31	1.66	142.75	1.00	142.75	26	1519	0.67	0.265	0.19	19.28	0.21	0.002
32.35	1.67	142.60	1.00	142.60	26	1530	0.67	0.259	0.19	19.28	0.21	0.001
32.45	1.67	141.79	1.00	141.79	26	1540	0.67	0.255	0.18	19.28	0.21	0.002
32.50	1.68	140.53	1.00	140.53	26	1551	0.67	0.250	0.18	19.28	0.20	0.001
32.59	1.69	139.37	1.00	139.37	26	1557	0.67	0.248	0.18	19.28	0.20	0.002
32.62	1.70	138.23	1.00	138.23	26	1559	0.67	0.247	0.18	19.28	0.20	0.001
32.68	1.70	137.80	1.00	137.80	26	1564	0.67	0.245	0.18	19.28	0.20	0.001
32.77	1.71	136.88	1.00	137.25	26	1568	0.67	0.244	0.18	19.28	0.20	0.002
32.81	1.71	136.95	1.01	137.83	26	1574	0.67	0.241	0.18	19.28	0.20	0.001
32.91	1.71	136.90	1.01	137.85	26	1577	0.67	0.241	0.18	19.28	0.20	0.002
32.95	1.70	138.68	1.00	138.68	26	1580	0.67	0.240	0.18	19.28	0.20	0.001
33.04	1.69	140.34	1.00	140.34	26	1584	0.67	0.239	0.17	19.28	0.19	0.002
33.09	1.68	142.76	1.00	142.76	26	1586	0.67	0.239	0.17	19.28	0.19	0.001
33.19	1.68	144.11	1.00	144.11	27	1585	0.67	0.240	0.17	19.28	0.19	0.002
33.23	1.67	144.92	1.00	144.92	27	1583	0.67	0.241	0.17	19.28	0.19	0.001
33.28	1.67	144.36	1.00	144.36	27	1580	0.67	0.244	0.17	19.28	0.19	0.001
33.37	1.67	143.44	1.00	143.44	27	1577	0.67	0.246	0.18	19.28	0.20	0.002
33.42	1.67	142.39	1.00	142.39	26	1571	0.67	0.250	0.18	19.28	0.20	0.001
33.51	1.68	141.50	1.00	141.50	26	1565	0.67	0.254	0.18	19.28	0.21	0.002
33.56	1.68	140.57	1.00	140.57	26	1559	0.67	0.257	0.19	19.28	0.21	0.001
33.60	1.68	138.78	1.00	138.78	26	1553	0.67	0.261	0.19	19.28	0.22	0.001
33.69	1.69	136.58	1.00	136.58	25	1546	0.67	0.266	0.20	19.28	0.22	0.002
33.73	1.70	133.11	1.00	133.11	25	1535	0.67	0.273	0.21	19.28	0.24	0.001
33.82	1.71	129.48	1.01	131.22	25	1520	0.67	0.283	0.22	19.28	0.25	0.003
33.92	1.72	126.06	1.04	130.66	25	1506	0.67	0.293	0.23	19.28	0.26	0.003
33.94	1.73	123.75	1.05	129.63	24	1491	0.67	0.304	0.24	19.28	0.27	0.000
34.02	1.73	122.15	1.05	128.62	24	1480	0.67	0.313	0.25	19.28	0.28	0.003
34.06	1.74	119.95	1.06	127.15	24	1463	0.67	0.327	0.26	19.28	0.29	0.001
34.14	1.74	117.68	1.07	125.88	24	1448	0.67	0.339	0.28	19.28	0.31	0.003
34.20	1.75	114.84	1.08	124.47	24	1432	0.67	0.354	0.29	19.28	0.32	0.002
34.33	1.76	111.82	1.10	122.98	23	1418	0.67	0.369	0.31	19.28	0.34	0.006
34.43	1.77	108.70	1.12	121.56	23	1407	0.67	0.382	0.32	19.28	0.36	0.004
34.52	1.78	106.03	1.13	120.34	23	1400	0.67	0.390	0.33	19.28	0.37	0.004
34.62	1.79	104.97	1.14	119.73	23	1397	0.67	0.394	0.33	19.28	0.37	0.004
34.65	1.79	104.79	1.14	119.41	23	1394	0.67	0.399	0.34	19.28	0.38	0.001
34.75	1.78	105.28	1.13	119.10	23	1389	0.67	0.405	0.35	19.28	0.39	0.005
34.80	1.78	105.34	1.13	118.76	23	1385	0.67	0.411	0.35	19.28	0.39	0.002
34.84	1.78	105.57	1.13	119.02	23	1389	0.67	0.407	0.35	19.28	0.39	0.002
34.94	1.78	105.34	1.13	119.36	23	1397	0.67	0.400	0.34	19.28	0.38	0.004
34.98	1.80	103.50	1.15	118.95	23	1401	0.67	0.397	0.34	19.28	0.38	0.002
35.08	1.80	102.07	1.16	118.12	23	1398	0.67	0.401	0.34	19.28	0.38	0.005
35.11	1.81	101.30	1.16	117.38	23	1391	0.67	0.409	0.35	19.28	0.39	0.001
35.17	1.79	103.32	1.14	117.92	23	1389	0.67	0.413	0.35	19.28	0.40	0.003

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
35.26	1.77	106.47	1.12	118.79	23	1392	0.67	0.410	0.35	19.28	0.39	0.004
35.31	1.75	110.76	1.08	119.95	23	1404	0.67	0.398	0.34	19.28	0.38	0.002
35.37	1.73	114.45	1.06	120.95	23	1421	0.67	0.382	0.33	19.28	0.36	0.003
35.45	1.74	115.80	1.06	122.76	23	1443	0.67	0.362	0.30	19.28	0.34	0.003
35.56	1.75	115.31	1.08	124.30	24	1461	0.67	0.348	0.29	19.28	0.32	0.004
35.59	1.76	113.18	1.10	124.90	24	1469	0.67	0.341	0.28	19.28	0.31	0.001
35.67	1.77	110.39	1.12	123.74	24	1461	0.67	0.349	0.28	19.28	0.32	0.003
35.73	1.79	107.52	1.14	122.25	24	1450	0.67	0.359	0.30	19.28	0.33	0.003
35.77	1.80	104.45	1.15	120.59	23	1441	0.67	0.368	0.31	19.28	0.34	0.002
35.87	1.82	100.96	1.18	118.65	23	1437	0.67	0.373	0.31	19.28	0.35	0.004
35.91	1.86	94.90	1.21	114.58	23	1428	0.67	0.382	0.33	19.28	0.37	0.002
35.98	1.90	88.80	1.24	109.72	22	1419	0.67	0.393	0.35	19.28	0.39	0.003
36.05	1.92	83.63	1.25	104.77	21	1390	0.67	0.425	0.40	19.28	0.44	0.004
36.10	1.92	81.35	1.25	101.49	20	1337	0.67	0.494	0.48	19.28	0.54	0.003
36.18	1.90	79.85	1.24	98.68	20	1281	0.67	0.591	0.60	19.28	0.67	0.007
36.23	1.89	78.11	1.23	96.27	19	1245	0.67	0.667	0.70	19.28	0.78	0.005
36.32	1.91	76.26	1.24	94.64	19	1240	0.67	0.679	0.72	19.28	0.81	0.009
36.38	1.92	74.38	1.25	93.07	19	1239	0.67	0.683	0.74	19.28	0.82	0.005
36.42	1.94	72.10	1.26	90.97	18	1230	0.67	0.705	0.77	19.28	0.87	0.005
36.49	1.95	70.14	1.27	88.98	18	1217	0.67	0.742	0.83	19.28	0.93	0.008
36.56	1.95	68.39	1.27	87.08	18	1200	0.67	0.791	0.91	19.28	1.02	0.009
36.62	1.95	67.58	1.27	85.89	18	1181	0.67	0.851	1.00	19.28	1.11	0.007
36.70	1.95	66.55	1.27	84.48	17	1161	0.67	0.924	1.10	19.28	1.24	0.012
36.75	1.94	65.68	1.27	83.17	17	1138	0.67	1.014	1.24	19.28	1.38	0.008
36.81	1.95	64.18	1.27	81.31	17	1115	0.67	1.121	1.40	19.28	1.57	0.012
36.89	1.95	62.09	1.27	79.02	16	1094	0.67	1.232	1.59	19.28	1.78	0.018
36.98	1.98	59.21	1.29	76.25	16	1078	0.67	1.328	1.77	19.28	1.98	0.020
37.07	2.00	56.35	1.31	73.74	15	1069	0.67	1.390	1.91	19.28	2.14	0.024
37.11	2.03	53.78	1.33	71.64	15	1061	0.67	1.441	2.02	19.28	2.27	0.012
37.16	2.05	51.70	1.36	70.08	15	1056	0.67	1.478	2.11	19.28	2.36	0.014
37.21	2.07	50.07	1.38	69.13	15	1057	0.67	1.476	2.12	19.28	2.37	0.013
37.29	2.09	48.61	1.41	68.50	15	1061	0.67	1.451	2.08	19.28	2.33	0.023
37.35	2.11	48.04	1.43	68.57	15	1069	0.67	1.399	1.99	19.28	2.23	0.015
37.44	2.11	47.82	1.44	68.67	15	1075	0.67	1.366	1.94	19.28	2.17	0.023
37.49	2.11	48.03	1.43	68.86	15	1078	0.67	1.347	1.91	19.28	2.14	0.013
37.54	2.10	48.35	1.42	68.86	15	1076	0.67	1.362	1.94	19.28	2.17	0.013
37.62	2.10	48.70	1.41	68.78	15	1072	0.67	1.390	1.98	19.28	2.22	0.022
37.67	2.09	48.70	1.40	68.39	15	1064	0.67	1.447	2.09	19.28	2.34	0.014
37.76	2.09	48.34	1.40	67.76	15	1055	0.67	1.516	2.21	19.28	2.48	0.025
37.81	2.08	48.59	1.39	67.54	14	1047	0.67	1.575	2.32	19.28	2.59	0.016
37.95	2.07	49.18	1.38	67.71	14	1046	0.66	1.592	2.35	19.28	2.63	0.043
37.97	2.06	50.29	1.36	68.50	15	1050	0.66	1.558	2.28	19.28	2.55	0.009
38.04	2.05	51.12	1.36	69.40	15	1062	0.66	1.474	2.13	19.28	2.38	0.018
38.08	2.05	52.40	1.35	70.79	15	1080	0.66	1.359	1.92	19.28	2.15	0.012
38.13	2.04	54.36	1.34	72.91	15	1106	0.66	1.213	1.66	19.28	1.86	0.011
38.22	2.02	56.57	1.33	75.17	16	1131	0.66	1.092	1.45	19.28	1.62	0.018
38.27	2.00	60.53	1.30	78.98	16	1162	0.66	0.961	1.22	19.28	1.36	0.007
38.37	1.97	65.00	1.28	83.24	17	1191	0.66	0.861	1.04	19.28	1.16	0.015

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
38.45	1.93	70.42	1.26	88.44	18	1224	0.66	0.766	0.87	19.28	0.98	0.010
38.56	1.91	74.06	1.24	91.87	18	1245	0.66	0.714	0.79	19.28	0.88	0.012
38.59	1.89	77.18	1.23	94.69	19	1258	0.66	0.682	0.73	19.28	0.82	0.003
38.65	1.86	79.93	1.21	96.88	19	1265	0.66	0.668	0.70	19.28	0.79	0.005
38.73	1.84	83.78	1.19	99.64	20	1273	0.66	0.653	0.67	19.28	0.75	0.007
38.87	1.81	87.06	1.17	101.62	20	1280	0.66	0.641	0.65	19.28	0.73	0.012
38.92	1.79	90.13	1.14	103.14	20	1285	0.66	0.631	0.64	19.28	0.71	0.004
39.01	1.78	91.74	1.13	103.68	20	1287	0.66	0.627	0.63	19.28	0.71	0.008
39.05	1.77	93.50	1.11	103.98	20	1287	0.66	0.629	0.63	19.28	0.71	0.004
39.14	1.76	94.57	1.10	103.93	20	1286	0.66	0.632	0.64	19.28	0.72	0.008
39.19	1.75	95.67	1.00	95.67	18	1281	0.66	0.643	0.72	19.28	0.81	0.004
39.24	1.73	96.42	1.00	96.42	18	1263	0.66	0.683	0.77	19.28	0.86	0.005
39.33	1.72	97.04	1.00	97.04	18	1246	0.66	0.724	0.81	19.28	0.91	0.010
39.38	1.71	97.63	1.00	97.63	18	1235	0.66	0.754	0.84	19.28	0.94	0.005
39.46	1.71	97.87	1.00	97.87	18	1239	0.66	0.745	0.83	19.28	0.93	0.010
39.51	1.71	97.86	1.00	97.86	18	1240	0.66	0.743	0.83	19.28	0.92	0.005
39.57	1.71	97.51	1.00	97.51	18	1239	0.66	0.746	0.83	19.28	0.93	0.007
39.66	1.71	97.22	1.00	97.22	18	1238	0.66	0.750	0.84	19.28	0.94	0.011
39.72	1.71	97.17	1.00	97.17	18	1235	0.66	0.758	0.85	19.28	0.95	0.006
39.80	1.71	96.99	1.00	96.99	18	1235	0.66	0.760	0.85	19.28	0.95	0.010
39.85	1.71	96.42	1.00	96.42	18	1238	0.66	0.754	0.85	19.28	0.95	0.006
39.90	1.73	95.57	1.00	95.57	18	1247	0.66	0.732	0.83	19.28	0.93	0.005
39.98	1.74	94.21	1.00	94.21	18	1263	0.66	0.693	0.79	19.28	0.89	0.008
40.04	1.78	91.50	1.12	102.84	20	1293	0.66	0.630	0.64	19.28	0.72	0.005
40.13	1.81	88.59	1.17	103.37	20	1324	0.66	0.573	0.57	19.28	0.64	0.007
40.17	1.86	84.90	1.21	102.65	20	1365	0.66	0.508	0.50	19.28	0.56	0.003
40.27	1.90	82.20	1.24	101.55	20	1400	0.66	0.462	0.45	19.28	0.51	0.006
40.31	1.93	80.02	1.26	100.64	20	1435	0.66	0.421	0.41	19.28	0.46	0.002
40.36	1.95	79.61	1.27	101.30	21	1478	0.66	0.380	0.36	19.28	0.41	0.002
40.45	1.98	79.80	1.29	102.76	21	1532	0.66	0.335	0.31	19.28	0.35	0.004
40.52	2.00	79.87	1.31	104.40	22	1595	0.66	0.294	0.27	19.28	0.30	0.003
40.59	2.02	79.35	1.33	105.25	22	1639	0.66	0.269	0.24	19.28	0.27	0.002
40.63	2.05	77.80	1.36	105.73	22	1688	0.66	0.246	0.21	19.28	0.24	0.001
40.73	2.08	76.21	1.39	106.05	23	1725	0.66	0.231	0.20	19.28	0.22	0.003
40.77	2.10	75.06	1.42	106.70	23	1756	0.66	0.219	0.18	19.28	0.21	0.001
40.82	2.08	78.57	1.39	109.18	23	1777	0.66	0.211	0.17	19.28	0.20	0.001
40.96	2.05	83.36	1.35	112.56	24	1797	0.65	0.206	0.17	19.28	0.19	0.003
41.00	2.00	90.65	1.31	118.30	25	1816	0.65	0.199	0.16	19.28	0.17	0.001
41.04	1.96	96.64	1.28	123.27	25	1825	0.65	0.197	0.15	19.28	0.17	0.001
41.09	1.92	103.24	1.25	128.76	26	1829	0.65	0.196	0.14	19.28	0.16	0.001
41.15	1.88	108.44	1.22	132.66	26	1822	0.65	0.198	0.14	19.28	0.16	0.001
41.21	1.85	111.51	1.20	134.20	26	1800	0.65	0.205	0.15	19.28	0.16	0.001
41.28	1.84	111.75	1.19	133.14	26	1766	0.65	0.217	0.16	19.28	0.18	0.002
41.38	1.83	110.38	1.19	131.07	26	1735	0.65	0.229	0.17	19.28	0.19	0.002
41.42	1.84	107.20	1.20	128.23	25	1712	0.65	0.239	0.18	19.28	0.20	0.001
41.51	1.86	104.65	1.21	126.27	25	1707	0.65	0.241	0.19	19.28	0.21	0.002
41.55	1.87	102.27	1.22	124.57	25	1709	0.65	0.241	0.19	19.28	0.21	0.001
41.60	1.89	100.11	1.23	123.32	25	1725	0.65	0.234	0.18	19.28	0.20	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
41.70	1.91	97.84	1.25	121.91	25	1744	0.65	0.227	0.18	19.28	0.20	0.002
41.74	1.93	96.20	1.26	121.05	25	1766	0.65	0.219	0.17	19.28	0.19	0.001
41.83	1.94	96.15	1.26	121.36	25	1783	0.65	0.213	0.17	19.28	0.19	0.002
41.88	1.92	98.25	1.25	123.04	25	1782	0.65	0.214	0.16	19.28	0.18	0.001
41.97	1.90	101.17	1.24	125.08	25	1771	0.65	0.218	0.17	19.28	0.19	0.002
42.02	1.87	105.30	1.21	127.76	25	1753	0.65	0.225	0.17	19.28	0.19	0.001
42.07	1.84	108.87	1.19	130.02	26	1749	0.65	0.227	0.17	19.28	0.19	0.001
42.15	1.82	111.95	1.18	131.65	26	1746	0.65	0.228	0.17	19.28	0.19	0.002
42.20	1.82	111.34	1.18	130.83	25	1735	0.65	0.233	0.17	19.28	0.19	0.001
42.26	1.82	110.15	1.18	129.63	25	1723	0.65	0.238	0.18	19.28	0.20	0.001
42.34	1.83	108.42	1.18	128.20	25	1713	0.65	0.243	0.19	19.28	0.21	0.002
42.46	1.83	108.21	1.18	127.72	25	1706	0.65	0.246	0.19	19.28	0.21	0.003
42.50	1.83	106.80	1.19	126.66	25	1701	0.65	0.248	0.19	19.28	0.21	0.001
42.55	1.84	105.07	1.19	125.42	25	1698	0.65	0.250	0.19	19.28	0.22	0.001
42.59	1.86	103.16	1.21	124.42	25	1707	0.65	0.246	0.19	19.28	0.22	0.001
42.69	1.87	101.66	1.22	123.71	25	1722	0.65	0.240	0.19	19.28	0.21	0.002
42.73	1.88	100.87	1.23	123.67	25	1744	0.65	0.231	0.18	19.28	0.20	0.001
42.81	1.89	101.53	1.23	124.84	25	1771	0.65	0.221	0.17	19.28	0.19	0.002
42.87	1.89	103.98	1.23	127.80	26	1813	0.65	0.207	0.15	19.28	0.17	0.001
42.96	1.89	106.30	1.23	130.64	26	1855	0.65	0.194	0.14	19.28	0.16	0.002
43.01	1.89	107.82	1.23	132.68	26	1890	0.65	0.185	0.13	19.28	0.15	0.001
43.06	1.90	107.74	1.24	133.12	27	1912	0.65	0.179	0.13	19.28	0.14	0.001
43.13	1.90	107.60	1.24	133.27	27	1924	0.65	0.176	0.12	19.28	0.14	0.001
43.20	1.90	107.35	1.24	133.12	27	1928	0.65	0.176	0.12	19.28	0.14	0.001
43.24	1.90	106.70	1.24	132.15	27	1911	0.65	0.180	0.13	19.28	0.14	0.001
43.33	1.90	104.87	1.24	129.98	26	1885	0.65	0.187	0.14	19.28	0.15	0.002
43.38	1.92	99.97	1.25	124.80	25	1836	0.64	0.201	0.15	19.28	0.17	0.001
43.47	1.94	94.12	1.26	118.76	24	1786	0.64	0.218	0.17	19.28	0.19	0.002
43.51	1.97	85.67	1.28	109.91	23	1705	0.64	0.250	0.22	19.28	0.24	0.001
43.61	1.99	79.52	1.30	103.30	21	1638	0.64	0.284	0.26	19.28	0.29	0.003
43.64	2.01	73.12	1.32	96.43	20	1563	0.64	0.331	0.33	19.28	0.37	0.001
43.70	2.04	68.96	1.34	92.39	19	1527	0.64	0.359	0.37	19.28	0.41	0.003
43.79	2.06	65.53	1.36	89.21	19	1498	0.64	0.383	0.41	19.28	0.46	0.005
43.83	2.08	63.42	1.39	88.41	19	1511	0.64	0.372	0.40	19.28	0.44	0.002
43.93	2.10	62.57	1.42	88.82	19	1536	0.64	0.353	0.37	19.28	0.41	0.005
43.98	2.12	62.03	1.45	89.98	20	1571	0.64	0.327	0.33	19.28	0.37	0.002
44.07	2.12	62.77	1.45	91.12	20	1594	0.64	0.312	0.31	19.28	0.35	0.004
44.12	2.11	64.64	1.44	92.80	20	1617	0.64	0.298	0.29	19.28	0.33	0.002
44.16	2.05	73.12	1.35	99.05	21	1665	0.64	0.271	0.26	19.28	0.29	0.002
44.26	1.98	83.44	1.30	108.05	22	1721	0.64	0.246	0.21	19.28	0.24	0.003
44.30	1.90	99.06	1.24	122.79	25	1804	0.64	0.214	0.17	19.28	0.19	0.001
44.39	1.85	111.01	1.20	133.61	26	1873	0.64	0.193	0.14	19.28	0.16	0.002
44.44	1.82	122.25	1.17	142.98	28	1946	0.64	0.174	0.12	19.28	0.13	0.001
44.53	1.80	127.46	1.15	147.15	28	1987	0.64	0.165	0.11	19.28	0.12	0.001
44.58	1.80	129.56	1.15	149.38	29	2017	0.64	0.159	0.10	19.28	0.11	0.001
44.62	1.81	129.31	1.16	149.98	29	2034	0.64	0.156	0.10	19.28	0.11	0.001
44.71	1.82	127.74	1.17	149.47	29	2042	0.64	0.154	0.10	19.28	0.11	0.001
44.76	1.84	123.11	1.19	146.67	29	2041	0.64	0.155	0.10	19.28	0.11	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
44.85	1.86	117.64	1.21	142.44	28	2026	0.64	0.158	0.10	19.28	0.12	0.001
44.89	1.91	108.31	1.24	134.66	27	2007	0.64	0.162	0.11	19.28	0.13	0.001
44.99	1.96	99.03	1.28	126.49	26	1988	0.64	0.166	0.12	19.28	0.14	0.002
45.04	2.02	89.78	1.32	118.74	25	1971	0.64	0.170	0.13	19.28	0.15	0.001
45.08	2.07	81.92	1.38	113.09	24	1957	0.64	0.173	0.14	19.28	0.15	0.001
45.17	2.12	76.27	1.44	110.14	24	1955	0.64	0.173	0.14	19.28	0.16	0.002
45.22	2.16	71.13	1.53	108.67	24	1959	0.64	0.173	0.14	19.28	0.15	0.001
45.28	2.21	66.63	1.63	108.63	25	1968	0.64	0.171	0.13	19.28	0.15	0.001
45.37	2.27	60.43	1.81	109.10	25	1961	0.64	0.172	0.13	19.28	0.15	0.002
45.50	2.32	55.54	1.98	110.18	26	1949	0.63	0.176	0.13	19.28	0.14	0.002
45.54	2.33	54.11	2.04	110.24	26	1940	0.63	0.178	0.13	19.28	0.14	0.001
45.56	2.29	57.55	1.90	109.09	26	1951	0.63	0.175	0.13	19.28	0.15	0.000
45.64	2.24	63.44	1.71	108.65	25	1974	0.63	0.170	0.13	19.28	0.15	0.001
45.68	2.18	70.67	1.56	109.96	25	2000	0.63	0.164	0.13	19.28	0.14	0.001
45.77	2.13	77.28	1.46	112.89	25	2030	0.63	0.158	0.12	19.28	0.14	0.001
45.82	2.09	82.48	1.41	116.33	25	2063	0.63	0.152	0.12	19.28	0.13	0.001
45.89	2.08	85.60	1.39	119.05	26	2095	0.63	0.147	0.11	19.28	0.12	0.001
45.96	2.08	85.73	1.40	119.77	26	2116	0.63	0.143	0.11	19.28	0.12	0.001
46.01	2.11	82.63	1.43	118.48	26	2123	0.63	0.142	0.11	19.28	0.12	0.001
46.09	2.14	79.30	1.48	117.35	26	2129	0.63	0.141	0.10	19.28	0.12	0.001
46.14	2.15	76.05	1.51	115.02	25	2099	0.63	0.147	0.11	19.28	0.12	0.001
46.23	2.15	75.85	1.50	113.52	25	2070	0.63	0.152	0.12	19.28	0.13	0.001
46.28	2.11	79.20	1.44	113.72	25	2048	0.63	0.156	0.12	19.28	0.14	0.001
46.37	2.08	84.02	1.39	116.46	25	2059	0.63	0.154	0.12	19.28	0.13	0.001
46.40	2.05	87.69	1.36	118.94	25	2069	0.63	0.152	0.12	19.28	0.13	0.001
46.50	2.06	86.98	1.36	118.36	25	2067	0.63	0.153	0.12	19.28	0.13	0.001
46.54	2.07	84.88	1.38	117.13	25	2070	0.63	0.152	0.12	19.28	0.13	0.001
46.59	2.08	83.84	1.39	116.72	25	2077	0.63	0.151	0.12	19.28	0.13	0.001
46.68	2.08	83.69	1.39	116.70	25	2081	0.63	0.150	0.11	19.28	0.13	0.001
46.72	2.07	85.29	1.38	117.57	25	2082	0.63	0.150	0.11	19.28	0.13	0.001
46.82	2.05	87.84	1.36	119.30	25	2089	0.63	0.149	0.11	19.28	0.13	0.001
46.91	2.03	90.86	1.34	121.58	26	2101	0.63	0.147	0.11	19.28	0.12	0.001
46.93	2.03	92.69	1.33	123.43	26	2122	0.63	0.144	0.11	19.28	0.12	0.000
47.01	2.04	91.74	1.34	123.33	26	2145	0.63	0.140	0.10	19.28	0.11	0.001
47.06	2.07	88.47	1.38	122.09	26	2174	0.63	0.136	0.10	19.28	0.11	0.001
47.15	2.11	84.25	1.43	120.36	26	2190	0.63	0.134	0.10	19.28	0.11	0.001
47.20	2.14	80.15	1.48	118.96	26	2196	0.63	0.133	0.10	19.28	0.11	0.001
47.28	2.15	78.24	1.51	118.34	26	2197	0.63	0.133	0.10	19.28	0.11	0.001
47.33	2.15	78.63	1.51	118.71	26	2204	0.63	0.132	0.10	19.28	0.11	0.001
47.38	2.14	79.86	1.49	119.26	26	2211	0.62	0.131	0.09	19.28	0.11	0.001
47.47	2.14	80.58	1.48	119.37	26	2211	0.62	0.131	0.09	19.28	0.11	0.001
47.51	2.14	80.13	1.48	118.66	26	2199	0.62	0.133	0.10	19.28	0.11	0.001
47.61	2.14	79.51	1.48	117.87	26	2188	0.62	0.135	0.10	19.28	0.11	0.001
47.65	2.11	83.40	1.43	119.26	26	2187	0.62	0.135	0.10	19.28	0.11	0.001
47.74	2.06	90.38	1.36	123.31	26	2197	0.62	0.133	0.10	19.28	0.11	0.001
47.77	1.98	103.63	1.29	133.45	28	2212	0.62	0.131	0.09	19.28	0.10	0.000
47.85	1.91	115.21	1.24	143.22	29	2220	0.62	0.130	0.08	19.28	0.09	0.001
47.92	1.85	125.66	1.20	151.34	30	2224	0.62	0.130	0.08	19.28	0.09	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
47.97	1.83	131.24	1.18	154.73	30	2219	0.62	0.131	0.08	19.28	0.09	0.001
48.03	1.80	136.02	1.15	156.24	30	2199	0.62	0.133	0.08	19.28	0.09	0.001
48.10	1.75	142.84	1.09	155.87	30	2163	0.62	0.139	0.09	19.28	0.10	0.001
48.21	1.72	149.82	1.02	152.67	29	2136	0.62	0.143	0.09	19.28	0.10	0.001
48.25	1.68	157.43	1.00	157.43	29	2138	0.62	0.143	0.09	19.28	0.10	0.000
48.34	1.67	162.01	1.00	162.01	30	2168	0.62	0.138	0.09	19.28	0.10	0.001
48.39	1.67	165.32	1.00	165.32	31	2198	0.62	0.134	0.08	19.28	0.09	0.001
48.44	1.68	165.10	1.00	165.10	31	2216	0.62	0.131	0.08	19.28	0.09	0.001
48.54	1.68	164.02	1.00	164.02	30	2227	0.62	0.130	0.08	19.28	0.09	0.001
48.58	1.69	161.05	1.00	161.05	30	2234	0.62	0.129	0.08	19.28	0.09	0.000
48.66	1.71	158.24	1.00	158.81	30	2243	0.62	0.128	0.08	19.28	0.09	0.001
48.70	1.71	155.84	1.02	158.66	30	2233	0.62	0.129	0.08	19.28	0.09	0.000
48.76	1.70	155.72	1.00	155.72	29	2187	0.62	0.136	0.09	19.28	0.10	0.001
48.85	1.68	157.25	1.00	157.25	29	2144	0.62	0.142	0.09	19.28	0.10	0.001
48.89	1.66	160.98	1.00	160.98	30	2122	0.62	0.146	0.09	19.28	0.10	0.001
48.97	1.65	166.54	1.00	166.54	31	2148	0.62	0.142	0.09	19.28	0.10	0.001
49.04	1.63	173.29	1.00	173.29	32	2180	0.62	0.137	0.08	19.28	0.09	0.001
49.08	1.63	175.55	1.00	175.55	32	2203	0.62	0.134	0.08	19.28	0.09	0.000
49.18	1.62	178.27	1.00	178.27	32	2224	0.61	0.131	0.07	19.28	0.08	0.001
49.24	1.62	180.86	1.00	180.86	33	2246	0.61	0.128	0.07	19.28	0.08	0.001
49.28	1.60	187.13	1.00	187.13	34	2283	0.61	0.124	0.07	19.28	0.07	0.000
49.37	1.60	190.41	1.00	190.41	34	2319	0.61	0.120	0.06	19.28	0.07	0.001
49.42	1.61	190.52	1.00	190.52	35	2365	0.61	0.115	0.06	19.28	0.07	0.000
49.51	1.63	188.40	1.00	188.40	34	2399	0.61	0.111	0.06	19.28	0.07	0.001
49.56	1.65	185.68	1.00	185.68	34	2423	0.61	0.109	0.06	19.28	0.06	0.000
49.61	1.65	186.00	1.00	186.00	34	2436	0.61	0.108	0.06	19.28	0.06	0.000
49.70	1.65	187.10	1.00	187.10	34	2448	0.61	0.107	0.06	19.28	0.06	0.001
49.74	1.65	187.90	1.00	187.90	34	2459	0.61	0.106	0.06	19.28	0.06	0.000
49.82	1.66	185.39	1.00	185.39	34	2460	0.61	0.106	0.06	19.28	0.06	0.001
49.88	1.67	183.08	1.00	183.08	34	2460	0.61	0.106	0.06	19.28	0.06	0.000
49.97	1.67	179.74	1.00	179.74	33	2446	0.61	0.107	0.06	19.28	0.07	0.001
50.03	1.67	180.12	1.00	180.12	33	2440	0.61	0.108	0.06	19.28	0.07	0.000
50.11	1.66	182.00	1.00	182.00	33	2423	0.61	0.109	0.06	19.28	0.07	0.001
50.15	1.64	188.00	1.00	188.00	34	2426	0.61	0.109	0.06	19.28	0.06	0.000
50.21	1.62	193.79	1.00	193.79	35	2457	0.61	0.106	0.05	19.28	0.06	0.000
50.30	1.61	200.64	1.00	200.64	36	2511	0.61	0.102	0.05	19.28	0.06	0.001
50.34	1.61	207.36	1.00	207.36	38	2582	0.61	0.096	0.05	19.28	0.05	0.000
50.44	1.60	212.64	1.00	212.64	38	2636	0.61	0.092	0.04	19.28	0.05	0.001
50.48	1.60	217.37	1.00	217.37	39	2690	0.61	0.089	0.04	19.28	0.04	0.000
50.53	1.60	219.26	1.00	219.26	39	2692	0.61	0.089	0.04	19.28	0.04	0.000
50.60	1.56	224.54	1.00	224.54	40	2633	0.61	0.093	0.04	19.28	0.05	0.000
50.66	1.58	232.31	1.00	232.31	42	2810	0.61	0.082	0.03	19.28	0.04	0.000
50.77	1.61	241.16	1.00	241.16	44	3015	0.61	0.072	0.03	19.28	0.03	0.000
50.80	1.65	246.43	1.00	246.43	45	3248	0.61	0.064	0.02	19.28	0.03	0.000
50.89	1.64	249.15	1.00	249.15	46	3265	0.61	0.063	0.02	19.28	0.03	0.000
50.94	1.64	249.55	1.00	249.55	46	3272	0.61	0.063	0.02	19.28	0.03	0.000
50.98	1.65	246.87	1.00	246.87	45	3253	0.60	0.064	0.02	19.28	0.03	0.000
51.06	1.65	237.86	1.00	237.86	44	3181	0.60	0.066	0.03	19.28	0.03	0.000

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
51.13	1.81	135.87	1.17	158.39	31	3800	0.60	0.050	0.03	19.28	0.03	0.000
51.22	1.80	132.77	1.16	153.70	30	3684	0.60	0.052	0.03	19.28	0.04	0.000
51.27	1.78	132.19	1.13	149.17	29	3561	0.60	0.055	0.04	19.28	0.04	0.000
51.31	1.77	129.92	1.11	144.04	28	3440	0.60	0.058	0.04	19.28	0.04	0.000
51.40	1.77	127.10	1.11	140.94	27	3372	0.60	0.060	0.04	19.28	0.05	0.000
51.45	1.77	123.35	1.12	137.74	26	3299	0.60	0.062	0.04	19.28	0.05	0.000
51.54	1.77	120.16	1.11	133.73	26	3209	0.60	0.065	0.05	19.28	0.05	0.001
51.59	1.76	116.80	1.11	129.32	25	3107	0.60	0.069	0.05	19.28	0.06	0.000
51.68	1.77	114.24	1.12	127.57	24	3070	0.60	0.071	0.06	19.28	0.06	0.001
51.73	1.79	112.00	1.14	127.32	24	3071	0.60	0.071	0.06	19.28	0.06	0.000
51.77	1.79	110.91	1.14	126.39	24	3053	0.60	0.071	0.06	19.28	0.06	0.000
51.84	1.79	110.37	1.15	126.47	24	3063	0.60	0.071	0.06	19.28	0.06	0.001
51.92	1.80	109.95	1.15	126.64	24	3076	0.60	0.070	0.06	19.28	0.06	0.001
51.99	1.80	111.34	1.15	128.00	25	3112	0.60	0.069	0.05	19.28	0.06	0.001
52.06	1.78	114.47	1.13	129.20	25	3136	0.60	0.068	0.05	19.28	0.06	0.001
52.11	1.77	117.92	1.11	130.85	25	3177	0.60	0.067	0.05	19.28	0.06	0.000
52.20	1.77	118.01	1.12	132.14	25	3215	0.60	0.065	0.05	19.28	0.06	0.001
52.25	1.79	116.25	1.14	132.75	26	3240	0.60	0.065	0.05	19.28	0.05	0.000
52.32	1.80	113.61	1.16	131.58	25	3228	0.60	0.065	0.05	19.28	0.05	0.000
52.38	1.81	111.83	1.16	129.80	25	3191	0.60	0.066	0.05	19.28	0.06	0.000
52.43	1.82	108.24	1.17	126.55	25	3124	0.60	0.069	0.05	19.28	0.06	0.000
52.52	1.83	103.77	1.19	123.03	24	3066	0.60	0.071	0.06	19.28	0.06	0.001
52.57	1.87	97.76	1.21	118.67	23	3022	0.60	0.073	0.06	19.28	0.07	0.000
52.63	1.90	92.38	1.24	114.39	23	2988	0.60	0.074	0.06	19.28	0.07	0.000
52.71	1.93	87.60	1.26	109.95	22	2931	0.59	0.077	0.07	19.28	0.08	0.001
52.80	1.94	84.88	1.26	106.95	22	2873	0.59	0.080	0.07	19.28	0.08	0.001
52.84	1.93	83.77	1.26	105.27	21	2819	0.59	0.083	0.08	19.28	0.09	0.000
52.89	1.92	84.14	1.25	105.31	21	2807	0.59	0.083	0.08	19.28	0.09	0.000
53.02	1.92	84.21	1.25	105.24	21	2806	0.59	0.083	0.08	19.28	0.09	0.001
53.11	1.95	80.04	1.27	101.60	21	2774	0.59	0.085	0.08	19.28	0.09	0.001
53.19	1.96	78.18	1.28	99.83	20	2753	0.59	0.087	0.08	19.28	0.09	0.001
53.23	1.97	77.10	1.28	98.87	20	2743	0.59	0.087	0.09	19.28	0.10	0.000
53.28	1.94	80.59	1.27	101.98	21	2782	0.59	0.085	0.08	19.28	0.09	0.001
53.38	1.94	81.69	1.26	103.10	21	2808	0.59	0.084	0.08	19.28	0.09	0.001
53.41	1.93	84.25	1.25	105.62	21	2851	0.59	0.081	0.08	19.28	0.08	0.000
53.52	1.91	86.59	1.25	107.86	22	2891	0.59	0.079	0.07	19.28	0.08	0.001
53.56	1.90	89.21	1.24	110.34	22	2932	0.59	0.077	0.07	19.28	0.08	0.000
53.61	1.89	90.64	1.23	111.74	22	2959	0.59	0.076	0.07	19.28	0.07	0.000
53.70	1.89	91.83	1.23	112.89	23	2984	0.59	0.075	0.07	19.28	0.07	0.001
53.75	1.88	92.59	1.23	113.50	23	2992	0.59	0.075	0.06	19.28	0.07	0.000
53.84	1.88	92.95	1.22	113.49	23	2983	0.59	0.075	0.07	19.28	0.07	0.001
53.89	1.86	93.92	1.21	113.67	22	2962	0.59	0.076	0.07	19.28	0.07	0.000
53.97	1.85	94.59	1.20	113.64	22	2945	0.59	0.077	0.07	19.28	0.08	0.001
54.02	1.83	96.51	1.19	114.62	22	2946	0.59	0.077	0.07	19.28	0.07	0.000
54.07	1.82	98.41	1.18	115.94	23	2969	0.59	0.076	0.07	19.28	0.07	0.000
54.15	1.82	100.27	1.17	117.28	23	2996	0.59	0.075	0.06	19.28	0.07	0.001
54.21	1.82	100.61	1.17	117.81	23	3015	0.59	0.074	0.06	19.28	0.07	0.001
54.30	1.82	100.37	1.17	117.92	23	3028	0.59	0.073	0.06	19.28	0.07	0.001

:: Post-earthquake settlement of dry sands :: (continued)

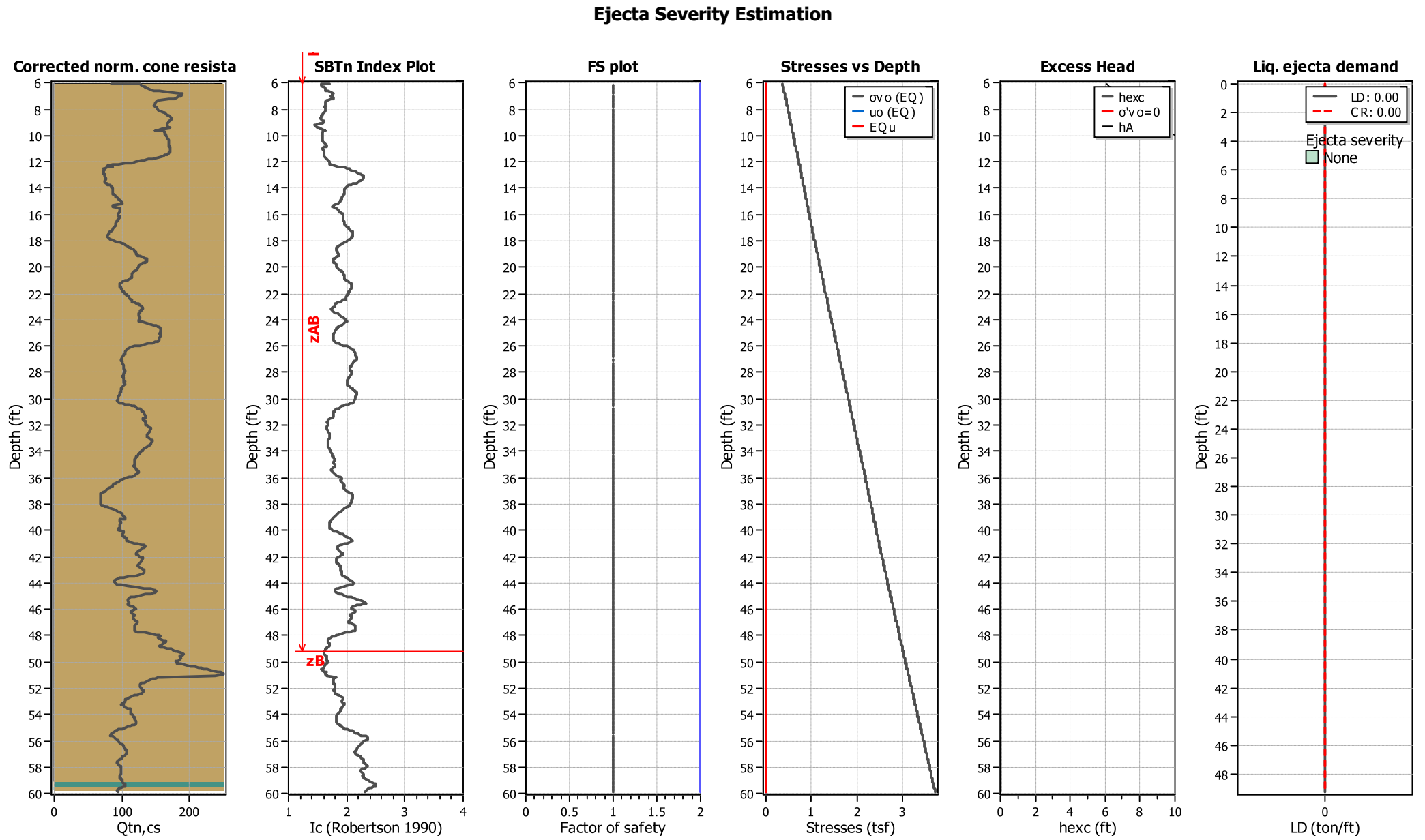
Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
54.34	1.83	100.44	1.18	118.46	23	3051	0.59	0.072	0.06	19.28	0.07	0.000
54.43	1.83	101.09	1.18	119.20	23	3075	0.59	0.071	0.06	19.28	0.07	0.001
54.48	1.83	101.84	1.18	120.19	23	3105	0.58	0.070	0.06	19.28	0.07	0.000
54.53	1.82	102.41	1.18	120.57	23	3114	0.58	0.070	0.06	19.28	0.06	0.000
54.62	1.83	102.39	1.18	120.74	24	3127	0.58	0.070	0.06	19.28	0.06	0.001
54.67	1.84	100.63	1.19	119.78	23	3124	0.58	0.070	0.06	19.28	0.06	0.000
54.76	1.86	97.77	1.21	118.02	23	3120	0.58	0.070	0.06	19.28	0.07	0.001
54.84	1.89	93.70	1.23	115.02	23	3103	0.58	0.071	0.06	19.28	0.07	0.001
54.90	1.90	90.73	1.24	112.31	23	3067	0.58	0.072	0.06	19.28	0.07	0.001
54.94	1.92	87.26	1.25	108.89	22	3009	0.58	0.074	0.07	19.28	0.07	0.000
54.99	1.93	83.03	1.26	104.48	21	2924	0.58	0.078	0.07	19.28	0.08	0.000
55.08	1.96	78.08	1.28	99.65	20	2849	0.58	0.082	0.08	19.28	0.09	0.001
55.13	2.00	72.73	1.31	95.07	20	2798	0.58	0.085	0.09	19.28	0.10	0.001
55.22	2.05	68.41	1.35	92.35	20	2792	0.58	0.085	0.09	19.28	0.10	0.001
55.27	2.09	63.78	1.41	89.72	19	2762	0.58	0.087	0.09	19.28	0.10	0.001
55.32	2.13	59.97	1.46	87.69	19	2720	0.58	0.089	0.09	19.28	0.11	0.001
55.41	2.16	56.25	1.52	85.50	19	2658	0.58	0.093	0.10	19.28	0.11	0.001
55.45	2.19	52.55	1.60	84.13	19	2603	0.58	0.097	0.10	19.28	0.12	0.001
55.58	2.23	48.99	1.70	83.46	19	2554	0.58	0.101	0.11	19.28	0.12	0.002
55.59	2.30	43.88	1.92	84.44	20	2490	0.58	0.106	0.11	19.28	0.12	0.000
55.65	2.34	41.73	2.07	86.25	21	2479	0.58	0.107	0.10	19.28	0.12	0.001
55.72	2.36	40.61	2.17	88.26	21	2490	0.58	0.106	0.10	19.28	0.11	0.001
55.78	2.35	41.95	2.13	89.29	21	2543	0.58	0.102	0.09	19.28	0.10	0.001
55.86	2.36	41.95	2.18	91.47	22	2584	0.58	0.099	0.09	19.28	0.10	0.001
55.92	2.36	42.95	2.18	93.51	23	2647	0.58	0.094	0.08	19.28	0.09	0.001
55.98	2.33	46.16	2.05	94.41	23	2741	0.58	0.088	0.08	19.28	0.09	0.001
56.05	2.29	50.37	1.88	94.72	22	2840	0.58	0.083	0.07	19.28	0.08	0.001
56.11	2.26	54.32	1.78	96.54	22	2950	0.58	0.077	0.07	19.28	0.08	0.001
56.20	2.25	56.33	1.75	98.58	23	3031	0.58	0.074	0.06	19.28	0.07	0.001
56.25	2.24	58.51	1.73	101.00	23	3120	0.58	0.070	0.06	19.28	0.07	0.000
56.33	2.23	60.34	1.69	102.10	23	3175	0.57	0.068	0.06	19.28	0.06	0.001
56.37	2.21	62.99	1.63	102.96	23	3228	0.57	0.066	0.06	19.28	0.06	0.000
56.47	2.19	64.62	1.60	103.30	23	3258	0.57	0.065	0.05	19.28	0.06	0.001
56.52	2.18	66.19	1.57	103.84	23	3287	0.57	0.065	0.05	19.28	0.06	0.000
56.57	2.18	67.01	1.56	104.56	23	3316	0.57	0.064	0.05	19.28	0.06	0.000
56.66	2.17	67.85	1.55	105.28	23	3346	0.57	0.063	0.05	19.28	0.06	0.001
56.70	2.16	69.34	1.53	106.03	24	3377	0.57	0.062	0.05	19.28	0.06	0.000
56.77	2.15	70.85	1.50	106.41	23	3396	0.57	0.061	0.05	19.28	0.06	0.000
56.85	2.14	72.27	1.48	106.70	23	3409	0.57	0.061	0.05	19.28	0.06	0.001
56.89	2.14	71.92	1.48	106.39	23	3402	0.57	0.061	0.05	19.28	0.06	0.000
56.99	2.15	70.16	1.51	105.89	23	3393	0.57	0.061	0.05	19.28	0.06	0.001
57.04	2.18	67.55	1.56	105.52	24	3376	0.57	0.062	0.05	19.28	0.06	0.000
57.09	2.20	65.01	1.61	104.80	24	3341	0.57	0.063	0.05	19.28	0.06	0.000
57.16	2.22	62.29	1.66	103.39	23	3281	0.57	0.065	0.05	19.28	0.06	0.001
57.22	2.23	59.45	1.70	101.32	23	3199	0.57	0.068	0.06	19.28	0.06	0.000
57.31	2.25	56.52	1.76	99.63	23	3122	0.57	0.071	0.06	19.28	0.07	0.001
57.35	2.27	53.53	1.83	98.04	23	3039	0.57	0.074	0.06	19.28	0.07	0.000
57.43	2.29	51.67	1.87	96.64	23	2979	0.57	0.077	0.07	19.28	0.07	0.001

:: Post-earthquake settlement of dry sands :: (continued)

Depth (ft)	I _c	Q _{tn}	K _c	Q _{tn,cs}	N _{1,60} (blows)	G _{max} (tsf)	CSR	Shear, γ (%)	e _{vol(15)} (%)	N _c	e _v (%)	Settle. (in)
57.48	2.29	50.66	1.87	94.86	22	2926	0.57	0.079	0.07	19.28	0.08	0.000
57.57	2.28	50.70	1.84	93.47	22	2903	0.57	0.080	0.07	19.28	0.08	0.001
57.62	2.28	50.15	1.85	92.74	22	2880	0.57	0.081	0.07	19.28	0.08	0.000
57.70	2.29	49.31	1.88	92.82	22	2870	0.57	0.082	0.07	19.28	0.08	0.001
57.76	2.31	47.96	1.95	93.59	22	2860	0.57	0.082	0.07	19.28	0.08	0.001
57.84	2.33	46.60	2.04	94.85	23	2858	0.57	0.083	0.07	19.28	0.08	0.001
57.89	2.35	45.44	2.12	96.51	23	2863	0.57	0.082	0.07	19.28	0.08	0.000
57.94	2.35	45.67	2.14	97.61	23	2891	0.57	0.081	0.07	19.28	0.07	0.000
58.03	2.34	46.99	2.09	97.97	23	2935	0.57	0.079	0.07	19.28	0.07	0.001
58.08	2.30	50.48	1.94	97.78	23	3014	0.57	0.075	0.06	19.28	0.07	0.000
58.16	2.27	53.87	1.82	97.84	23	3087	0.56	0.072	0.06	19.28	0.07	0.001
58.21	2.24	56.79	1.73	98.30	23	3148	0.56	0.070	0.06	19.28	0.07	0.000
58.31	2.24	57.44	1.72	98.54	23	3169	0.56	0.069	0.06	19.28	0.07	0.001
58.35	2.25	56.58	1.74	98.54	23	3158	0.56	0.070	0.06	19.28	0.07	0.000
58.45	2.27	54.04	1.82	98.50	23	3120	0.56	0.071	0.06	19.28	0.07	0.001
58.49	2.29	52.38	1.88	98.33	23	3087	0.56	0.072	0.06	19.28	0.07	0.000
58.54	2.29	51.67	1.89	97.82	23	3065	0.56	0.073	0.06	19.28	0.07	0.000
58.63	2.28	52.38	1.85	96.73	23	3061	0.56	0.073	0.06	19.28	0.07	0.001
58.67	2.28	52.25	1.84	96.13	22	3048	0.56	0.074	0.06	19.28	0.07	0.000
58.74	2.28	52.05	1.85	96.19	22	3049	0.56	0.074	0.06	19.28	0.07	0.001
58.81	2.29	51.36	1.89	96.87	23	3054	0.56	0.074	0.06	19.28	0.07	0.001
58.86	2.31	49.62	1.98	98.04	23	3043	0.56	0.074	0.06	19.28	0.07	0.000
58.96	2.34	47.46	2.09	98.99	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.00	2.37	44.98	2.22	99.77	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.05	2.39	43.05	2.32	99.65	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.14	2.43	39.73	2.54	100.86	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.27	2.47	36.96	2.77	102.53	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.28	2.50	35.02	2.99	104.54	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.32	2.50	35.04	2.99	104.88	27	2752	0.56	0.089	0.06	19.28	0.07	0.000
59.42	2.49	35.80	2.91	104.36	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.46	2.46	37.61	2.74	102.98	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.53	2.42	39.99	2.49	99.46	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.60	2.38	42.30	2.27	96.00	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.66	2.35	44.08	2.13	93.68	0	0	0.56	0.000	0.00	0.00	0.00	0.000
59.74	2.33	45.35	2.06	93.39	22	2899	0.56	0.081	0.07	19.28	0.08	0.001
59.79	2.32	46.43	2.02	93.89	22	2938	0.56	0.079	0.07	19.28	0.08	0.000
59.88	2.32	47.03	2.00	94.06	22	2960	0.56	0.078	0.07	19.28	0.08	0.001

Total estimated settlement: 3.42**Abbreviations**

Q _{tn} :	Equivalent clean sand normalized cone resistance
K _c :	Fines correction factor
Q _{tn,cs} :	Post-liquefaction volumetric strain
G _{max} :	Small strain shear modulus
CSR:	Soil cyclic stress ratio
γ:	Cyclic shear strain
e _{vol(15)} :	Volumetric strain after 15 cycles
N _c :	Equivalent number of cycles
e _v :	Volumetric strain
Settle.:	Calculated settlement



APPENDIX E

GENERAL GRADING GUIDELINES

Geotechnical and Infiltration Evaluation

Proposed Residential Development

APNs 167-091-09 through -12

Northeast of the 210 Freeway and West San Bernardino Avenue

Redlands, San Bernardino County, California

Project No. 4212-CR



GENERAL GRADING GUIDELINES

Guidelines presented herein are intended to address general construction procedures for earthwork construction. Specific situations and conditions often arise which cannot reasonably be discussed in general guidelines, when anticipated these are discussed in the text of the report. Often unanticipated conditions are encountered which may necessitate modification or changes to these guidelines. It is our hope that these will assist the contractor to more efficiently complete the project by providing a reasonable understanding of the procedures that would be expected during earthwork and the testing and observation used to evaluate those procedures.

General

Grading should be performed to at least the minimum requirements of governing agencies, Chapters 18 and 33 of the Uniform Building Code, CBC (2022) and the guidelines presented below.

Preconstruction Meeting

A preconstruction meeting should be held prior to site earthwork. Any questions the contractor has regarding our recommendations, general site conditions, apparent discrepancies between reported and actual conditions and/or differences in procedures the contractor intends to use should be brought up at that meeting. The contractor (including the main onsite representative) should review our report and these guidelines in advance of the meeting. Any comments the contractor may have regarding these guidelines should be brought up at that meeting.

Grading Observation and Testing

1. Observation of the fill placement should be provided by our representative during grading. Verbal communication during the course of each day will be used to inform the contractor of test results. The contractor should receive a copy of the "Daily Field Report" indicating results of field density tests that day. If our representative does not provide the contractor with these reports, our office should be notified.
2. Testing and observation procedures are, by their nature, specific to the work or area observed and location of the tests taken, variability may occur in other locations. The contractor is responsible for the uniformity of the grading operations; our observations and test results are intended to evaluate the contractor's overall level of efforts during grading. The contractor's personnel are the only individuals participating in all aspects of site work. Compaction testing and observation should not be considered as relieving the contractor's responsibility to properly compact the fill.
3. Cleanouts, processed ground to receive fill, key excavations, and subdrains should be observed by our representative prior to placing any fill. It will be the contractor's responsibility to notify our representative or office when such areas are ready for observation.



4. Density tests may be done on the surface material to receive fill, as considered warranted by this firm.
5. In general, density tests would be made at maximum intervals of two feet of fill height, or every 1,000 cubic yards of fill placed. Criteria will vary depending on soil conditions and size of the fill. More frequent testing may be performed. In any case, an adequate number of field density tests should be made to evaluate the required compaction and moisture content is generally being obtained.
6. Laboratory testing to support field test procedures will be performed, as considered warranted, based on conditions encountered (e.g. change of material sources, types, etc.) Every effort will be made to process samples in the laboratory as quickly as possible and in progress construction projects are our first priority. However, laboratory workloads may cause delays and some soils may require a **minimum of 48 to 72 hours to complete test procedures**. Whenever possible, our representative(s) should be informed in advance of operational changes that might result in different source areas for materials.
7. Procedures for testing of fill slopes are as follows:
 - a) Density tests should be taken periodically during grading on the flat surface of the fill, three to five feet horizontally from the face of the slope.
 - b) If a method other than over building and cutting back to the compacted core is to be employed, slope compaction testing during construction should include testing the outer six inches to three feet in the slope face to determine if the required compaction is being achieved.
8. Finish grade testing of slopes and pad surfaces should be performed after construction is complete.

Site Clearing

1. All vegetation, and other deleterious materials, should be removed from the site. If material is not immediately removed from the site it should be stockpiled in a designated area(s) well outside of all current work areas and delineated with flagging or other means. Site clearing should be performed in advance of any grading in a specific area.
2. Efforts should be made by the contractor to remove all organic or other deleterious material from the fill, as even the most diligent efforts may result in the incorporation of some materials. This is especially important when grading is occurring near the natural grade. All equipment operators should be aware of these efforts. Laborers may be required as root pickers.
3. Nonorganic debris or concrete may be placed in deeper fill areas provided the procedures used are observed and found acceptable by our representative.

Treatment of Existing Ground

1. Following site clearing, all surficial deposits of alluvium and colluvium as well as weathered or creep affected bedrock, should be removed unless otherwise specifically indicated in the text of this report.
2. In some cases, removal may be recommended to a specified depth (e.g. flat sites where partial alluvial removals may be sufficient). The contractor should not exceed these depths unless directed otherwise by our representative.
3. Groundwater existing in alluvial areas may make excavation difficult. Deeper removals than indicated in the text of the report may be necessary due to saturation during winter months.
4. Subsequent to removals, the natural ground should be processed to a depth of six inches, moistened to near optimum moisture conditions and compacted to fill standards.
5. Exploratory backhoe or dozer trenches still remaining after site removal should be excavated and filled with compacted fill if they can be located.

Fill Placement

1. Unless otherwise indicated, all site soil and bedrock may be reused for compacted fill; however, some special processing or handling may be required (see text of report).
2. Material used in the compacting process should be evenly spread, moisture conditioned, processed, and compacted in thin lifts six (6) to eight (8) inches in compacted thickness to obtain a uniformly dense layer. The fill should be placed and compacted on a nearly horizontal plane, unless otherwise found acceptable by our representative.
3. If the moisture content or relative density varies from that recommended by this firm, the contractor should rework the fill until it is in accordance with the following:
 - a) The moisture content of the fill should be at or above optimum moisture. Moisture should be evenly distributed without wet and dry pockets. Pre-watering of cut or removal areas should be considered in addition to watering during fill placement, particularly in clay or dry surficial soils. The ability of the contractor to obtain the proper moisture content will control production rates.
 - b) Each six-inch layer should be compacted to at least 90 percent of the maximum dry density in compliance with the testing method specified by the controlling governmental agency. In most cases, the testing method is ASTM Test Designation D 1557.
4. Rock fragments less than eight inches in diameter may be utilized in the fill, provided:
 - a) They are not placed in concentrated pockets;
 - b) There is a sufficient percentage of fine-grained material to surround the rocks;
 - c) The distribution of the rocks is observed by, and acceptable to, our representative.

5. Rocks exceeding eight (8) inches in diameter should be taken off site, broken into smaller fragments, or placed in accordance with recommendations of this firm in areas designated suitable for rock disposal. On projects where significant large quantities of oversized materials are anticipated, alternate guidelines for placement may be included. If significant oversize materials are encountered during construction, these guidelines should be requested.
6. In clay soil, dry or large chunks or blocks are common. If in excess of eight (8) inches minimum dimension, then they are considered as oversized. Sheepsfoot compactors or other suitable methods should be used to break up blocks. When dry, they should be moisture conditioned to provide a uniform condition with the surrounding fill.

Slope Construction

1. The contractor should obtain a minimum relative compaction of 90 percent out to the finished slope face of fill slopes. This may be achieved by either overbuilding the slope and cutting back to the compacted core, or by direct compaction of the slope face with suitable equipment.
2. Slopes trimmed to the compacted core should be overbuilt by at least three (3) feet with compaction efforts out to the edge of the false slope. Failure to properly compact the outer edge results in trimming not exposing the compacted core and additional compaction after trimming may be necessary.
3. If fill slopes are built "at grade" using direct compaction methods, then the slope construction should be performed so that a constant gradient is maintained throughout construction. Soil should not be "spilled" over the slope face nor should slopes be "pushed out" to obtain grades. Compaction equipment should compact each lift along the immediate top of slope. Slopes should be back rolled or otherwise compacted at approximately every 4 feet vertically as the slope is built.
4. Corners and bends in slopes should have special attention during construction as these are the most difficult areas to obtain proper compaction.
5. Cut slopes should be cut to the finished surface. Excessive undercutting and smoothing of the face with fill may necessitate stabilization.

UTILITY TRENCH CONSTRUCTION AND BACKFILL

Utility trench excavation and backfill is the contractor's responsibility. The geotechnical consultant typically provides periodic observation and testing of these operations. While efforts are made to make sufficient observations and tests to verify that the contractors' methods and procedures are adequate to achieve proper compaction, it is typically impractical to observe all backfill procedures. As such, it is critical that the contractor use consistent backfill procedures.

Compaction methods vary for trench compaction and experience indicates many methods can be successful. However, procedures that “worked” on previous projects may or may not prove effective on a given site. The contractor(s) should outline the procedures proposed, so that we may discuss them **prior** to construction. We will offer comments based on our knowledge of site conditions and experience.

1. Utility trench backfill in slopes, structural areas, in streets and beneath flat work or hardscape should be brought to at least optimum moisture and compacted to at least 90 percent of the laboratory standard. Soil should be moisture conditioned prior to placing in the trench.
2. Flooding and jetting are not typically recommended or acceptable for native soils. Flooding or jetting may be used with select sand having a Sand Equivalent (SE) of 30 or higher. This is typically limited to the following uses:
 - a) shallow (12 + inches) under slab interior trenches and,
 - b) as bedding in pipe zone.

The water should be allowed to dissipate prior to pouring slabs or completing trench compaction.

3. Care should be taken not to place soils at high moisture content within the upper three feet of the trench backfill in street areas, as overly wet soils may impact subgrade preparation. Moisture may be reduced to 2% below optimum moisture in areas to be paved within the upper three feet below sub grade.
4. Sand backfill should not be allowed in exterior trenches adjacent to and within an area extending below a 1:1 projection from the outside bottom edge of a footing, unless it is similar to the surrounding soil.
5. Trench compaction testing is generally at the discretion of the geotechnical consultant. Testing frequency will be based on trench depth and the contractors’ procedures. A probing rod would be used to assess the consistency of compaction between tested areas and untested areas. If zones are found that are considered less compact than other areas, this should be brought to the contractor’s attention.

JOB SAFETY

General

Personnel safety is a primary concern on all job sites. The following summaries are safety considerations for use by all our employees on multi-employer construction sites. On ground personnel are at the highest risk of injury and possible fatality on grading construction projects. The company recognizes that construction activities will vary on each site and that job site safety is the contractor's responsibility. However, it is imperative that all personnel be safety conscious to avoid accidents and potential injury.



In an effort to minimize risks associated with geotechnical testing and observation, the following precautions are to be implemented for the safety of our field personnel on grading and construction projects.

1. Safety Meetings: Our field personnel are directed to attend the contractor's regularly scheduled safety meetings.
2. Safety Vests: Safety vests are provided for and are to be worn by our personnel while on the job site.
3. Safety Flags: Safety flags are provided to our field technicians; one is to be affixed to the vehicle when on site, the other is to be placed atop the spoil pile on all test pits.

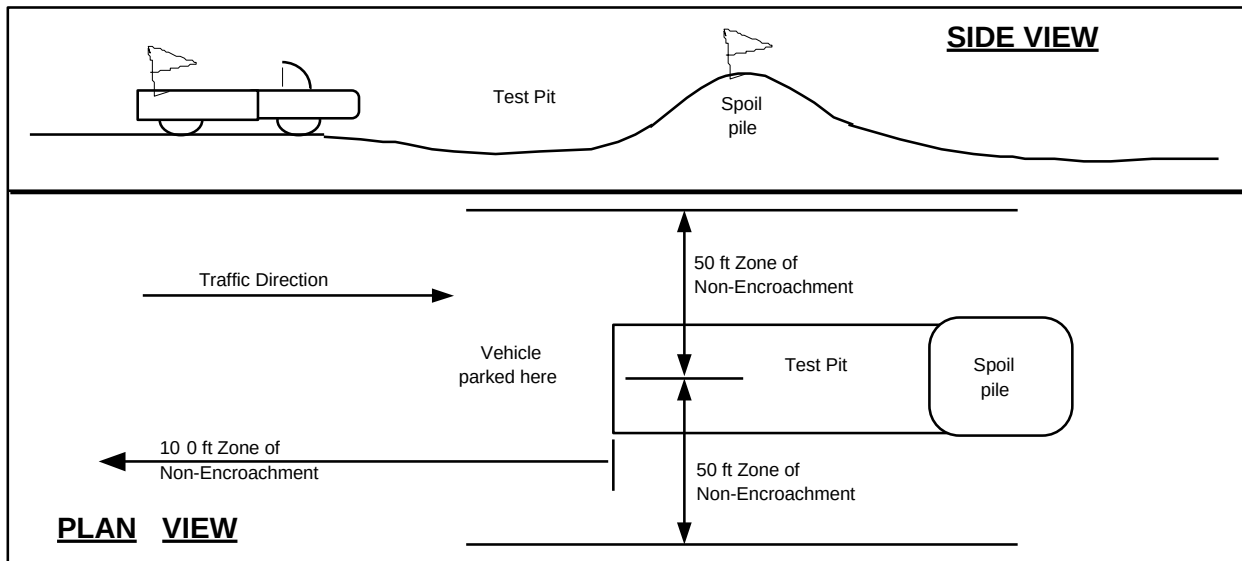
In the event that the contractor's representative observes any of our personnel not following the above, we request that it be brought to the attention of our office.

Test Pits Location, Orientation and Clearance

The technician is responsible for selecting test pit locations. The primary concern is the technician's safety. However, it is necessary to take sufficient tests at various locations to obtain a representative sampling of the fill. As such, efforts will be made to coordinate locations with the grading contractors authorized representatives (e.g. dump man, operator, supervisor, grade checker, etc.), and to select locations following or behind the established traffic pattern, preferably outside of current traffic. The contractors authorized representative should direct excavation of the pit and safety during the test period. Again, safety is the paramount concern.

Test pits should be excavated so that the spoil pile is placed away from oncoming traffic. The technician's vehicle is to be placed next to the test pit, opposite the spoil pile. This necessitates that the fill be maintained in a drivable condition. Alternatively, the contractor may opt to park a piece of equipment in front of test pits, particularly in small fill areas or those with limited access.

A zone of non-encroachment should be established for all test pits (see diagram below). No grading equipment should enter this zone during the test procedure. The zone should extend outward to the sides approximately 50 feet from the center of the test pit and 100 feet in the direction of traffic flow. This zone is established both for safety and to avoid excessive ground vibration, which typically decreases test results.

TEST PIT SAFETY PLAN**Slope Tests**

When taking slope tests, the technician should park their vehicle directly above or below the test location on the slope. The contractor's representative should effectively keep all equipment at a safe operation distance (e.g. 50 feet) away from the slope during testing.

The technician is directed to withdraw from the active portion of the fill as soon as possible following testing. The technician's vehicle should be parked at the perimeter of the fill in a highly visible location.

Trench Safety

It is the contractor's responsibility to provide safe access into trenches where compaction testing is needed. Trenches for all utilities should be excavated in accordance with CAL-OSHA and any other applicable safety standards. Safe conditions will be required to enable compaction testing of the trench backfill.

All utility trench excavations in excess of 5 feet deep, which a person enters, are to be shored or laid back. Trench access should be provided in accordance with OSHA standards. Our personnel are directed not to enter any trench by being lowered or "riding down" on the equipment.

Our personnel are directed not to enter any excavation which;

1. is 5 feet or deeper unless shored or laid back,
2. exit points or ladders are not provided,
3. displays any evidence of instability, has any loose rock or other debris which could fall into the trench, or

4. displays any other evidence of any unsafe conditions regardless of depth.

If the contractor fails to provide safe access to trenches for compaction testing, our company policy requires that the soil technician withdraws and notifies their supervisor. The contractor's representative will then be contacted in an effort to effect a solution. All backfill not tested due to safety concerns or other reasons is subject to reprocessing and/or removal.

Procedures

In the event that the technician's safety is jeopardized or compromised as a result of the contractor's failure to comply with any of the above, the technician is directed to inform both the developer's and contractor's representatives. If the condition is not rectified, the technician is required, by company policy, to immediately withdraw and notify their supervisor. The contractor's representative will then be contacted in an effort to affect a solution. No further testing will be performed until the situation is rectified. Any fill placed in the interim can be considered unacceptable and subject to reprocessing, recompaction or removal.

In the event that the soil technician does not comply with the above or other established safety guidelines, we request that the contractor bring this to the technician's attention and notify our project manager or office. Effective communication and coordination between the contractors' representative and the field technician(s) is strongly encouraged in order to implement the above safety program and safety in general.

The safety procedures outlined above should be discussed at the contractor's safety meetings. This will serve to inform and remind equipment operators of these safety procedures, particularly the zone of non-encroachment.

The safety procedures outlined above should be discussed at the contractor's safety meetings. This will serve to inform and remind equipment operators of these safety procedures, particularly the zone of non-encroachment.